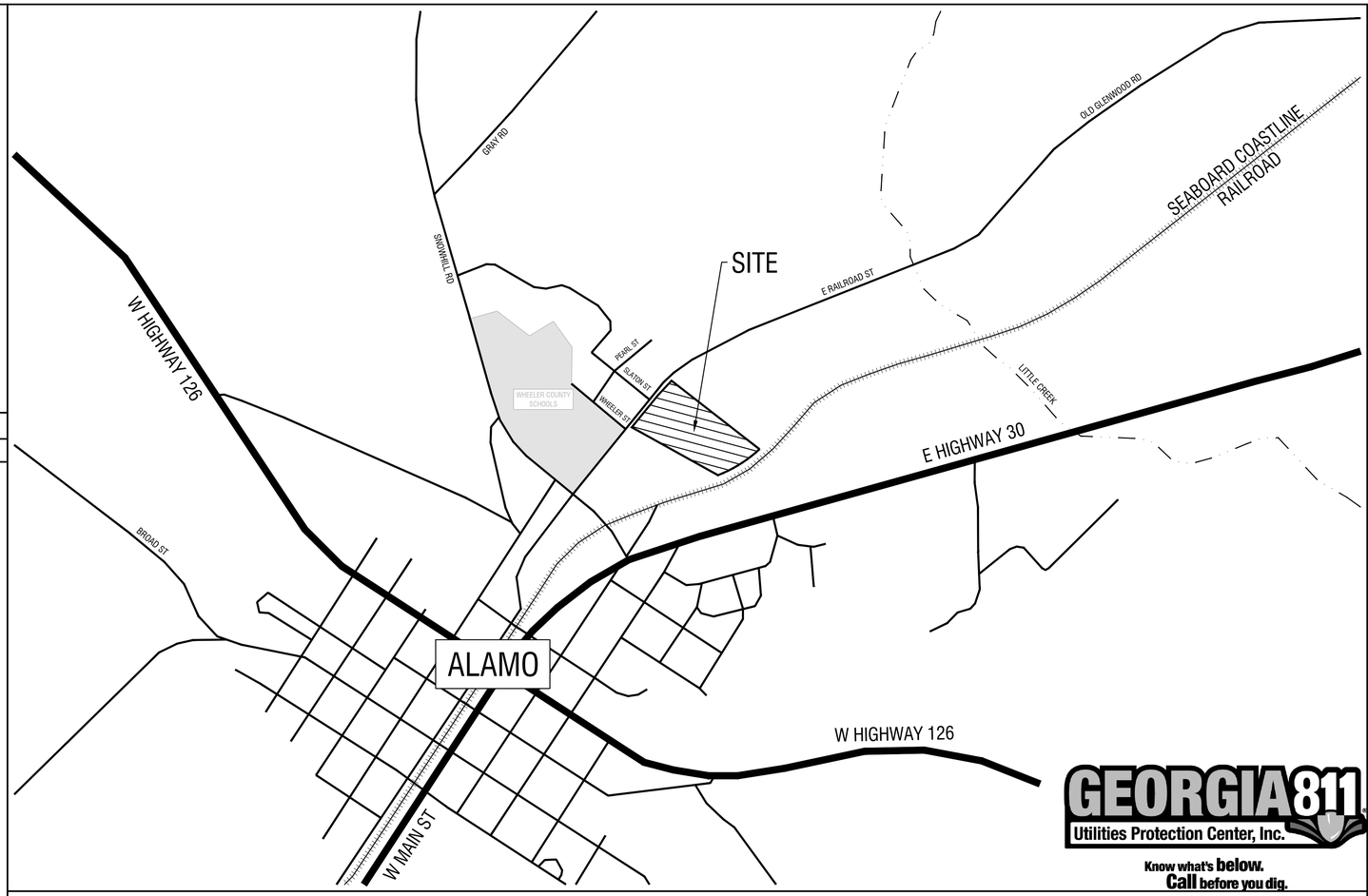
EAST SIDE PARK REMODEL
FOR THE CITY OF ALAMO
WHEELER COUNTY, GEORGIA
DATE: NOVEMBER 7, 2023

CIVIL DRAWING LEGEND

DESCRIPTION	PROPOSED	EXISTING
SANITARY SEWER		SS
UNDERGROUND WATER LINE	W	W
FORCE MAIN	FM	FM
STORM DRAINAGE PIPE		
UNDERGROUND TELEPHONE LINE	T	T
UNDERGROUND TELEPHONE CONDUIT	TC	TC
UNDERGROUND GAS LINE	12"G	12"G
DITCH CENTERLINE		
SPOT ELEVATION	X=90.00	X=90.00
TOP OF CURB ELEVATION	TC=90.00	TC=90.00
FIRE HYDRANT		
SEWER MANHOLE		
WATER VALVE	WV	WV
TELEPHONE MANHOLE		
LIGHT POLE		
SIGN		
WATER METER		
BENCHMARK		
CONCRETE MONUMENT FOUND		
GUY POLE		
IRON PIN FOUND		
IRON PIN SET		
TELEPHONE PEDESTAL		
POWER POLE		
HANDICAP SPACE		
SEDIMENT BASIN MARKER W/NOTCH	SBM	SBM



VICINITY MAP
Not To Scale

GEORGIA 811
Utilities Protection Center, Inc.
Know what's **below.**
Call before you dig.

MINIMUM PROJECT SPECIFICATIONS

ANY JOB DESIGNED BY M.E. SACK ENGINEERING SHALL BE CONSTRUCTED IN ACCORDANCE WITH M.E. SACK ENGINEERING MINIMUM SPECIFICATIONS. IN MUNICIPALITIES WITH THEIR OWN SPECIFICATIONS, THEIR SPECIFICATIONS SHALL SUPERSEDE THE MINIMUM SPECIFICATIONS. LIKEWISE, PROJECTS THAT ARE BID SHALL BE HELD TO THE SPECIFICATIONS WITHIN THE BID DOCUMENT WHICH MAY DIFFER FROM THE MINIMUM SPECIFICATION REQUIREMENTS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THEY HAVE THE APPROPRIATE SPECIFICATIONS FOR EACH PROJECT. OBTAIN THE MINIMUM STANDARDS BY SCANNING THE QR CODE OR CLICKING THIS LINK: https://www.mesack.com/technical-specification/

CONTACT ADMIN@MESACK.COM TO REQUEST CONFIRMATION OF CORRECT SPECIFICATIONS AT ANYTIME.

OWNER:
CITY OF ALAMO
5 WEST MAIN STREET
ALAMO, GA 30411
(912) 568-7153
MAYOR@CITYOFALAMO.US

GPS COORDINATES OF CO:
E: -82.773519
N: 32.154578

DATUM: NAVD88

24 HOUR CONTACT:
JEFFERY FLOYD
5 WEST MAIN STREET
ALAMO, GA 30411
(912) 568-7153
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DESIGN PROFESSIONAL:
GEORGIA REGISTERED No. 25556 PROFESSIONAL ENGINEER MARCUS E. SACK
DATE: 4/2/2025

M.E. SACK ENGINEERING

515 NORTH MAIN STREET
P.O. BOX 649
HINESVILLE, GA 31313
TEL: (912) 368-5212

10375 FORD AVENUE
UNIT A-2
RICHMOND HILL, GA 31324

GENERAL NOTES

1. ALL EXISTING UTILITIES SHOWN ARE LOCATED FROM BEST AVAILABLE INFORMATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACTUAL FIELD LOCATION AND PROTECTION OF EXISTING UTILITIES. OVERHEAD LINES ARE NOT SHOWN FOR CLARITY.
2. ALL DISTURBED AREAS TO BE REVEGETATED IMMEDIATELY AFTER CONSTRUCTION, IN ACCORDANCE WITH THE MANUAL FOR EROSION AND SEDIMENT CONTROL IN GEORGIA.
3. ALL EROSION AND SEDIMENTATION CONTROL STRUCTURES SHALL BE INSTALLED PRIOR TO THE START OF CONSTRUCTION.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REPLACEMENT OF ANY PROPERTY CORNERS, RIGHT OF WAY MONUMENTS, SIGNS OR OTHER STRUCTURES DISTURBED DURING CONSTRUCTION.
5. ALL TRAFFIC AND SIGNAGE CONTROL SHALL BE IN ACCORDANCE WITH THE MANUAL FOR UNIFORM TRAFFIC CONTROL DEVICES, MUTCD, CURRENT EDITION.

JOB NO. 2023-66PRJ

REVISION NO.	DATE	DESCRIPTION
1.	2/4/2025	ARCH/STRUC/ELEC FINAL ADJUSTMENTS

COVER SHEET

PLOT DATE: April 2, 2025

GENERAL

CIVIL

ARCHITECTURAL

STRUCTURAL - EAST SIDE PARK AMPHITHEATER (& RETAINING WALLS)

STRUCTURAL - EAST SIDE PARK REMODEL (OVERHEAD STEEL STRUCTURE)

ELECTRICAL

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
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GSWCC LEVEL II # 70248
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MARCUS@MESACK.COM

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GEORGIA REGISTERED No. 36559 PROFESSIONAL ENGINEER MARCUS E. SACK

DATE: 4/2/2025

M.E. SACK ENGINEERING 00

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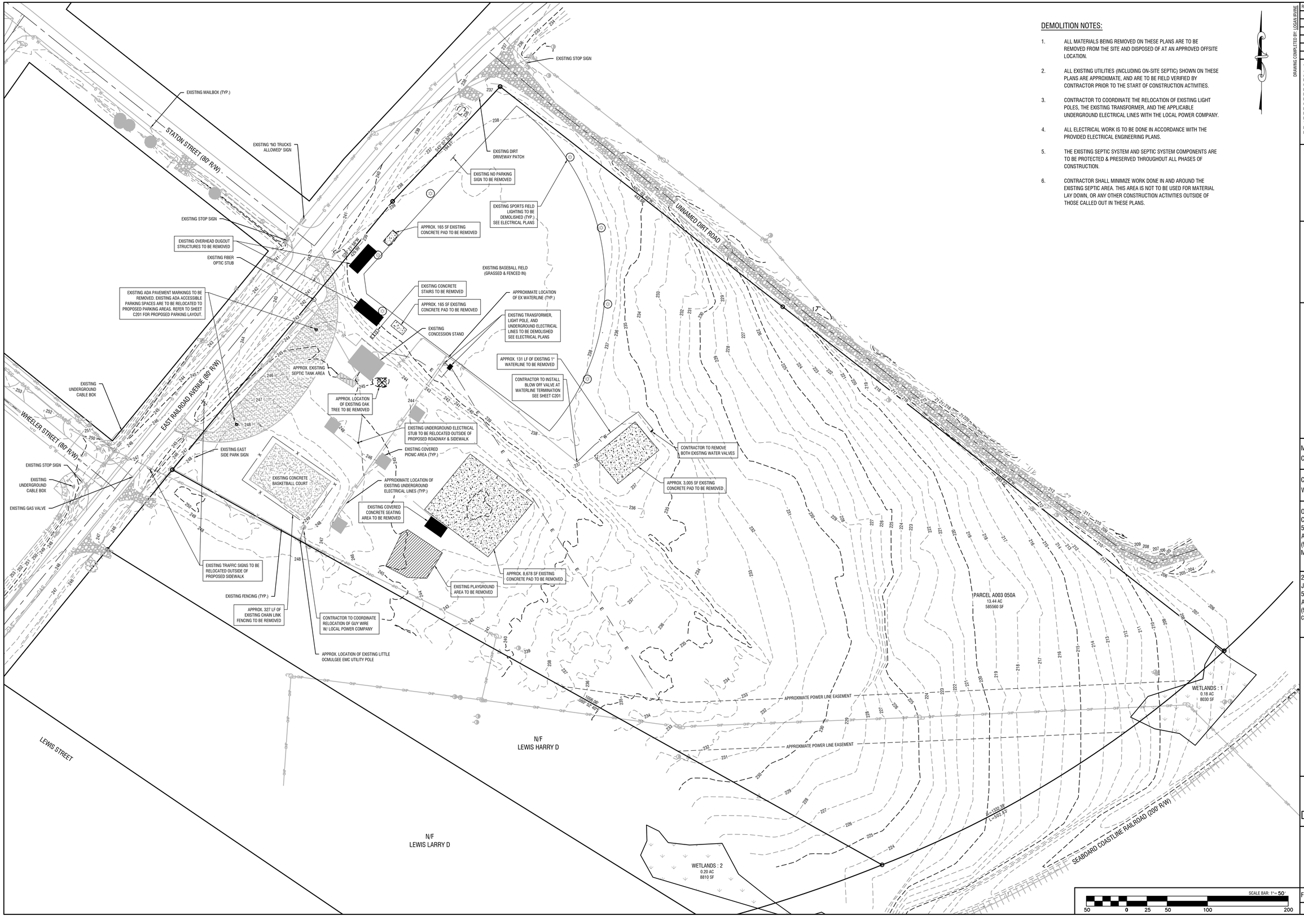
EAST SIDE PARK REMODEL

DRAWING INDEX

G1

FILE NO: 2023-66

PLOT DATE: April 2, 2025



DEMOLITION NOTES:

1. ALL MATERIALS BEING REMOVED ON THESE PLANS ARE TO BE REMOVED FROM THE SITE AND DISPOSED OF AT AN APPROVED OFFSITE LOCATION.
2. ALL EXISTING UTILITIES (INCLUDING ON-SITE SEPTIC) SHOWN ON THESE PLANS ARE APPROXIMATE, AND ARE TO BE FIELD VERIFIED BY CONTRACTOR PRIOR TO THE START OF CONSTRUCTION ACTIVITIES.
3. CONTRACTOR TO COORDINATE THE RELOCATION OF EXISTING LIGHT POLES, THE EXISTING TRANSFORMER, AND THE APPLICABLE UNDERGROUND ELECTRICAL LINES WITH THE LOCAL POWER COMPANY.
4. ALL ELECTRICAL WORK IS TO BE DONE IN ACCORDANCE WITH THE PROVIDED ELECTRICAL ENGINEERING PLANS.
5. THE EXISTING SEPTIC SYSTEM AND SEPTIC SYSTEM COMPONENTS ARE TO BE PROTECTED & PRESERVED THROUGHOUT ALL PHASES OF CONSTRUCTION.
6. CONTRACTOR SHALL MINIMIZE WORK DONE IN AND AROUND THE EXISTING SEPTIC AREA. THIS AREA IS NOT TO BE USED FOR MATERIAL LAY DOWN, OR ANY OTHER CONSTRUCTION ACTIVITIES OUTSIDE OF THOSE CALLED OUT IN THESE PLANS.

SCALE BAR: 1"= 50'
50 0 25 50 100 200

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REVISIONS:

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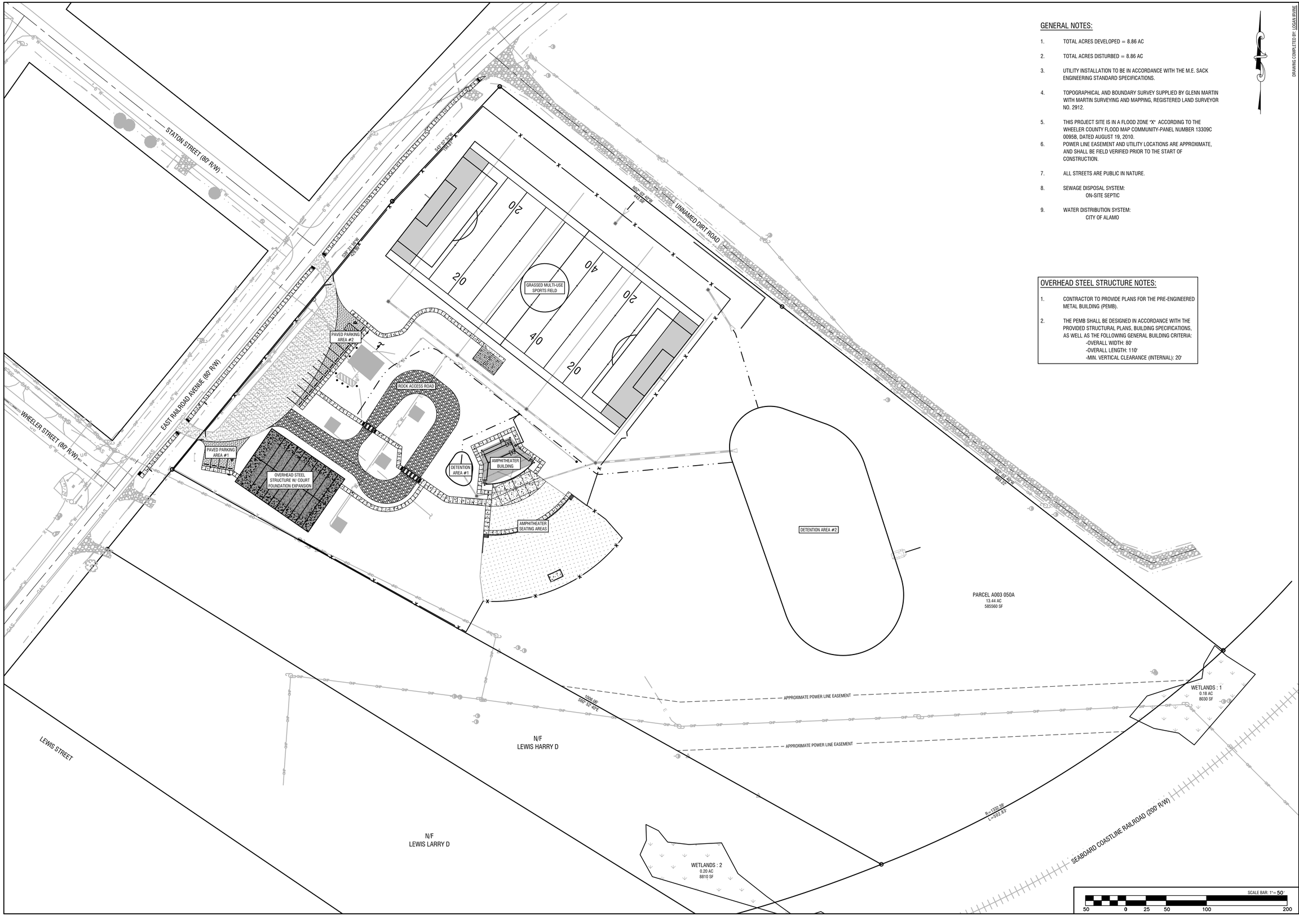
EAST SIDE PARK REMODEL

EXISTING CONDITIONS & DEMOLITION PLAN

C100

FILE NO: 2023-66

PLOT DATE: April 2, 2025



GENERAL NOTES:

1. TOTAL ACRES DEVELOPED = 8.86 AC
2. TOTAL ACRES DISTURBED = 8.86 AC
3. UTILITY INSTALLATION TO BE IN ACCORDANCE WITH THE M.E. SACK ENGINEERING STANDARD SPECIFICATIONS.
4. TOPOGRAPHICAL AND BOUNDARY SURVEY SUPPLIED BY GLENN MARTIN WITH MARTIN SURVEYING AND MAPPING, REGISTERED LAND SURVEYOR NO. 2912.
5. THIS PROJECT SITE IS IN A FLOOD ZONE "X" ACCORDING TO THE WHEELER COUNTY FLOOD MAP COMMUNITY-PANEL NUMBER 13309C 0095B, DATED AUGUST 19, 2010.
6. POWER LINE EASEMENT AND UTILITY LOCATIONS ARE APPROXIMATE, AND SHALL BE FIELD VERIFIED PRIOR TO THE START OF CONSTRUCTION.
7. ALL STREETS ARE PUBLIC IN NATURE.
8. SEWAGE DISPOSAL SYSTEM:
   ON-SITE SEPTIC
9. WATER DISTRIBUTION SYSTEM:
   CITY OF ALAMO

OVERHEAD STEEL STRUCTURE NOTES:

1. CONTRACTOR TO PROVIDE PLANS FOR THE PRE-ENGINEERED METAL BUILDING (PEMB).
2. THE PEMB SHALL BE DESIGNED IN ACCORDANCE WITH THE PROVIDED STRUCTURAL PLANS, BUILDING SPECIFICATIONS, AS WELL AS THE FOLLOWING GENERAL BUILDING CRITERIA:
   -OVERALL WIDTH: 80'
   -OVERALL LENGTH: 110'
   -MIN. VERTICAL CLEARANCE (INTERNAL): 20'

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

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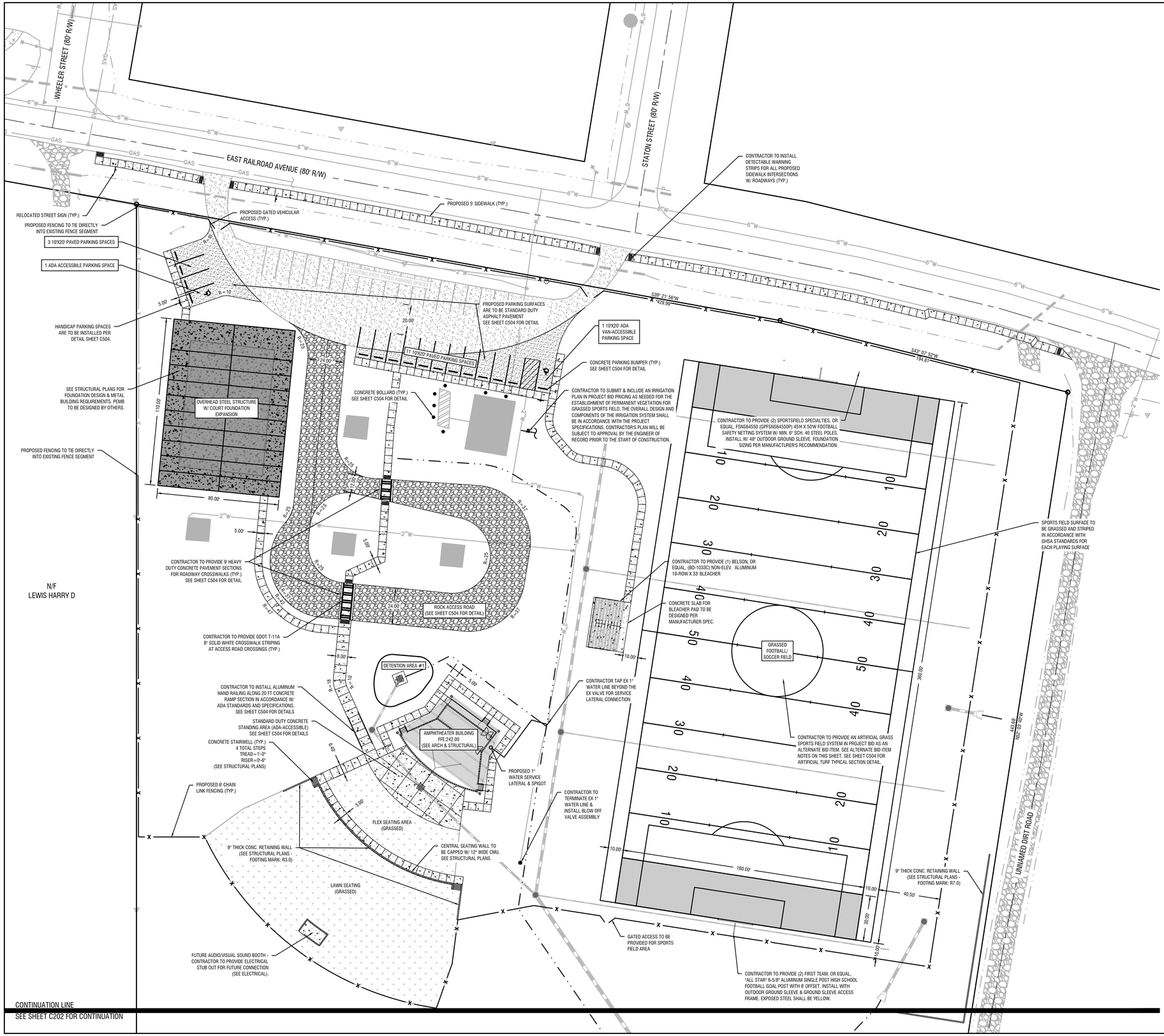
EAST SIDE PARK REMODEL

OVERALL SITE & UTILITY PLAN

C200

FILE NO: 2023-66

PLOT DATE: April 2, 2025



CONTINUATION LINE
SEE SHEET C202 FOR CONTINUATION

ALTERNATE BID ITEM NOTES:

1. CONTRACTOR TO INCLUDE THE DESIGN AND INSTALLATION OF AN ARTIFICIAL GRASS (TURF) SPORTS FIELD SYSTEM IN PROJECT BIS AS AN ALTERNATE BID ITEM.
2. THE PRODUCT MANUFACTURER PROVIDED IN THE ALTERNATE BID ITEM SHALL BE IN COMPLIANCE WITH THE SPECIFICATIONS.
3. THE PRODUCT INSTALLATION SHALL BE DONE IN ACCORDANCE WITH THE PROJECT PLANS AND BID DOCUMENTS, AS WELL AS THE MANUFACTURER SPECIFICATIONS AND RECOMMENDATIONS.
4. THE ARTIFICIAL GRASS SPORTS FIELD SHALL, AT A MINIMUM, BE INSTALLED WITH THE DIMENSIONS SHOWN ON THIS PLAN SHEET.

SCALE BAR: 1"= 30'
30 0 15 30 60 120

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REVISIONS:

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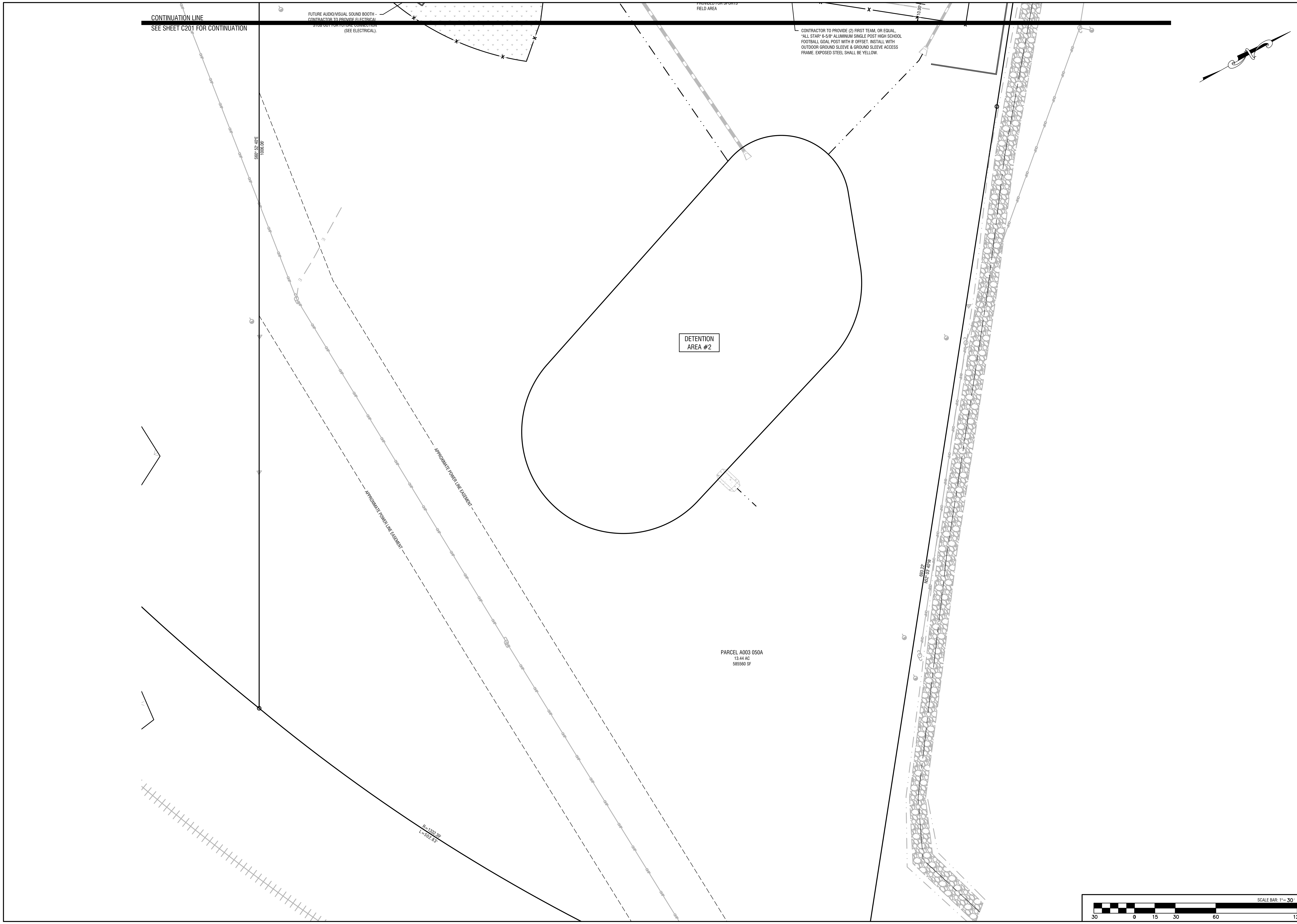
EAST SIDE PARK REMODEL

SITE & UTILITY PLAN

C201

FILE NO: 2023-66

PLOT DATE: April 2, 2025



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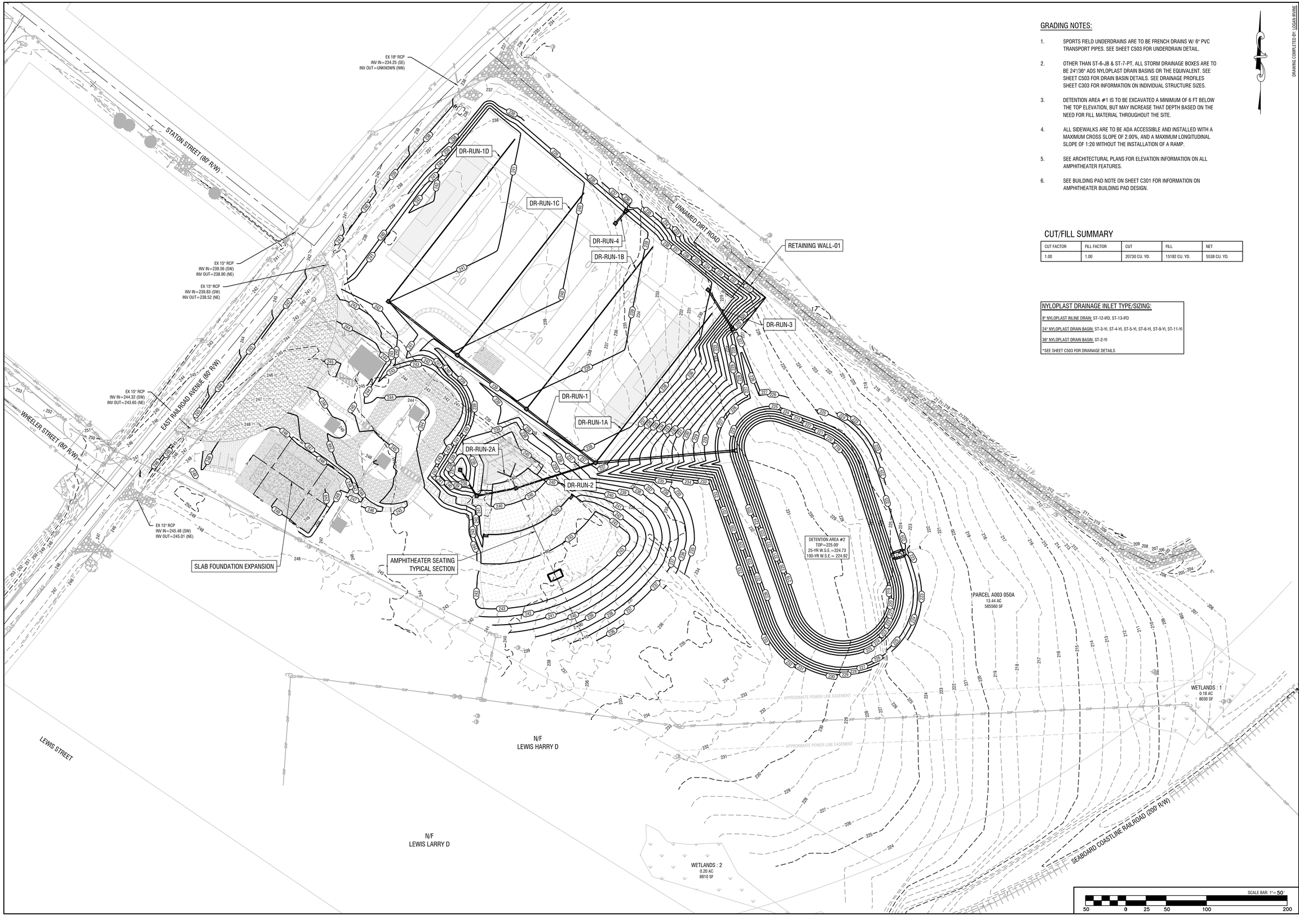
EAST SIDE PARK REMODEL

SITE & UTILITY PLAN

C202

FILE NO: 2023-66

PLOT DATE: April 2, 2025



GRADING NOTES:

1. SPORTS FIELD UNDERDRAINS ARE TO BE FRENCH DRAINS W/ 6" PVC TRANSPORT PIPES. SEE SHEET C503 FOR UNDERDRAIN DETAIL..
2. OTHER THAN ST-6-JB & ST-7-PT, ALL STORM DRAINAGE BOXES ARE TO BE 24"/36" ADS NYLOPLAST DRAIN BASINS OR THE EQUIVALENT. SEE SHEET C503 FOR DRAIN BASIN DETAILS. SEE DRAINAGE PROFILES SHEET C303 FOR INFORMATION ON INDIVIDUAL STRUCTURE SIZES.
3. DETENTION AREA #1 IS TO BE EXCAVATED A MINIMUM OF 6 FT BELOW THE TOP ELEVATION, BUT MAY INCREASE THAT DEPTH BASED ON THE NEED FOR FILL MATERIAL THROUGHOUT THE SITE.
4. ALL SIDEWALKS ARE TO BE ADA ACCESSIBLE AND INSTALLED WITH A MAXIMUM CROSS SLOPE OF 2.00%, AND A MAXIMUM LONGITUDINAL SLOPE OF 1:20 WITHOUT THE INSTALLATION OF A RAMP.
5. SEE ARCHITECTURAL PLANS FOR ELEVATION INFORMATION ON ALL AMPHITHEATER FEATURES.
6. SEE BUILDING PAD NOTE ON SHEET C301 FOR INFORMATION ON AMPHITHEATER BUILDING PAD DESIGN.

CUT/FILL SUMMARY

CUT FACTOR	FILL FACTOR	CUT	FILL	NET
1.00	1.00	20730 CU. YD.	15192 CU. YD.	5538 CU. YD.

NYLOPLAST DRAINAGE INLET TYPE/SIZING:

8" NYLOPLAST INLINE DRAIN: ST-12-IFD, ST-13-IFD

24" NYLOPLAST DRAIN BASIN: ST-3-YI, ST-4-YI, ST-5-YI, ST-6-YI, ST-9-YI, ST-11-YI

36" NYLOPLAST DRAIN BASIN: ST-2-YI

*SEE SHEET C503 FOR DRAINAGE DETAILS

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

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EAST SIDE PARK REMODEL

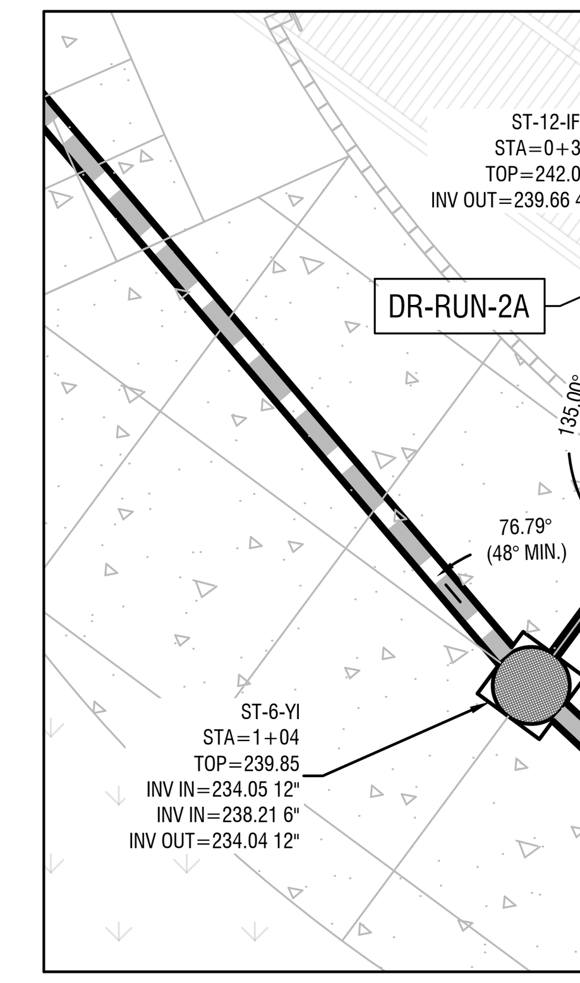
OVERALL GRADING PLAN

C300

FILE NO: 2023-66

PLOT DATE: April 2, 2025


WHEELER STREET (80' R/W)
STATON STREET (80' R/W)
EAST RAILROAD AVENUE (80' R/W)
UNNAMED DIRT ROAD
N/F
LEWIS HARRY D
EXPANDED SLAB
(ELEV. VARIES)
SEE SHEET C304 FOR
GRADING DETAILS
ALL PROPOSED SIDEWALKS TO TIE-IN FLUSH WITH PARKING LOTS & ROADWAYS (TYP.)
CONTRACTOR TO PROVIDE POSITIVE DRAINAGE AROUND EX SEPTIC AREA
SLOPE OF CONCRETE PAD SHALL NOT EXCEED 1/8" PER FOOT PER MANUFACTURER RECOMMENDATION
CONTRACTOR TO PROVIDE 5' SHOULDER W/ 1.00% SLOPE ALONG OUTER PERIMETER OF GRAVEL ROADWAY LOOP (TYP.)
FLAT LANDING AREA W/ MIN. LENGTH OF 5 FT TO BE PROVIDED AT TOP & BOTTOM OF ADA ACCESSIBLE RAMP SECTION (TYP.)
CONTRACTOR SHALL TIE BACK TO EX GRADE AT A SLOPE OF 5:1, SLOPES SHALL NOT EXCEED 3:1 (TYP.)
SEE SHEET C503 FOR PEDESTAL TOP DETAIL
SEE FLOOR DRAIN JUNCTION DETAIL ON THIS SHEET
PERIMETER SIDEWALK AROUND AMPHITHEATER SHALL HAVE A CONSTANT CROSS SLOPE OF 2.00% (TYP.)
BUILDING PAD LIMITS (TYP.) SEE NOTE THIS SHEET
PAD ELEV. 240.00 (FFE:242.00)
PAD ELEV. 244.22
AMPHITHEATER SEATING TYPICAL SECTION
DR-RUN-1
DR-RUN-1A
DR-RUN-1B
DR-RUN-1C
DR-RUN-1D
DR-RUN-2
DR-RUN-2A
DR-RUN-3
DR-RUN-4
RETAINING WALL-01
UNDERDRAIN INV #1 STA=2+35 INV OUT=235.39 6"
UNDERDRAIN INV #2 STA=2+55 INV OUT=236.38 6"
UNDERDRAIN INV #3 STA=2+55 INV OUT=237.46 6"
UNDERDRAIN INV #4 STA=2+55 INV OUT=238.55 6"
ST-2-YI STA=1+74 TOP=237.76 INV IN=229.79 18" INV IN=233.04 12" INV IN=233.04 6" INV OUT=229.79 24"
ST-3-YI STA=2+82 TOP=238.84 INV IN=233.83 18" INV IN=233.83 6" INV OUT=233.83 18"
ST-4-YI STA=3+91 TOP=239.93 INV IN=234.91 18" INV IN=234.91 6" INV OUT=234.91 18"
ST-5-YI STA=4+99 TOP=241.01 INV IN=236.00 6" INV OUT=236.00 18"
ST-6-YI STA=1+04 TOP=239.85 INV IN=234.05 12" INV IN=238.21 6" INV OUT=234.04 12"
ST-7-JB STA=1+53 TOP=240.28 INV IN=234.53 12" INV OUT=234.52 12"
ST-8-PT STA=1+91 TOP=237.50 THROAT I.E.=236.50 INV OUT=234.90 12"
ST-9-FES STA=0+00 TOP=229.85 INV IN=228.13 18"
ST-10-YI STA=0+56 TOP=238.03 INV OUT=233.75 18"
ST-11-FES STA=0+00 TOP=238.49 INV IN=236.77 18"
ST-12-YI STA=0+20 TOP=239.47 INV OUT=236.84 18"
EX 15" RCP INV IN=244.32 (SW) INV OUT=243.65 (NE)
EX 15" RCP INV IN=245.48 (SW) INV OUT=245.01 (NE)
EX 15" RCP INV IN=239.56 (SW) INV OUT=238.90 (NE)
EX 15" RCP INV IN=239.83 (SW) INV OUT=238.52 (NE)
EX 18" RCP INV IN=234.25 (SE) INV OUT=UNKNOWN (NW)
CONTINUATION LINE
SEE SHEET C302 FOR CONTINUATION


SEE ARCH PLANS FOR INFORMATION ON AMPHITHEATER ELEVATIONS, FLOOR DRAIN ELEVATIONS, AND 6" DRAIN LINE PENETRATION DETAIL.
ST-12-IFD STA=0+33 TOP=242.00 INV OUT=239.66 4"
ST-13-IFD TOP=242.00 INV OUT=239.66 4"
DR-RUN-2A
30 LF 4" PVC FLOOR DRAIN PIPES @ MIN. 1/4" PER FOOT SLOPE (TYP.)
4"X6" WYE FITTING STA=0+16 TOP=239.41 INV IN=238.95 4" INV IN=238.95 4" INV OUT=238.87 6"
ST-6-YI STA=1+04 TOP=239.85 INV IN=234.05 12" INV IN=238.21 6" INV OUT=234.04 12"

FLOOR DRAIN JUNCTION DETAIL
SCALE: 1"= 10'

BUILDING PAD NOTE:
THE LIMITS OF THE AMPHITHEATER BUILDING PAD ELEVATION ARE TO EXTEND TO THE LIMITS OF THE BUILDING FOOTING, AS SHOWN ON THIS PLAN SHEET. SEE ARCHITECTURAL FOUNDATION PLAN & STRUCTURAL PLANS FOR DETAILS ON BUILDING FOUNDATION.

SCALE BAR: 1"= 30'
30 0 15 30 60 120

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:	
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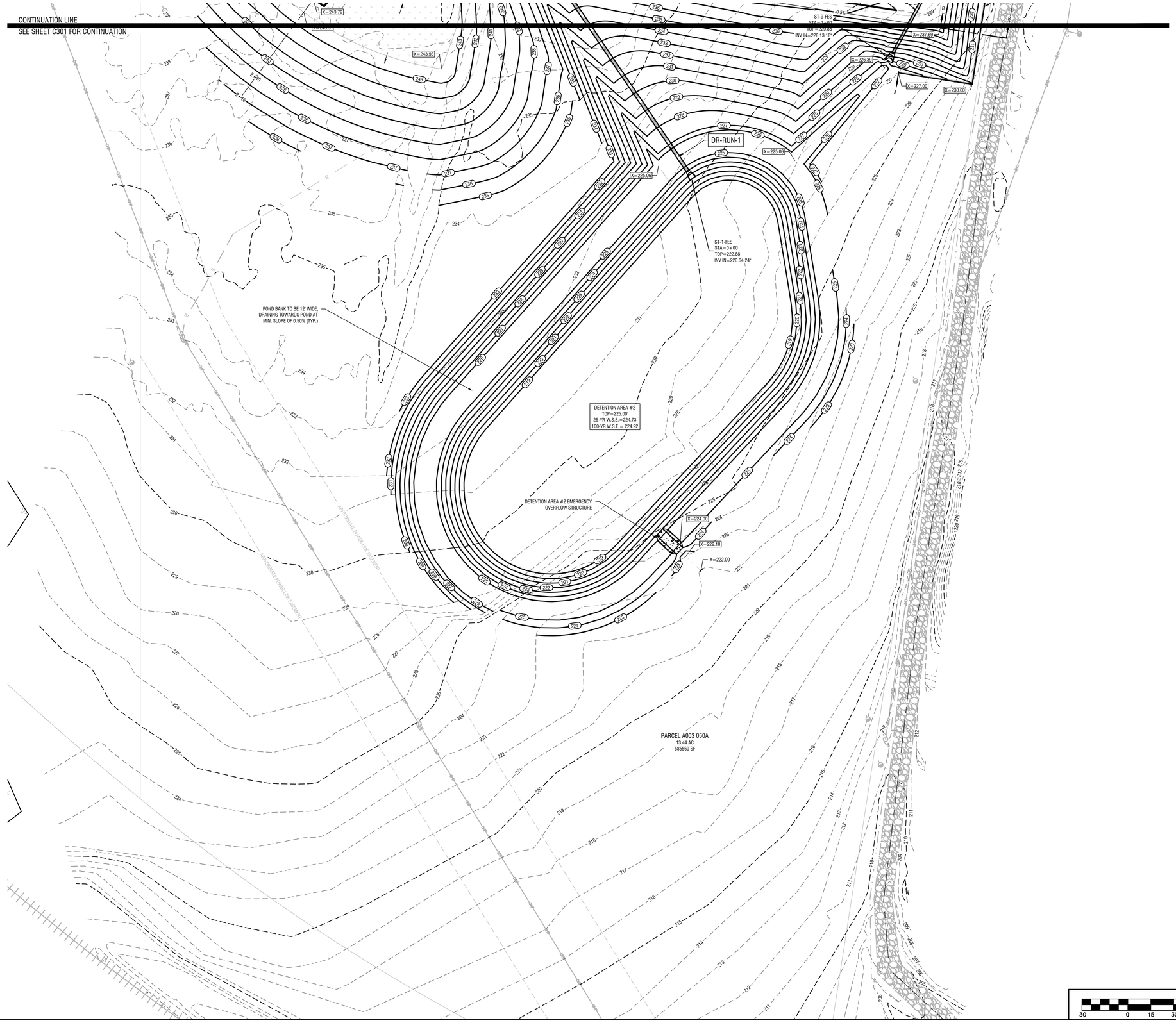
EAST SIDE PARK REMODEL

GRADING PLAN

C301

FILE NO: 2023-66
PLOT DATE: April 2, 2025



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COUNTY:
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OWNER:
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EAST SIDE PARK REMODEL

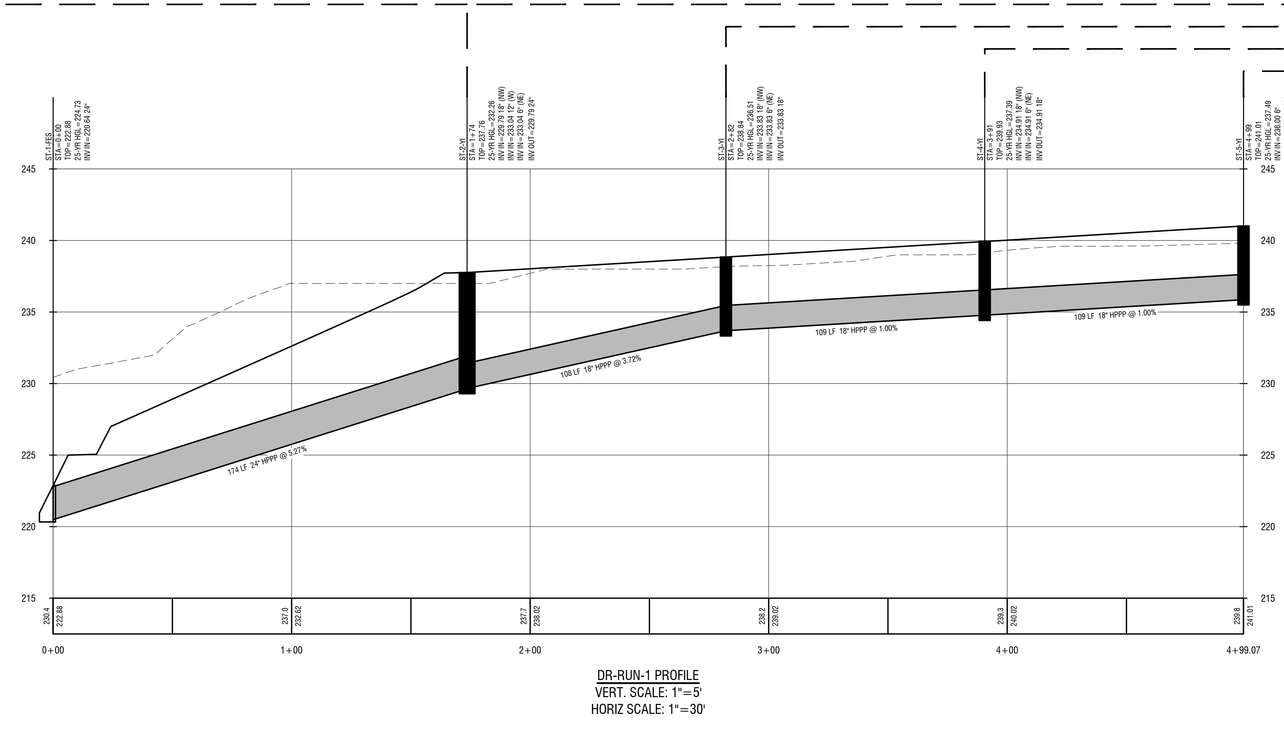
GRADING PLAN

C302

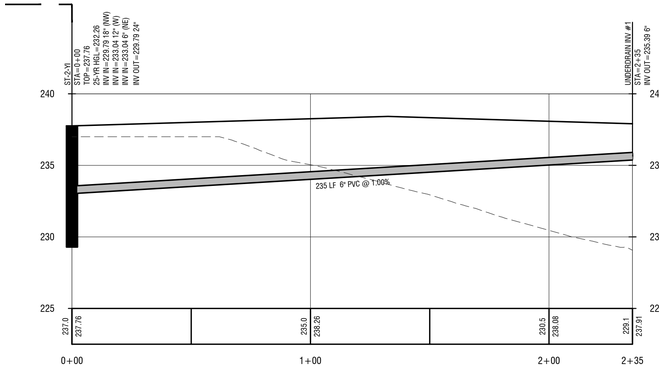
SCALE BAR: 1"= 30'
30 0 15 30 60 120

FILE NO: 2023-66

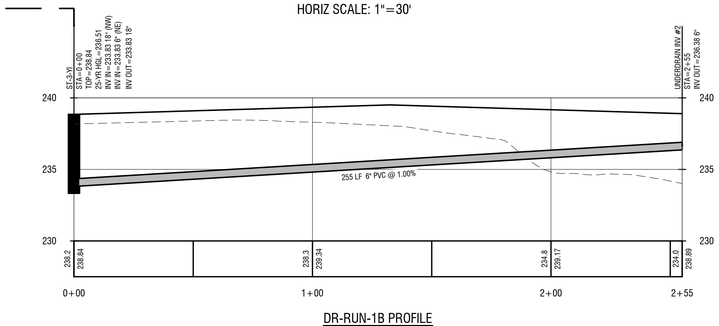
PLOT DATE: April 2, 2025



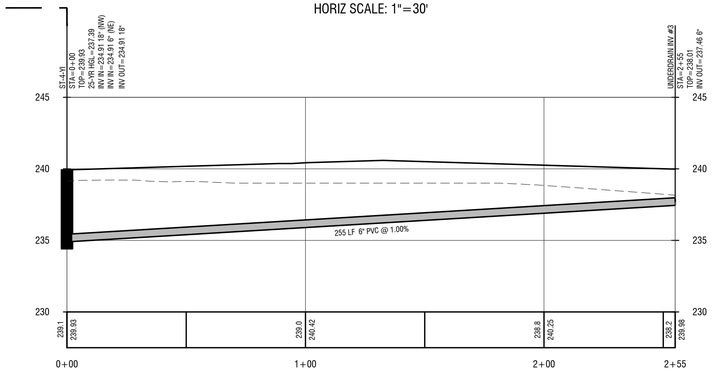
DR-RUN-1 PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



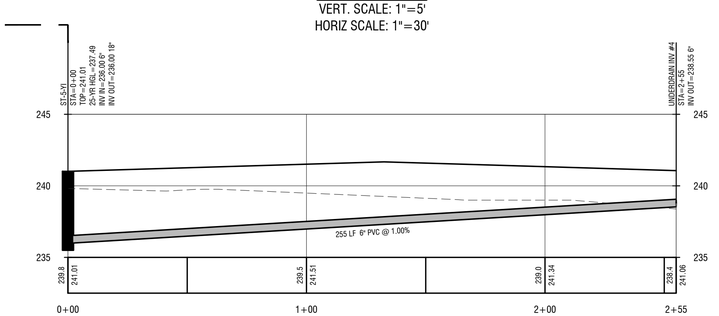
DR-RUN-1A PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



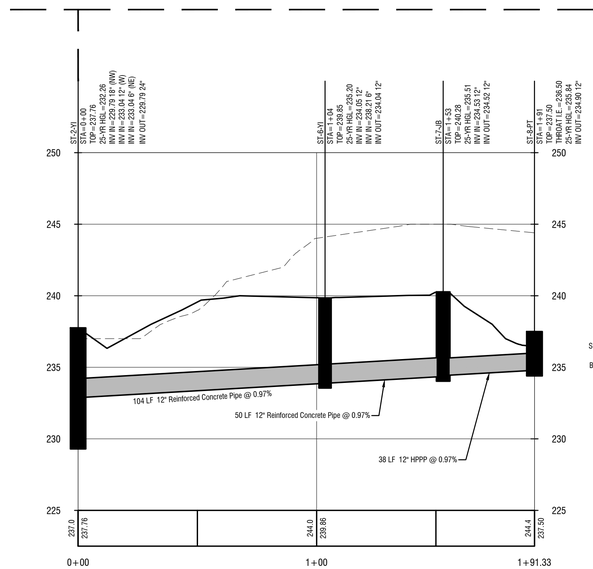
DR-RUN-1B PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



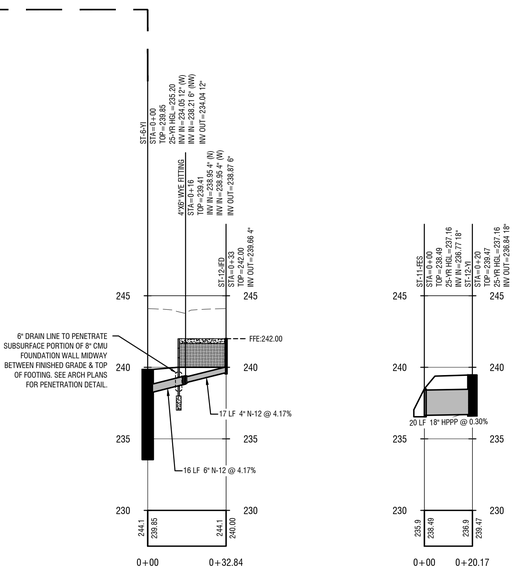
DR-RUN-1C PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



DR-RUN-1D PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'

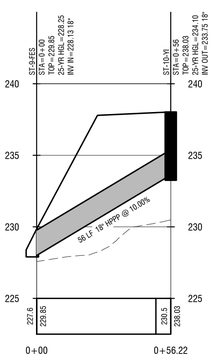


DR-RUN-2 PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



DR-RUN-2A PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'

DR-RUN-4 PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



DR-RUN-3 PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DRAWING COMPLETED BY: LOGAN IRVINE

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GEORGIA REGISTERED No. 36559 PROFESSIONAL ENGINEER MARCUS E. SACK

DATE: 4/2/2025

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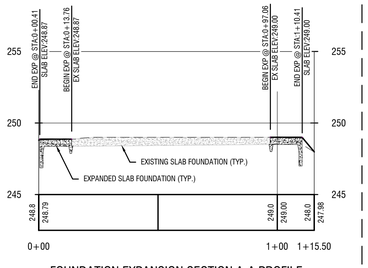
EAST SIDE PARK REMODEL

DRAINAGE PROFILES

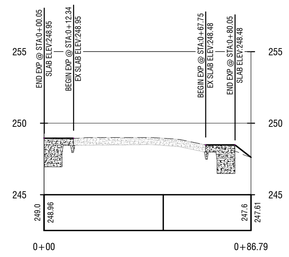
C303

FILE NO: 2023-66

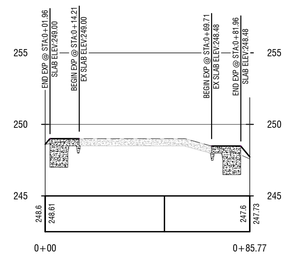
PLOT DATE: April 2, 2025



FOUNDATION EXPANSION SECTION A-A PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



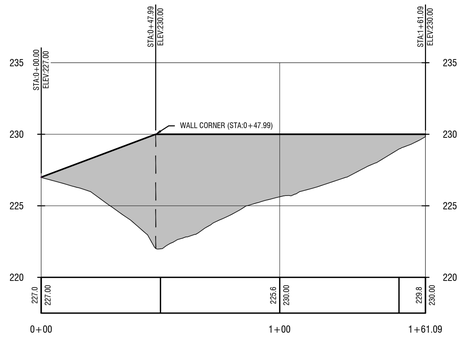
FOUNDATION EXPANSION SECTION B-B PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



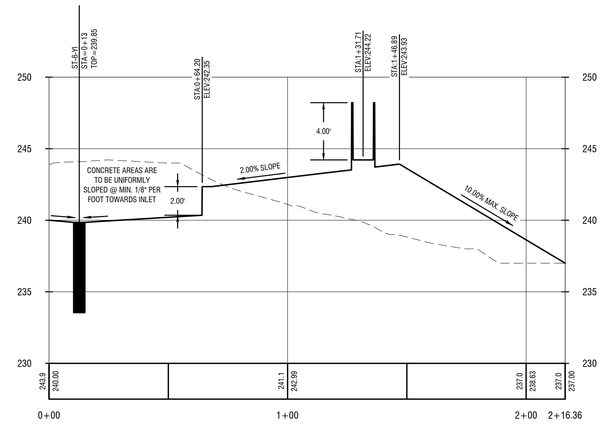
FOUNDATION EXPANSION SECTION C-C PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'

BASKETBALL COURT SLAB
FOUNDATION EXPANSION SECTIONS

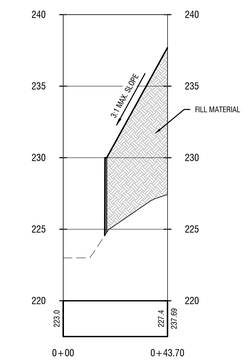
NOTES:
1. THE SECTIONS & ELEVATIONS SHOWN ABOVE ARE NOT INDICATIVE OF THE ENTIRE SLAB EXPANSION, AND ARE ONLY INTENDED TO BE USED AS A GUIDE FOR CONSTRUCTING THE PROPOSED SLAB TO THE APPROPRIATE ELEVATIONS. THE PROPOSED SLAB ELEVATIONS DEPEND ENTIRELY ON THE EXISTING SLAB ELEVATIONS (SEE NOTE #2).
2. THE EXPANSION SLAB PORTIONS SHALL BEGIN AT GRADE & TIE IN FLUSH, HAVING A SMOOTH TRANSITION, W/ THE EXISTING CONCRETE SLAB EDGE.
3. THE EXISTING SLAB ELEVATION SHALL REMAIN CONSTANT THROUGH THE EDGE OF THE EXPANDED SLAB.
4. ACTUAL WIDTH OF THE EXPANSION SLAB WILL VARY AS NEEDED TO ACHIEVE THE OVERALL SLAB DIMENSIONS AS SHOWN ON THE PLANS.
5. CONTRACTOR SHALL GRADE AWAY FROM THE PROPOSED SLAB EDGE AS NEEDED TO PROVIDE NO GREATER THAN A 5:1 SLOPE, ENSURING POSITIVE DRAINAGE AWAY FROM AND/OR AROUND THE BASKETBALL COURT AREA.
6. SEE STRUCTURAL PLANS FOR INFORMATION ON SLAB FOUNDATION DESIGN.



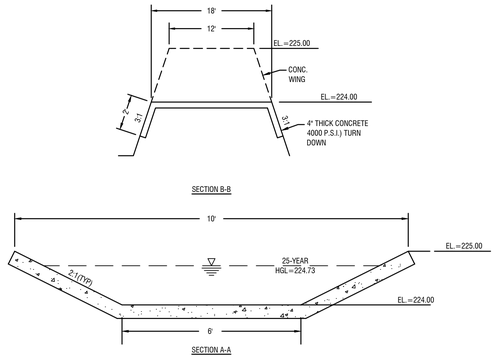
RETAINING WALL SECTION A-A PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



AMPHITHEATER SEATING TYPICAL SECTION PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



RETAINING WALL SECTION B-B PROFILE
VERT. SCALE: 1"=5'
HORIZ SCALE: 1"=30'



DETENTION AREA #2 EMERGENCY OVERFLOW STRUCTURE
NTS

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

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DATE: 4/2/2025

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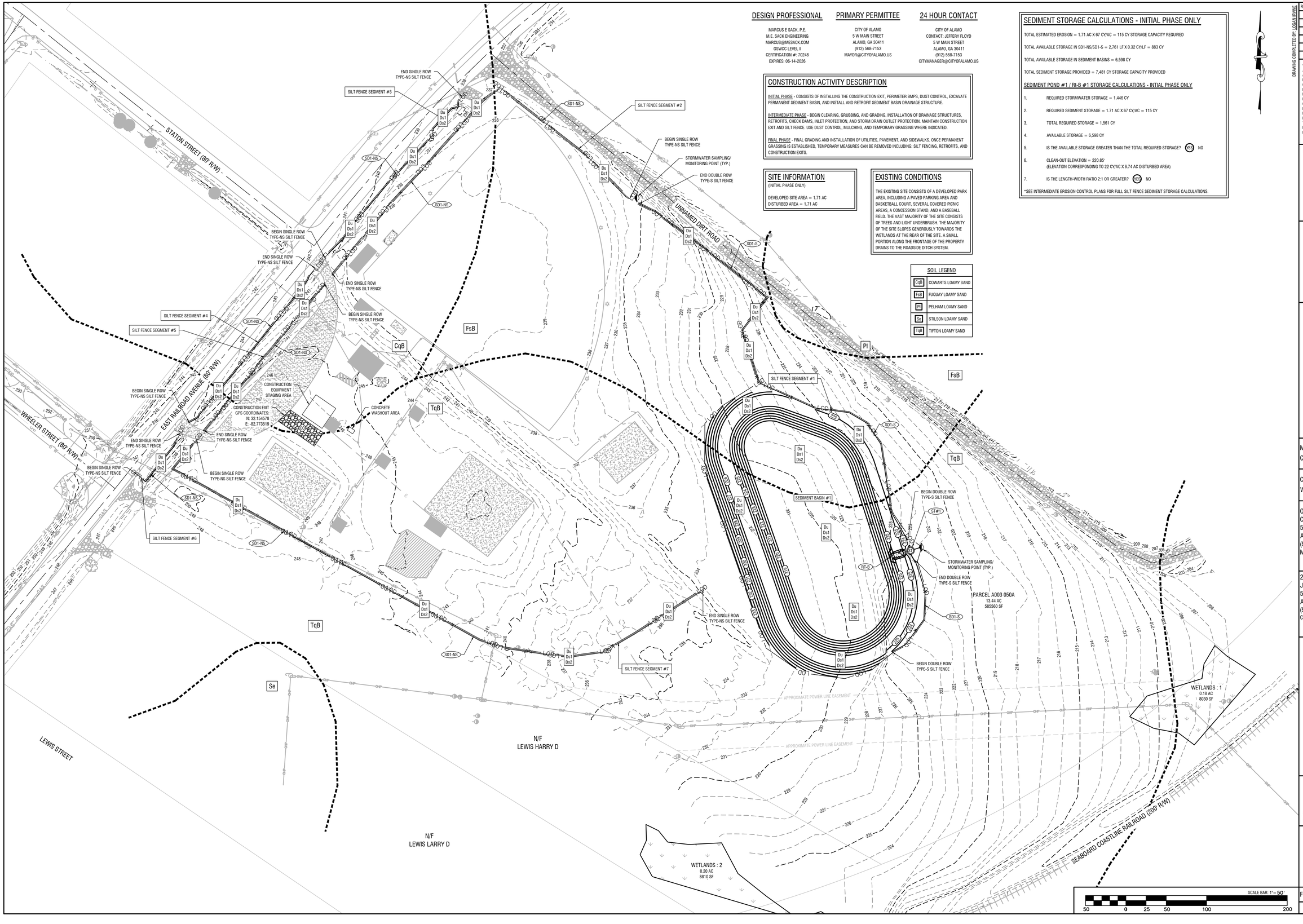
EAST SIDE PARK REMODEL

DRAINAGE PROFILES

C304

FILE NO: 2023-66

PLOT DATE: April 2, 2025



SCALE BAR: 1"= 50'
50 0 25 50 100 200

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CONSTRUCTION ACTIVITY DESCRIPTION

INITIAL PHASE - CONSISTS OF INSTALLING THE CONSTRUCTION EXIT, PERIMETER BMPS, DUST CONTROL, EXCAVATE PERMANENT SEDIMENT BASIN, AND INSTALL AND RETROFIT SEDIMENT BASIN DRAINAGE STRUCTURE.

INTERMEDIATE PHASE - BEGIN CLEARING, GRUBBING, AND GRADING. INSTALLATION OF DRAINAGE STRUCTURES, RETROFITS, CHECK DAMS, INLET PROTECTION, AND STORM DRAIN OUTLET PROTECTION. MAINTAIN CONSTRUCTION EXIT AND SILT FENCE. USE DUST CONTROL, MULCHING, AND TEMPORARY GRASSING WHERE INDICATED.

FINAL PHASE - FINAL GRADING AND INSTALLATION OF UTILITIES, PAVEMENT, AND SIDEWALKS. ONCE PERMANENT GRASSING IS ESTABLISHED, TEMPORARY MEASURES CAN BE REMOVED INCLUDING: SILT FENCING, RETROFITS, AND CONSTRUCTION EXITS.

SITE INFORMATION

(INITIAL PHASE ONLY)

DEVELOPED SITE AREA = 1.71 AC
DISTURBED AREA = 1.71 AC

EXISTING CONDITIONS

THE EXISTING SITE CONSISTS OF A DEVELOPED PARK AREA, INCLUDING A PAVED PARKING AREA AND BASKETBALL COURT, SEVERAL COVERED PICNIC AREAS, A CONCESSION STAND, AND A BASEBALL FIELD. THE VAST MAJORITY OF THE SITE CONSISTS OF TREES AND LIGHT UNDERBRUSH. THE MAJORITY OF THE SITE SLOPES GENEROUSLY TOWARDS THE WETLANDS AT THE REAR OF THE SITE. A SMALL PORTION ALONG THE FRONTAGE OF THE PROPERTY DRAINS TO THE ROADSIDE DITCH SYSTEM.

SOIL LEGEND

CqB	COWARTS LOAMY SAND
FsB	FUQUAY LOAMY SAND
Pl	PELHAM LOAMY SAND
St	STILSON LOAMY SAND
TqB	TIFTON LOAMY SAND

SEDIMENT STORAGE CALCULATIONS - INITIAL PHASE ONLY

TOTAL ESTIMATED EROSION = 1.71 AC X 67 CY/AC = 115 CY STORAGE CAPACITY REQUIRED

TOTAL AVAILABLE STORAGE IN SD1-NS/SD1-S = 2,761 LF X 0.32 CY/LF = 883 CY

TOTAL AVAILABLE STORAGE IN SEDIMENT BASINS = 6,598 CY

TOTAL SEDIMENT STORAGE PROVIDED = 7,481 CY STORAGE CAPACITY PROVIDED

SEDIMENT POND #1 / RI-B #1 STORAGE CALCULATIONS - INTIAL PHASE ONLY

1. REQUIRED STORMWATER STORAGE = 1,446 CY
2. REQUIRED SEDIMENT STORAGE = 1.71 AC X 67 CY/AC = 115 CY
3. TOTAL REQUIRED STORAGE = 1,561 CY
4. AVAILABLE STORAGE = 6,598 CY
5. IS THE AVAILABLE STORAGE GREATER THAN THE TOTAL REQUIRED STORAGE? (YES) NO
6. CLEAN-OUT ELEVATION = 220.85' (ELEVATION CORRESPONDING TO 22 CY/AC X 6.74 AC DISTURBED AREA)
7. IS THE LENGTH-WIDTH RATIO 2:1 OR GREATER? (YES) NO

*SEE INTERMEDIATE EROSION CONTROL PLANS FOR FULL SILT FENCE SEDIMENT STORAGE CALCULATIONS.

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
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EAST SIDE PARK REMODEL

INITIAL EROSION CONTROL PLAN

C400

FILE NO: 2023-66

PLOT DATE: April 2, 2025

STORM OUTLET PROTECTION

La - LENGTH OF APRON
Do - OUTLET PIPE DIA.
W1 - WIDTH AT END OF PIPE
W2 - DOWNSTREAM WIDTH
d50 - AVE. STONE SIZE
dMAX - MAX. STONE SIZE
D - STONE DEPTH

STRUCTURE NO.	Do	La	W1	W2	d50	D	FLOW RATE (CFS)	VELOCITY (FPS)
ST #1	10'	10'	20'	30'	6"	13.5"	14.44	2.7
ST #2	18"	10'	3'	13'	6"	13.5"	0.31	1.8
ST #3	18"	10'	3'	13'	6"	13.5"	0.63	2.0
ST#4	24"	10'	4'	14'	6"	13.5"	18.00	5.7

DESIGN PROFESSIONAL

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FINAL PHASE - FINAL GRADING AND INSTALLATION OF UTILITIES, PAVEMENT, AND SIDEWALKS. ONCE PERMANENT GRASSING IS ESTABLISHED, TEMPORARY MEASURES CAN BE REMOVED INCLUDING: SILT FENCING, RETROFITS, AND CONSTRUCTION EXITS.

SITE INFORMATION

DEVELOPED SITE AREA = 8.86 AC
DISTURBED AREA = 8.86 AC

EXISTING CONDITIONS

THE EXISTING SITE CONSISTS OF A DEVELOPED PARK AREA, INCLUDING A PAVED PARKING AREA AND BASKETBALL COURT, SEVERAL COVERED PICNIC AREAS, A CONCESSION STAND, AND A BASEBALL FIELD. THE VAST MAJORITY OF THE SITE CONSISTS OF TREES AND LIGHT UNDERBRUSH. THE MAJORITY OF THE SITE SLOPES GENEROUSLY TOWARDS THE WETLANDS AT THE REAR OF THE SITE. A SMALL PORTION ALONG THE FRONTAGE OF THE PROPERTY DRAINS TO THE ROADSIDE DITCH SYSTEM.

SEDIMENT STORAGE CALCULATIONS

TOTAL ESTIMATED EROSION = 8.86 AC X 67 CY/AC = 594 CY STORAGE CAPACITY REQUIRED

TOTAL AVAILABLE STORAGE IN SD1-NS/SD1-S = 2,761 LF X 0.32 CY/LF = 883 CY

TOTAL AVAILABLE STORAGE IN SEDIMENT BASINS = 6,598 CY

TOTAL SEDIMENT STORAGE PROVIDED = 7,481 CY STORAGE CAPACITY PROVIDED

SEDIMENT POND #1 / RI-B #1 STORAGE CALCULATIONS

1. REQUIRED STORMWATER STORAGE = 1,446 CY
2. REQUIRED SEDIMENT STORAGE = 6.74 AC X 67 CY/AC = 452 CY
3. TOTAL REQUIRED STORAGE = 1,898 CY
4. AVAILABLE STORAGE = 6,598 CY
5. IS THE AVAILABLE STORAGE GREATER THAN THE TOTAL REQUIRED STORAGE? (YES) NO
6. CLEAN-OUT ELEVATION = 220.85'
   (ELEVATION CORRESPONDING TO 22 CY/AC X 6.74 AC DISTURBED AREA)
7. IS THE LENGTH-WIDTH RATIO 2:1 OR GREATER? (YES) NO

SILT FENCE STORAGE CALCULATIONS

NOTE: SILT FENCING IS ONLY TO BE UTILIZED FOR SEDIMENT STORAGE WHERE THE DRAINAGE AREA DOES NOT EXCEED 0.25 AC FOR EVERY 100 LF OF SILT FENCING.

SEDIMENT STORAGE PROVIDED IN SILT FENCE SEGMENT #1 = 198 CY
MAX. DRAINAGE AREA = 618 LF X (0.25 AC / 100 LF) = 1.55 AC
REQUIRED SEDIMENT STORAGE = 0.26 AC X 67 CY/AC = 18 CY
AVAILABLE STORAGE = 618 LF X 0.32 CY/LF = 198 CY

SEDIMENT STORAGE PROVIDED IN SILT FENCE SEGMENT #2 = 167 CY
MAX. DRAINAGE AREA = 523 LF X (0.25 AC / 100 LF) = 1.31 AC
REQUIRED SEDIMENT STORAGE = 0.35 AC X 67 CY/AC = 24 CY
AVAILABLE STORAGE = 523 LF X 0.32 CY/LF = 167 CY

SEDIMENT STORAGE PROVIDED IN SILT FENCE SEGMENT #3 = 85 CY
MAX. DRAINAGE AREA = 267 LF X (0.25 AC / 100 LF) = 0.67 AC
REQUIRED SEDIMENT STORAGE = 0.07 AC X 67 CY/AC = 5 CY
AVAILABLE STORAGE = 267 LF X 0.32 CY/LF = 85 CY

SEDIMENT STORAGE PROVIDED IN SILT FENCE SEGMENT #4 = 79 CY
MAX. DRAINAGE AREA = 248 LF X (0.25 AC / 100 LF) = 0.62 AC
REQUIRED SEDIMENT STORAGE = 0.07 AC X 67 CY/AC = 5 CY
AVAILABLE STORAGE = 248 LF X 0.32 CY/LF = 79 CY

SEDIMENT STORAGE PROVIDED IN SILT FENCE SEGMENT #5 = 75 CY
MAX. DRAINAGE AREA = 235 LF X (0.25 AC / 100 LF) = 0.59 AC
REQUIRED SEDIMENT STORAGE = 0.36 AC X 67 CY/AC = 24 CY
AVAILABLE STORAGE = 235 LF X 0.32 CY/LF = 75 CY

SEDIMENT STORAGE PROVIDED IN SILT FENCE SEGMENT #6 = 28 CY
MAX. DRAINAGE AREA = 86 LF X (0.25 AC / 100 LF) = 0.23 AC
REQUIRED SEDIMENT STORAGE = 0.02 AC X 67 CY/AC = 2 CY
AVAILABLE STORAGE = 86 LF X 0.32 CY/LF = 28 CY

SEDIMENT STORAGE PROVIDED IN SILT FENCE SEGMENT #7 = 251 CY
MAX. DRAINAGE AREA = 784 LF X (0.25 AC / 100 LF) = 1.96 AC
REQUIRED SEDIMENT STORAGE = 67 CY/AC X 0.98 AC = 66 CY
AVAILABLE STORAGE = 784 LF X 0.32 CY/LF = 251 CY

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
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EAST SIDE PARK REMODEL

INTERMEDIATE EROSION CONTROL PLAN

C401

FILE NO: 2023-66

PLOT DATE: April 2, 2025

SCALE BAR: 1"= 50'
50 0 25 50 100 200

DESIGN PROFESSIONAL

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CONSTRUCTION ACTIVITY DESCRIPTION

INITIAL PHASE - CONSISTS OF INSTALLING THE CONSTRUCTION EXIT, PERIMETER BMPS, DUST CONTROL, EXCAVATE PERMANENT SEDIMENT BASIN, AND INSTALL AND RETROFIT SEDIMENT BASIN DRAINAGE STRUCTURE.

INTERMEDIATE PHASE - BEGIN CLEARING, GRUBBING, AND GRADING. INSTALLATION OF DRAINAGE STRUCTURES, RETROFITS, CHECK DAMS, INLET PROTECTION, AND STORM DRAIN OUTLET PROTECTION. MAINTAIN CONSTRUCTION EXIT AND SILT FENCE. USE DUST CONTROL, MULCHING, AND TEMPORARY GRASSING WHERE INDICATED.

FINAL PHASE - FINAL GRADING AND INSTALLATION OF UTILITIES, PAVEMENT, AND SIDEWALKS. ONCE PERMANENT GRASSING IS ESTABLISHED, TEMPORARY MEASURES CAN BE REMOVED INCLUDING: SILT FENCING, RETROFITS, AND CONSTRUCTION EXITS.

SITE INFORMATION

DEVELOPED SITE AREA = 8.86 AC
DISTURBED AREA = 8.86 AC

EXISTING CONDITIONS

THE EXISTING SITE CONSISTS OF A DEVELOPED PARK AREA, INCLUDING A PAVED PARKING AREA AND BASKETBALL COURT, SEVERAL COVERED PICNIC AREAS, A CONCESSION STAND, AND A BASEBALL FIELD. THE VAST MAJORITY OF THE SITE CONSISTS OF TREES AND LIGHT UNDERBRUSH. THE MAJORITY OF THE SITE SLOPES GENEROUSLY TOWARDS THE WETLANDS AT THE REAR OF THE SITE. A SMALL PORTION ALONG THE FRONTAGE OF THE PROPERTY DRAINS TO THE ROADSIDE DITCH SYSTEM.

STATON STREET (80' R/W)
EAST RAILROAD AVENUE (60' R/W)
WHEELER STREET (80' R/W)
UNNAMED DIRT ROAD
LEWIS STREET
SEABOARD COASTLINE RAILROAD (200' R/W)

SILT FENCE SEGMENT #1
SILT FENCE SEGMENT #2
SILT FENCE SEGMENT #3
SILT FENCE SEGMENT #4
SILT FENCE SEGMENT #5
SILT FENCE SEGMENT #6
SILT FENCE SEGMENT #7

BEGIN SINGLE ROW TYPE-NS SILT FENCE
END SINGLE ROW TYPE-NS SILT FENCE
BEGIN DOUBLE ROW TYPE-S SILT FENCE
END DOUBLE ROW TYPE-S SILT FENCE

ST#1
ST#2
ST#3
ST#4
STORMWATER SAMPLING/ MONITORING POINT (TYP.)

SEDIMENT BASIN #1
RT-8

CONSTRUCTION EQUIPMENT STAGING AREA
CONCRETE WASHOUT AREA
CONSTRUCTION EXIT GPS COORDINATES:
N: 32.154578
E: -82.773519

FLOOR DRAINS TO BE TEMPORARILY CAPPED UNTIL AMPHITHEATER CONSTRUCTION IS COMPLETED.

PARCEL A003 050A
13.44 AC
585560 SF

WETLANDS : 1
0.18 AC
8030 SF

WETLANDS : 2
0.20 AC
8810 SF

APPROXIMATE POWER LINE EASEMENT

N/F
LEWIS HARRY D

N/F
LEWIS LARRY D

SCALE BAR: 1"= 50'
50 0 25 50 100 200

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
MARCUS E. SACK
GSWCC LEVEL II # 70248
EXPIRES: 06/14/2026
MARCUS@MESACK.COM

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GEORGIA REGISTERED No. 36559 PROFESSIONAL ENGINEER MARCUS E. SACK

DATE: 4/2/2025

ME SACK ENGINEERING

MUNICIPALITY:
CITY OF ALAMO

COUNTY:
WHEELER COUNTY

OWNER:
CITY OF ALAMO
5 WEST MAIN STREET
ALAMO, GA 30411
(912) 568-7153
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EAST SIDE PARK REMODEL

FINAL EROSION CONTROL PLAN

C402

FILE NO: 2023-66

PLOT DATE: April 2, 2025

NPDES PERMIT REQUIREMENTS GAR 100001

4. INSPECTIONS.

A. PERMITTEE REQUIREMENTS.

(1). EACH DAY WHEN ANY TYPE OF CONSTRUCTION ACTIVITY HAS TAKEN PLACE AT A PRIMARY PERMITTEE'S SITE, CERTIFIED PERSONNEL PROVIDED BY THE PRIMARY PERMITTEE SHALL INSPECT: (A) ALL AREAS AT THE PRIMARY PERMITTEE'S SITE WHERE PETROLEUM PRODUCTS ARE STORED, USED, OR HANDLED FOR SPILLS AND LEAKS FROM VEHICLES AND EQUIPMENT AND (B) ALL LOCATIONS AT THE PRIMARY PERMITTEE'S SITE WHERE VEHICLES ENTER OR EXIT THE SITE FOR EVIDENCE OF OFF-SITE SEDIMENT TRACKING. THESE INSPECTIONS MUST BE CONDUCTED UNTIL A NOTICE OF TERMINATION IS SUBMITTED.

(2). MEASURE AND RECORD RAINFALL WITHIN DISTURBED AREAS OF THE SITE THAT HAVE NOT FINAL STABILIZATION ONCE EVERY 24 HOURS EXCEPT ANY NON-WORKING SATURDAY, NON-WORKING SUNDAY AND NON-WORKING FEDERAL HOLIDAY. THE DATA COLLECTED FOR THE PURPOSE OF COMPLIANCE WITH THIS PERMIT SHALL BE REPRESENTATIVE OF THE MONITORED ACTIVITY. MEASUREMENT OF RAINFALL MAY BE SUSPENDED IF ALL AREAS OF THE SITE HAVE UNDERGONE FINAL STABILIZATION OR ESTABLISHED A CROP OF ANNUAL VEGETATION AND A SEEDING OF TARGET PERENNIALS APPROPRIATE FOR THE REGION.

(3). CERTIFIED PERSONNEL (PROVIDED BY THE PRIMARY PERMITTEE) SHALL INSPECT THE FOLLOWING AT LEAST ONCE EVERY SEVEN (7) CALENDAR DAYS AND WITHIN 24 HOURS OF THE END OF A STORM THAT IS 0.5 INCHES RAINFALL OR GREATER (UNLESS SUCH STORM ENDS AFTER 5:00 PM ON ANY FRIDAY OR ON ANY NON-WORKING SATURDAY, NON-WORKING SUNDAY OR ANY NON-WORKING FEDERAL HOLIDAY IN WHICH CASE THE INSPECTION SHALL BE COMPLETED BY THE END OF THE NEXT BUSINESS DAY AND/OR WORKING DAY, WHICHEVER OCCURS FIRST): (A) DISTURBED AREAS OF THE PRIMARY PERMITTEE'S CONSTRUCTION SITE; (B) AREAS USED BY THE PRIMARY PERMITTEE FOR STORAGE OF MATERIALS THAT ARE EXPOSED TO PRECIPITATION; AND (C) STRUCTURAL CONTROL MEASURES, EROSION AND SEDIMENT CONTROL MEASURES IDENTIFIED IN THE PLAN APPLICABLE TO THE PRIMARY PERMITTEE'S SITE SHALL BE OBSERVED TO ENSURE THAT THEY ARE OPERATING CORRECTLY. WHERE DISCHARGE LOCATIONS OR POINTS ARE ACCESSIBLE, THEY SHALL BE INSPECTED TO ASCERTAIN WHETHER EROSION CONTROL MEASURES ARE EFFECTIVE IN PREVENTING SIGNIFICANT IMPACTS TO RECEIVING WATER(S). FOR AREAS OF A SITE THAT HAVE UNDERGONE FINAL STABILIZATION OR ESTABLISHED A CROP OF ANNUAL VEGETATION AND A SEEDING OF TARGET PERENNIALS APPROPRIATE FOR THE REGION, THE PERMITTEE MUST COMPLY WITH PART IV.D.4.A(4). THESE INSPECTIONS MUST BE CONDUCTED UNTIL A NOTICE OF TERMINATION IS SUBMITTED.

(4). CERTIFIED PERSONNEL (PROVIDED BY THE PRIMARY PERMITTEE) SHALL INSPECT AT LEAST ONCE PER MONTH DURING THE TERM OF THIS PERMIT (I.E., UNTIL A NOTICE OF TERMINATION HAS BEEN SUBMITTED) THE AREAS OF THE SITE THAT HAVE UNDERGONE FINAL STABILIZATION OR ESTABLISHED A CROP OF ANNUAL VEGETATION AND A SEEDING OF TARGET PERENNIALS APPROPRIATE FOR THE REGION. THESE AREAS SHALL BE INSPECTED FOR EVIDENCE OF, OR THE POTENTIAL FOR, POLLUTANTS ENTERING THE DRAINAGE SYSTEM AND THE RECEIVING WATER(S). EROSION AND SEDIMENT CONTROL MEASURES IDENTIFIED IN THE PLAN SHALL BE OBSERVED TO ENSURE THAT THEY ARE OPERATING CORRECTLY. WHERE DISCHARGE LOCATIONS OR POINTS ARE ACCESSIBLE, THEY SHALL BE INSPECTED TO ASCERTAIN WHETHER EROSION CONTROL MEASURES ARE EFFECTIVE IN PREVENTING SIGNIFICANT IMPACTS TO RECEIVING WATER(S).

(5). BASED ON THE RESULTS OF EACH INSPECTION, THE SITE DESCRIPTION AND THE POLLUTION PREVENTION AND CONTROL MEASURES IDENTIFIED IN THE EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN, THE PLAN SHALL BE REVISED AS APPROPRIATE NOT LATER THAN SEVEN (7) CALENDAR DAYS FOLLOWING EACH INSPECTION. IMPLEMENTATION OF SUCH CHANGES SHALL BE MADE AS SOON AS PRACTICAL BUT IN NO CASE LATER THAN SEVEN (7) CALENDAR DAYS FOLLOWING EACH INSPECTION.

(6). A REPORT OF EACH INSPECTION THAT INDICATES THE NAME(S) OF CERTIFIED PERSONNEL MAKING EACH INSPECTION, THE DATE(S) OF EACH INSPECTION, CONSTRUCTION PHASE (I.E., INITIAL, INTERMEDIATE OR FINAL), MAJOR OBSERVATIONS RELATING TO THE IMPLEMENTATION OF THE EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN, AND ACTIONS TAKEN IN ACCORDANCE WITH PART IV.D.4.A.(5). OF THE PERMIT SHALL BE MADE AND RETAINED AT THE SITE OR BE READILY AVAILABLE AT A DESIGNATED ALTERNATE LOCATION UNTIL THE ENTIRE SITE OR THAT PORTION OF A CONSTRUCTION PROJECT THAT HAS BEEN PHASED HAS UNDERGONE FINAL STABILIZATION AND A NOTICE OF TERMINATION IS SUBMITTED TO EPD. SUCH REPORTS SHALL BE READILY AVAILABLE BY END OF THE SECOND BUSINESS DAY AND/OR WORKING DAY AND SHALL IDENTIFY ALL INCIDENTS OF BEST MANAGEMENT PRACTICES THAT HAVE NOT BEEN PROPERLY INSTALLED AND/OR MAINTAINED AS DESCRIBED IN THE PLAN. WHERE THE REPORT DOES NOT IDENTIFY ANY INCIDENTS, THE INSPECTION REPORT SHALL CONTAIN A CERTIFICATION THAT THE BEST MANAGEMENT PRACTICES ARE IN COMPLIANCE WITH THE EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN. THE REPORT SHALL BE SIGNED IN ACCORDANCE WITH PAR V.G.2. OF THIS PERMIT.

5. MAINTENANCE. THE PLAN SHALL INCLUDE A DESCRIPTION OF PROCEDURES TO ENSURE THE TIMELY MAINTENANCE OF VEGETATION, EROSION AND SEDIMENT CONTROL MEASURES AND OTHER PROTECTIVE MEASURES IDENTIFIED IN THE SITE PLAN.

6. SAMPLING REQUIREMENTS. THIS PERMIT REQUIRES THE MONITORING OF NEPHELOMETRIC TURBIDITY IN RECEIVING WATER(S) OR OUTFALLS IN ACCORDANCE WITH THIS PERMIT. THIS PARAGRAPH SHALL NOT APPLY TO ANY LAND DISTURBANCE ASSOCIATED WITH THE CONSTRUCTION OF SINGLE-FAMILY HOMES WHICH ARE NOT PART OF A SUBDIVISION OR PLANNED COMMON DEVELOPMENT UNLESS FIVE (5) ACRES OR MORE WILL BE DISTURBED. THE FOLLOWING PROCEDURES CONSTITUTE EPD'S GUIDELINES FOR SAMPLING TURBIDITY.

A. *SAMPLING REQUIREMENTS* SHALL INCLUDE THE FOLLOWING:

(1). A USGS TOPOGRAPHIC MAP, A TOPOGRAPHIC MAP OR A DRAWING (REFERRED TO AS A TOPOGRAPHIC MAP) THAT IS A SCALE EQUAL TO OR MORE DETAILED THAN A 1:24000 MAP SHOWING THE LOCATION OF THE SITE OR THE STAND ALONE CONSTRUCTION; (A) THE LOCATION OF ALL PERENNIAL AND INTERMITTENT STREAMS AND OTHER WATER BODIES AS SHOWN ON A USGS TOPOGRAPHIC MAP, AND ALL OTHER PERENNIAL AND INTERMITTENT STREAMS AND OTHER WATER BODIES LOCATED DURING MANDATORY FIELD VERIFICATION, INTO WHICH THE STORM WATER IS DISCHARGED AND (B) THE RECEIVING WATER AND/OR OUTFALL SAMPLING LOCATIONS. WHEN THE PERMITTEE HAS CHOSEN TO USE A USGS TOPOGRAPHIC MAP AND THE RECEIVING WATER(S) IS NOT SHOWN ON THE USGS TOPOGRAPHIC MAP, THE LOCATION OF THE RECEIVING WATER(S) MUST BE HAND-DRAWN ON THE USGS TOPOGRAPHIC MAP FROM WHERE THE STORM WATER(S) ENTERS THE RECEIVING WATER(S) TO THE POINT WHERE THE RECEIVING WATER(S) COMBINES WITH THE FIRST BLUE LINE STREAM SHOWN ON THE USGS TOPOGRAPHIC MAP;

(2). A WRITTEN NARRATIVE OF SITE SPECIFIC ANALYTICAL METHODS USED TO COLLECT, HANDLE AND ANALYZE THE SAMPLES INCLUDING QUALITY CONTROL/QUALITY ASSURANCE PROCEDURES. THIS NARRATIVE MUST INCLUDE PRECISE SAMPLING METHODOLOGY FOR EACH SAMPLING LOCATION;

(3). WHEN THE PERMITTEE HAS DETERMINED THAT SOME OR ALL OUTFALLS WILL BE SAMPLED, A RATIONALE MUST BE INCLUDED ON THE PLAN FOR THE NTU LIMIT(S) SELECTED FROM APPENDIX B. THIS RATIONALE MUST INCLUDE THE SIZE OF THE CONSTRUCTION SITE, THE CALCULATION OF THE SIZE OF THE SURFACE WATER DRAINAGE AREA, AND THE TYPE OF RECEIVING WATER(S) (I.E., TROUT STREAM OR SUPPORTING WARM WATER FISHERIES); AND

(4). ANY ADDITIONAL INFORMATION EPD DETERMINES NECESSARY TO BE PART OF THE PLAN. EPD WILL PROVIDE WRITTEN NOTICE TO THE PERMITTEE OF THE INFORMATION NECESSARY AND THE TIME LINE FOR SUBMITTAL.

B. *SAMPLE TYPE.* ALL SAMPLING SHALL BE COLLECTED BY "GRAB SAMPLES" AND THE ANALYSIS OF THESE SAMPLES MUST BE CONDUCTED IN ACCORDANCE WITH METHODOLOGY AND TEST PROCEDURES ESTABLISHED BY 40 CFR PART 136 (UNLESS OTHER TEST PROCEDURES HAVE BEEN APPROVED); THE GUIDANCE DOCUMENT TITLED "NPDES STORM WATER SAMPLING GUIDANCE DOCUMENT, EPA 833-B-92-001" AND GUIDANCE DOCUMENTS THAT MAY BE PREPARED BY THE EPD.

(1). SAMPLE CONTAINERS SHOULD NOT BE LABELED PRIOR TO COLLECTING THE SAMPLES.

(2). SAMPLES SHOULD BE WELL MIXED BEFORE TRANSFERRING TO A SECONDARY CONTAINER.

(3). LARGE MOUTH, WELL CLEANED AND RINSED GLASS OR PLASTIC JARS SHOULD BE USED FOR COLLECTING SAMPLES. THE JARS SHOULD BE CLEANED THOROUGHLY TO AVOID CONTAMINATION.

(4). MANUAL, AUTOMATIC OR RISING STAGE SAMPLING MAY BE UTILIZED. SAMPLES REQUIRED BY THIS PERMIT SHOULD BE ANALYZED IMMEDIATELY, BUT IN NO CASE LATER THAN 48 HOURS AFTER COLLECTION. HOWEVER, SAMPLES FROM AUTOMATIC SAMPLERS MUST BE COLLECTING NO LATER THAN THE NEXT BUSINESS DAY AFTER THEIR ACCUMULATION, UNLESS FLOW THROUGH AUTOMATED ANALYSIS IS UTILIZED. IF AUTOMATIC SAMPLING IS UTILIZED AND THE AUTOMATIC SAMPLER IS NOT ACTIVATED DURING THE QUALIFYING EVENT, THE PERMITTEE MUST UTILIZE MANUAL SAMPLING OR RISING STAGE SAMPLING DURING THE NEXT QUALIFYING EVENT. DILUTION OF SAMPLES IS NOT REQUIRED. SAMPLES MAY BE ANALYZED DIRECTLY WITH A PROPERLY CALIBRATED TURBIDIMETER. SAMPLES ARE NOT REQUIRED TO BE COOLED.

(5). SAMPLING AND ANALYSIS OF THE RECEIVING WATER(S) OR OUTFALLS BEYOND THE MINIMUM FREQUENCY STATED IN THIS PERMIT MUST BE REPORTED TO EPD AS SPECIFIED IN PART IV.E.

C. *SAMPLING POINTS.*

(1). FOR CONSTRUCTION ACTIVITIES THE PRIMARY PERMITTEE MUST SAMPLE ALL RECEIVING WATER(S), OR ALL OUTFALL(S), OR A COMBINATION OF RECEIVING WATER(S) AND OUTFALL(S). SAMPLES TAKEN FOR THE PURPOSE OF COMPLIANCE WITH THIS PERMIT SHALL BE REPRESENTATIVE OF THE MONITORED ACTIVITY AND REPRESENTATION OF THE WATER QUALITY OF THE RECEIVING WATER(S) AND/OR THE STORM WATER OUTFALLS USING THE FOLLOWING MINIMUM GUIDELINES:

(A). THE UPSTREAM SAMPLE FOR EACH RECEIVING WATER(S) MUST BE TAKEN IMMEDIATELY UPSTREAM OF THE CONFLUENCE OF THE FIRST STORM WATER DISCHARGE FROM THE PERMITTEE ACTIVITY (I.E., THE DISCHARGE FARTHEST UPSTREAM AT THE SITE) BUT DOWNSTREAM OF ANY OTHER STORM WATER DISCHARGES NOT ASSOCIATED WITH THE PERMITTED ACTIVITY. WHERE APPROPRIATE, SEVERAL UPSTREAM SAMPLES FROM ACROSS THE RECEIVING WATER(S) MAY NEED TO BE TAKEN AND THE ARITHMETIC AVERAGE OF THE TURBIDITY OF THESE SAMPLES USED FOR THE UPSTREAM TURBIDITY VALUE.

(B). THE DOWNSTREAM SAMPLE FOR EACH RECEIVING WATER(S) MUST BE TAKEN DOWNSTREAM OF THE CONFLUENCE OF THE LAST STORM WATER DISCHARGE FROM THE PERMITTED ACTIVITY (I.E., THE DISCHARGE FARTHEST DOWNSTREAM AT THE SITE) BUT UPSTREAM OF ANY OTHER STORM WATER DISCHARGE NOT ASSOCIATED WITH THE PERMITTED ACTIVITY. WHERE APPROPRIATE, SEVERAL DOWNSTREAM SAMPLES FROM ACROSS THE RECEIVING WATER(S) MAY NEED TO BE TAKEN AND THE ARITHMETIC AVERAGE OF THE TURBIDITY OF THESE SAMPLES USED FOR THE DOWNSTREAM TURBIDITY VALUE.

(C). IDEALLY THE SAMPLES SHOULD BE TAKEN FROM THE HORIZONTAL AND VERTICAL CENTER OF THE RECEIVING WATER(S) OR THE STORM WATER OUTFALL CHANNEL(S).

(D). CARE SHOULD BE TAKEN TO AVOID STIRRING THE BOTTOM SEDIMENTS IN THE RECEIVING WATER(S) OR IN THE OUTFALL STORM WATER CHANNEL.

(E). THE SAMPLING CONTAINER SHOULD BE HELD SO THAT THE OPENING FACES UPSTREAM.

(F). THE SAMPLES SHOULD BE KEPT FREE FROM FLOATING DEBRIS.

(G). PERMITTEES DO NOT HAVE TO SAMPLE SHEETFLOW THAT FLOWS ONTO UNDISTURBED NATURAL AREAS OR AREAS STABILIZED BY THE PROJECT. FOR PURPOSES OF THIS SECTION, STABILIZED SHALL MEAN, FOR UNPAVED AREAS AND AREAS NOT COVERED BY PERMANENT STRUCTURES AND AREAS LOCATED OUTSIDE THE WASTE DISPOSAL LIMITS OF A LANDFILL CELL THAT HAS BEEN CERTIFIED BY EPD FOR WASTE DISPOSAL, 100% OF THE SOIL SURFACE IS UNIFORMLY COVERED IN PERMANENT VEGETATION WITH A DENSITY OF 70% OR GREATER, OR LANDSCAPED ACCORDING TO THE PLAN (UNIFORMLY COVERED WITH LANDSCAPING MATERIALS IN PLANNED LANDSCAPED AREAS), OR EQUIVALENT PERMANENT STABILIZATION MEASURES AS DEFINED IN THE MANUAL (EXCLUDING A CROP OF ANNUAL VEGETATION AND A SEEDING OF TARGET PERENNIALS APPROPRIATE TO THE REGION).

(H). ALL SAMPLING PURSUANT TO THIS PERMIT MUST BE DONE IN SUCH A WAY (INCLUDING GENERALLY ACCEPTED SAMPLING METHODS, LOCATIONS, TIMING, AND FREQUENCY) AS TO ACCURATELY REFLECT WHETHER STORM WATER RUNOFF FROM THE CONSTRUCTION SITE IS IN COMPLIANCE WITH THE STANDARD SET FORTH IN PARTS III.D.3. OR III.D.4., WHICHEVER IS APPLICABLE.

D. *SAMPLING FREQUENCY.*

(1). THE PRIMARY PERMITTEE MUST SAMPLE IN ACCORDANCE WITH THE PLAN AT LEAST ONCE FOR EACH RAINFALL EVENT DESCRIBED BELOW. FOR A QUALIFYING EVENT, THE PERMITTEE SHALL SAMPLE AT THE BEGINNING OF ANY STORM WATER DISCHARGE TO A MONITORED RECEIVING WATER AND/OR FROM A MONITORED OUTFALL LOCATION WITHIN FORTY-FIVE (45) MINUTES OR AS SOON AS POSSIBLE.

(2). HOWEVER, WHERE MANUAL AND AUTOMATIC SAMPLING ARE IMPOSSIBLE (AS DEFINED IN THIS PERMIT), OR ARE BEYOND THE PERMITTEE'S CONTROL, THE PERMITTEE SHALL TAKE SAMPLES AS SOON AS POSSIBLE, BUT IN NO CASE MORE THAN TWELVE (12) HOURS AFTER THE BEGINNING OF THE STORM WATER DISCHARGE.

(3). SAMPLING BY THE PERMITTEE SHALL OCCUR FOR THE FOLLOWING QUALIFYING EVENTS:

(A). FOR EACH AREA OF THE SITE THAT DISCHARGES TO A RECEIVING WATER OR FROM AN OUTFALL, THE FIRST RAIN EVENT THAT REACHES OR EXCEEDS 0.5 INCH WITH A STORM WATER DISCHARGE THAT OCCURS DURING NORMAL BUSINESS HOURS AS DEFINED IN THIS PERMIT AFTER ALL CLEARING AND GRUBBING OPERATIONS HAVE BEEN COMPLETED, BUT PRIOR TO COMPLETION OF MASS GRADING OPERATIONS, IN THE DRAINAGE AREA OF THE LOCATION SELECTE3D AS THE SAMPLING LOCATION;

(B). IN ADDITION TO (A) ABOVE, FOR EACH AREA OF THE SITE THAT DISCHARGES TO A RECEIVING WATER OR FROM AN OUTFALL, THE FIRST RAIN EVENT THAT REACHES OR EXCEEDS 0.5 INCH WITH A STORM WATER DISCHARGE THAT OCCURS DURING NORMAL BUSINESS HOURS AS DEFINED BY THIS PERMIT EITHER 90 DAYS AFTER THE FIRST SAMPLING EVENT OR AFTER ALL MASS GRADING OPERATIONS HAVE BEEN COMPLETED, BUT PRIOR TO SUBMITTAL OF A NOT, IN THE DRAINAGE AREA OF THE LOCATION SELECTED AS THE SAMPLING LOCATION, WHICHEVER COMES FIRST;

(C). AT THE TIME OF SAMPLING PERFORMED PURSUANT TO (A) AND (B) ABOVE, IF BMPS IN ANY AREA OF THE SITE THAT DISCHARGES TO A RECEIVING WATER OR FROM AN OUTFALL ARE NOT PROPERLY DESIGNED, INSTALLED AND MAINTAINED, CORRECTIVE ACTION SHALL BE DEFINED AND IMPLEMENTED WITHIN TWO (2) BUSINESS DAYS, AND TURBIDITY SAMPLES SHALL BE TAKEN FROM DISCHARGES FROM THAT AREA OF THE SITE FOR EACH SUBSEQUENT RAIN EVENT THAT REACHES OR EXCEEDS 0.5 INCH DURING NORMAL BUSINESS HOURS* UNTIL THE SELECTED TURBIDITY STANDARD IS ATTAINED, OR UNTIL POST-STORM EVENT INSPECTIONS DETERMINE THAT BMPS ARE PROPERLY DESIGNED, INSTALLED AND MAINTAINED;

(D). WHERE SAMPLING PURSUANT TO (A), (B) OR (C) ABOVE IS REQUIRED BUT NOT POSSIBLE (OR NOT REQUIRED BECAUSE THERE WAS NO DISCHARGE), THE PERMITTEE, IN ACCORDANCE WITH PART IV.D.4.A.(6), MUST INCLUDE A WRITTEN JUSTIFICATION IN THE INSPECTION REPORT OF WHY SAMPLING WAS NOT PERFORMED. PROVIDING THIS JUSTIFICATION DOES NOT RELIEVE THE PERMITTEE OF ANY SUBSEQUENT SAMPLING OBLIGATIONS UNDER (A), (B) OR (C) ABOVE; AND

(E). EXISTING CONSTRUCTION ACTIVITIES, I.E., THOSE THAT ARE OCCURRING ON OR BEFORE THE EFFECTIVE DATE OF THIS PERMIT, THAT HAVE MET THE SAMPLING REQUIRED BY (A) ABOVE SHALL SAMPLE IN ACCORDANCE WITH (B). THOSE EXISTING CONSTRUCTION ACTIVITIES THAT HAVE MET THE SAMPLING REQUIRED BY (B) ABOVE SHALL NOT BE REQUIRED TO CONDUCT ADDITIONAL SAMPLING OTHER THAN AS REQUIRED BY (C) ABOVE.

*NOTE THAT THE PERMITTEE MAY CHOOSE TO MEET THE REQUIREMENTS OF (A) AND (B) ABOVE BY COLLECTING TURBIDITY SAMPLES FROM ANY RAIN EVENT THAT REACHES OR EXCEEDS 0.5 INCH AND ALLOWS FOR SAMPLING AT ANY TIME OF THE DAY OR WEEK.

7. NON-STORM WATER DISCHARGES. EXCEPT FOR FLOWS FROM FIRE FIGHTING ACTIVITIES, SOURCES OF NON-STORM WATER LISTED IN PART III.A.2. OF THIS PERMIT THAT ARE COMBINED WITH STORM WATER DISCHARGES ASSOCIATED WITH CONSTRUCTION ACTIVITY MUST BE IDENTIFIED IN THE PLAN. THE PLAN SHALL IDENTIFY AND ENSURE THE IMPLEMENTATION OF APPROPRIATE POLLUTION PREVENTION MEASURES FOR THE NON-STORM WATER COMPONENT(S) OF THE DISCHARGE.

E. REPORTING.

1. THE APPLICABLE PERMITTEES ARE REQUIRED TO SUBMIT THE SAMPLING RESULTS TO THE EPD AT THE ADDRESS SHOWN IN PART II.C. BY THE FIFTEENTH DAY OF THE MONTH FOLLOWING THE REPORTING PERIOD. REPORTING PERIODS ARE MONTHS DURING WHICH SAMPLES ARE TAKEN IN ACCORDANCE WITH THIS PERMIT. SAMPLING RESULTS SHALL BE IN A CLEARLY LEGIBLE FORMAT. UPON WRITTEN NOTIFICATION, EPD MAY REQUIRE THE APPLICABLE PERMITTEE TO SUBMIT THE SAMPLING RESULTS ON A MORE FREQUENT BASIS. SAMPLING AND ANALYSIS OF ANY STORM WATER DISCHARGE(S) OR THE RECEIVING WATER(S) BEYOND THE MINIMUM FREQUENCY STATED IN THIS PERMIT MUST BE REPORTED IN A SIMILAR MANNER TO THE EPD. THE SAMPLING REPORTS MUST BE SIGNED IN ACCORDANCE WITH PART V.G.2. SAMPLING REPORTS MUST BE SUBMITTED TO EPD USING THE ELECTRONIC SUBMITTAL SERVICE PROVIDED BY EPD. SAMPLING REPORTS MUST BE SUBMITTED TO EPD UNTIL SUCH TIME AS A NOT IS SUBMITTED IN ACCORDANCE WITH PART VI.

2. ALL SAMPLING REPORTS SHALL INCLUDE THE FOLLOWING INFORMATION:

a. THE RAINFALL AMOUNT, DATE, EXACT PLACE AND TIME OF SAMPLING OR MEASUREMENTS;

b. THE NAME(S) OF THE CERTIFIED PERSONNEL WHO PERFORMED THE SAMPLING AND MEASUREMENTS;

c. THE DATE(S) ANALYSES WERE PERFORMED;

d. THE TIME(S) ANALYSES WERE INITIATED;

e. THE NAME(S) OF THE CERTIFIED PERSONNEL WHO PERFORMED THE ANALYSES;

f. REFERENCES AND WRITTEN PROCEDURES, WHEN AVAILABLE, FOR THE ANALYTICAL TECHNIQUES OR METHODS USED;

g. THE RESULTS OF SUCH ANALYSES, INCLUDING THE BENCH SHEETS, INSTRUMENT READOUTS, COMPUTER DISKS OR TAPES, ETC., USED TO DETERMINE THESE RESULTS;

h. RESULTS WHICH EXCEED 1000 NTU SHALL BE REPORTED AS "EXCEEDS 1000 NTU;" AND

i. CERTIFICATION STATEMENT THAT SAMPLING WAS CONDUCTED AS PER THE PLAN.

3. ALL WRITTEN CORRESPONDENCE REQUIRED BY THIS PERMIT SHALL BE SUBMITTED BY RETURN RECEIPT CERTIFIED MAIL (OR SIMILAR SERVICE) TO THE APPROPRIATE DISTRICT OFFICE OF THE EPD ACCORDING TO THE SCHEDULE IN APPENDIX A OF THIS PERMIT. THE PERMITTEE SHALL RETAIN A COPY OF THE PROOF OF SUBMITTAL AT THE CONSTRUCTION SITE OR THE PROOF OF SUBMITTAL SHALL BE READILY AVAILABLE AT A DESIGNATED LOCATION FROM COMMENCEMENT OF CONSTRUCTION UNTIL SUCH TIME AS A NOT IS SUBMITTED IN ACCORDANCE WITH PART VI.

F. RETENTION OF RECORDS.

1. THE PRIMARY PERMITTEE SHALL RETAIN THE FOLLOWING RECORDS AT THE CONSTRUCTION SITE OR THE RECORDS SHALL BE READILY AVAILABLE AT A DESIGNATED ALTERNATE LOCATION FROM COMMENCEMENT OF CONSTRUCTION UNTIL SUCH TIME AS A NOT IS SUBMITTED IN ACCORDANCE WITH PART VI;

a. A COPY OF ALL NOTICES OF INTENT SUBMITTED TO EPD;

b. A COPY OF THE EROSION, SEDIMENTATION AND POLLUTION CONTROL PLAN REQUIRED BY THIS PERMIT;

c. THE DESIGN PROFESSIONAL'S REPORT OF THE RESULTS OF THE INSPECTION CONDUCTED IN ACCORDANCE WITH PART IV.A.5. OF THIS PERMIT;

d. A COPY OF ALL SAMPLING INFORMATION, RESULTS, AND REPORTS REQUIRED BY THIS PERMIT;

e. A COPY OF ALL INSPECTION REPORTS GENERATED IN ACCORDANCE WITH PART IV.D.4.A. OF THIS PERMIT;

f. A COPY OF ALL VIOLATION SUMMARIES AND VIOLATION SUMMARY REPORTS GENERATED IN ACCORDANCE WITH PART III.D.2. OF THIS PERMIT;

g. DAILY RAINFALL INFORMATION COLLECTED IN ACCORDANCE WITH PART IV.D.4.A.(2). OF THIS PERMIT.

2. COPIES OF ALL NOTICES OF INTENT, NOTICES OF TERMINATION, INSPECTION REPORTS, SAMPLING REPORTS (INCLUDING ALL CALIBRATION AND MAINTENANCE RECORDS AND ALL ORIGINAL STRIP CHART RECORDINGS FOR CONTINUOUS MONITORING INSTRUMENTATION) OR OTHER REPORTS REQUESTED BY THE EPD, EROSION, SEDIMENTATION AND POLLUTION CONTROL PLANS, RECORDS OF ALL DATA USED TO COMPLETE THE NOTICE OF INTENT TO BE COVERED BY THIS PERMIT AND ALL OTHER RECORDS REQUIRED BY THIS PERMIT SHALL BE RETAINED BY THE PERMITTEE WHO EITHER PRODUCED OR USED IT FOR A PERIOD OF AT LEAST THREE YEARS FROM THE DATE THAT THE NOT IS SUBMITTED IN ACCORDANCE WITH PART VI. OF THIS PERMIT. THESE RECORDS MUST BE MAINTAINED AT THE PERMITTEE'S PRIMARY PLACE OF BUSINESS OR AT A DESIGNATED ALTERNATIVE LOCATION ONCE THE CONSTRUCTION ACTIVITY HAS CEASED AT THE PERMITTED SITE. THIS PERIOD MAY BE EXTENDED BY REQUEST OF THE EPD AT ANY TIME UPON WRITTEN NOTIFICATION TO THE PERMITTEE.

PART V. STANDARD PERMIT CONDITIONS

A. DUTY TO COMPLY.

1. EACH PERMITTEE MUST COMPLY WITH ALL APPLICABLE CONDITIONS OF THIS PERMIT. ANY PERMIT NONCOMPLIANCE CONSTITUTES A VIOLATION OF THE GEORGIA WATER QUALITY CONTROL ACT (O.C.G.A. §§12-5-20, ET SEQ.) AND IS GROUNDS FOR ENFORCEMENT ACTION; FOR PERMIT TERMINATION; OR FOR DENIAL OF A PERMIT RENEWAL APPLICATION. FAILURE OF A PRIMARY PERMITTEE TO COMPLY WITH ANY APPLICABLE TERM OR CONDITION OF THIS PERMIT SHALL NOT RELIEVE ANY OTHER PRIMARY PERMITTEE FROM COMPLIANCE WITH THEIR APPLICABLE TERMS AND CONDITIONS OF THIS PERMIT.

2. EACH PERMITTEE MUST DOCUMENT IN THEIR RECORDS ANY AND ALL KNOWN VIOLATIONS OF THIS PERMIT AT HIS/HER SITE WITHIN SEVEN (7) DAYS OF HIS/HER KNOWLEDGE OF THE VIOLATION. A SUMMARY OF THESE VIOLATIONS MUST BE SUBMITTED TO EPD BY THE PERMITTEE AT THE ADDRESS SHOWN IN PART II.C. WITHIN FOURTEEN (14) DAYS OF HIS/HER DISCOVERY OF THE VIOLATION.

3. PENALTIES FOR VIOLATIONS OF PERMIT CONDITIONS. THE FEDERAL CLEAN WATER ACT AND THE GEORGIA WATER QUALITY CONTROL ACT (O.C.G.A. §§12-5-20, ET SEQ.) PROVIDE THAT ANY PERSON WHO FALSIFIES, TAMPERS WITH, OR KNOWINGLY RENDERS INACCURATE ANY MONITORING DEVICE OR METHOD REQUIRED TO BE MAINTAINED UNDER THIS PERMIT, MAKES ANY FALSE STATEMENT, REPRESENTATION, OR CERTIFICATION IN ANY RECORD OR OTHER DOCUMENT SUBMITTED OR REQUIRED TO BE MAINTAINED UNDER THIS PERMIT, INCLUDING MONITORING REPORTS OR REPORTS OF COMPLIANCE OR NONCOMPLIANCE SHALL, UPON CONVICTION BE PUNISHED BY A FINE OR BY IMPRISONMENT, OR BY BOTH. THE FEDERAL CLEAN WATER ACT AND THE GEORGIA WATER QUALITY CONTROL ACT ALSO PROVIDE PROCEDURES FOR IMPOSING CIVIL PENALTIES WHICH MAY BE LEVIED FOR VIOLATIONS OF THE ACTS, ANY PERMIT CONDITION OR LIMITATION ESTABLISHED PURSUANT TO THE ACTS, OR NEGLIGENTLY OR INTENTIONALLY FAILING OR REFUSING TO COMPLY WITH ANY FINAL OR EMERGENCY ORDER OF THE DIRECTOR.

B. CONTINUATION OF THE EXPIRED GENERAL PERMIT. THIS PERMIT EXPIRES ON THE DATE SHOWN ON THE COVER PAGE OF THIS PERMIT. HOWEVER, AN EXPIRED GENERAL PERMIT CONTINUES IN FORCE AND EFFECT UNTIL A NEW GENERAL PERMIT IS ISSUED, FINAL AND EFFECTIVE.

C. NEED TO HALT OR REDUCE ACTIVITY NOT A DEFENSE. IT SHALL NOT BE A DEFENSE FOR THE PERMITTEE IN AN ENFORCEMENT ACTION THAT IT WOULD HAVE BEEN NECESSARY TO HALT OR REDUCE THE PERMITTED ACTIVITY IN ORDER TO MAINTAIN COMPLIANCE WITH THE CONDITIONS OF THIS PERMIT.

D. DUTY TO MITIGATE. THE PERMITTEE SHALL TAKE ALL REASONABLE STEPS TO MINIMIZE OR PREVENT ANY DISCHARGE IN VIOLATION OF THIS PERMIT WHICH HAS A REASONABLE LIKELIHOOD OF ADVERSELY AFFECTING HUMAN HEALTH OR THE ENVIRONMENT.

E. DUTY TO PROVIDE INFORMATION. THE PERMITTEE SHALL FURNISH TO THE DIRECTOR; A STATE OR LOCAL AGENCY APPROVING SOIL EROSION AND SEDIMENTATION CONTROL PLANS, GRADING PLANS, OR STORM WATER MANAGEMENT PLANS; OR IN THE CASE OF A STORM WATER DISCHARGE ASSOCIATED WITH CONSTRUCTION ACTIVITY WHICH DISCHARGES THOUGH A MUNICIPAL SEPARATE STORM SEWER SYSTEM WITH AN NPDES PERMIT, TO THE LOCAL GOVERNMENT OPERATING THE MUNICIPAL SEPARATE STORM SEWER SYSTEM, ANY INFORMATION WHICH IS REQUESTED TO DETERMINE COMPLIANCE WITH THIS PERMIT. IN THE CASE OF INFORMATION SUBMITTED TO THE EPD SUCH INFORMATION SHALL BE CONSIDERED PUBLIC INFORMATION AND AVAILABLE UNDER THE GEORGIA OPEN RECORDS ACT.

F. OTHER INFORMATION. WHEN THE PERMITTEE BECOMES AWARE THAT HE FAILED TO SUBMIT ANY RELEVANT FACTS OR SUBMITTED INCORRECT INFORMATION IN THE NOTICE OF INTENT OR IN ANY OTHER REPORT REQUIRED TO BE SUBMITTED TO THE EPD, THE PERMITTEE SHALL PROMPTLY SUBMIT SUCH FACTS OR INFORMATION.

G. SIGNATORY REQUIREMENTS. ALL NOTICES OF INTENT, NOTICE OF TERMINATIONS, INSPECTION REPORTS, SAMPLING REPORTS OR OTHER REPORTS REQUESTED BY THE EPD SHALL BE SIGNED AS FOLLOWS:

1. ALL NOTICES OF INTENT AND NOTICES OF TERMINATION SHALL BE SIGNED AS FOLLOWS:

a. FOR A CORPORATION: BY A RESPONSIBLE CORPORATE OFFICER. FOR THE PURPOSE OF THIS PERMIT, A RESPONSIBLE CORPORATE OFFICER MEANS: (1) A PRESIDENT, SECRETARY, TREASURER, OR VICE-PRESIDENT OF THE CORPORATION IN CHARGE OF A PRINCIPAL BUSINESS FUNCTION, OR ANY OTHER PERSON WHO PERFORMS SIMILAR POLICY- OR DECISION-MAKING FUNCTIONS FOR THE CORPORATION; OR (2) THE MANAGER OF ONE OR MORE MANUFACTURING, PRODUCTION OR OPERATING FACILITIES PROVIDED THE MANAGER IS AUTHORIZED TO MAKE MANAGEMENT DECISI

GEORGIA
UNIFORM CODING SYSTEM
FOR SOIL EROSION AND SEDIMENT CONTROL PRACTICES
GEORGIA SOIL AND WATER CONSERVATION COMMISSION

STRUCTURAL PRACTICES

CODE	PRACTICE	DETAIL	MAP SYMBOL	DESCRIPTION
Cd	CHECKDAM			A small temporary barrier or dam constructed across a swale, drainage ditch or area of concentrated flow.
Ch	CHANNEL STABILIZATION			Improving, constructing or stabilizing an open channel, existing stream, or ditch.
Co	CONSTRUCTION EXIT		Co	A crushed stone pad located at the construction site exit to provide a place for removing mud from tires thereby protecting public streets.
Cr	CONSTRUCTION ROAD STABILIZATION		Cr	A roadway constructed as part of a construction plan including access roads, subdivision roads, parking areas and other on-site vehicle transportation routes.
Dc	STREAM DIVERSION CHANNEL			A temporary channel constructed to convey flow around a construction site while a permanent structure is being constructed.
Di	DIVERSION			An earth channel or dike located above, below or across a slope to divert runoff. This may be a temporary or permanent structure.
Dn1	TEMPORARY DOWNDRAIN STRUCTURE		Dn1	A flexible conduit of heavy-duty fabric or other material designed to safely conduct surface runoff down a slope. This is temporary and inexpensive.
Dn2	PERMANENT DOWNDRAIN STRUCTURE		Dn2	A paved chute, pipe, sectional conduit or similar material designed to safely conduct surface runoff down a slope.
Fr	FILTER RING			A temporary stone barrier constructed at storm drain inlets and pond outlets.
Ga	GABION			Rock filter baskets which are hand-placed into position forming soil stabilizing structures.
Gr	GRADE STABILIZATION STRUCTURE		Gr	Permanent structures installed to protect channels or waterways where otherwise the slope would be sufficient for the running water to form gullies.
Lv	LEVEL SPREADER			A structure to convert concentrated flow of water into less erosive sheet flow. This should be constructed only on undisturbed soils.
Rd	ROCK FILTER DAM			A permanent or temporary stone filter dam installed across small streams or drainageways.
Re	RETAINING WALL		Re	A wall installed to stabilize cut and fill slopes where maximum permissible slopes are not obtainable. Each situation will require special design.
Rt	RETRO FITTING		Rt	A device or structure placed in front of a permanent stormwater detention pond outlet structure to serve as a temporary sediment filter.
Sd1	SEDIMENT BARRIER		(INDICATE TYPE)	A barrier to prevent sediment from leaving the construction site. It may be sandbags, bales of straw or hay, brush, logs and poles, gravel, or a silt fence.
Sd2	INLET SEDIMENT TRAP			An impounding area created by excavating around a storm drain drop inlet. The excavated area will be filled and stabilized on completion of construction activities.
Sd3	TEMPORARY SEDIMENT BASIN		Sd3	A basin created by excavation or a dam across a waterway. The surface water runoff is temporarily stored allowing the bulk of the sediment to drop out.
Sd4	TEMPORARY SEDIMENT TRAP			A small temporary pond that drains a disturbed area so that sediment can settle out. The principle feature distinguishing a temporary sediment trap from a temporary sediment basin is the lack of a pipe or riser.
Sk	FLOATING SURFACE SKIMMER		Sk	A buoyant device that releases/drains water from the surface of sediment ponds, traps, or basins at a controlled rate of flow.
Spb	SEEP BERM		Spb	Linear control device constructed as a diversion perpendicular to the direction of runoff to enhance dissipation and infiltration, while creating multiple sedimentation chambers with the employment of intermediate dikes.

STRUCTURAL PRACTICES

CODE	PRACTICE	DETAIL	MAP SYMBOL	DESCRIPTION
Sr	TEMPORARY STREAM CROSSING		Sr	A temporary bridge or culvert-type structure protecting a stream or watercourse from damage by crossing construction equipment.
St	STORMDRAIN OUTLET PROTECTION		St	A paved or short section of riprap channel at the outlet of a storm drain system preventing erosion from the concentrated runoff.
Su	SURFACE ROUGHENING		Su	A rough soil surface with horizontal depressions on a contour or slopes left in a roughened condition after grading.
Tc	TURBIDITY CURTAIN		Tc	A floating or staked barrier installed within the water (it may also be referred to as a floating boom, silt barrier, or silt curtain).
Tp	TOPSOILING		(SAME SYMBOL AS TEMPORARY SEEDING)	The practice of stripping off the more fertile soil, storing it, then spreading it over the disturbed area after completion of construction activities.
Tr	TREE PROTECTION		(SAME TREE SYMBOL)	To protect desirable trees from injury during construction activity.
Wt	VEGETATED WATERWAY OR STORMWATER CONVEYANCE CHANNEL			Paved or vegetative water outlets for diversions, terraces, berms, dikes or similar structures.

VEGETATIVE PRACTICES

CODE	PRACTICE	DETAIL	MAP SYMBOL	DESCRIPTION
Bf	BUFFER ZONE		Bf	Strip of undisturbed original vegetation, enhanced or restored existing vegetation or the reestablishment of vegetation surrounding an area of disturbance or bordering streams.
Cs	COASTAL DUNE STABILIZATION (WITH VEGETATION)		Cs	Planting vegetation on dunes that are denuded artificially constructed, or re-nourished.
Ds1	DISTURBED AREA STABILIZATION (WITH MULCHING ONLY)		Ds1	Establishing temporary protection for disturbed areas where seedlings may not have a suitable growing season to produce an erosion retarding cover.
Ds2	DISTURBED AREA STABILIZATION (WITH TEMP SEEDING)		Ds2	Establishing a temporary vegetative cover with fast growing seedings on disturbed areas.
Ds3	DISTURBED AREA STABILIZATION (WITH PERM SEEDING)		Ds3	Establishing a permanent vegetative cover such as trees, shrubs, vines, grasses, or legumes on disturbed areas.
Ds4	DISTURBED AREA STABILIZATION (SODDING)		Ds4	A permanent vegetative cover using sods on highly erodable or critically eroded lands.
Du	DUST CONTROL ON DISTURBED AREAS		Du	Controlling surface and air movement of dust on construction site, roadways and similar sites.
Fl-Co	FLOCCULANTS AND COAGULANTS		Fl-Co	Substance formulated to assist in the solids/liquid separation of suspended particles in solution.
Sb	STREAMBANK STABILIZATION (USING PERM VEGETATION)		Sb	The use of readily available native plant materials to maintain and enhance streambanks, or to prevent, or restore and repair small streambank erosion problems.
Ss	SLOPE STABILIZATION		Ss	A protective covering used to prevent erosion and establish temporary or permanent vegetation on steep slopes, shore lines, or channels.
Tac	TACKIFIERS AND BINDERS		Tac	Substance used to anchor straw or hay mulch by causing the organic material to bind together.

GaSWCC (Amended – 2013)

Du DUST CONTROL ON DISTURBED AREAS

PURPOSE
A. To prevent surface and air movement of dust from exposed surfaces.
B. To reduce the presence of airborne substances which may be harmful or injurious to human health, welfare, or safety, or to animals or plant life.

Temporary Methods
1. Irrigation. This is generally done as an emergency treatment. Site is sprinkled with water until the surface is wet.
2. Mulching - See Ds1- Disturbed Area Stabilization (with Mulching only)
3. Vegetative Cover - See Ds2 - Disturbed Area Stabilization

Permanent Methods
1. Permanent Vegetation - See Ds3 - Disturbed Area Stabilization (with Permanent Vegetation)

MULCHING RATES FOR PERMANENT COVER

TYPE OF MULCH	RATE PER ACRE	NOTES
Dry straw	2 Tons	Free of weed seeds
Dry hay	2.5 Tons	Free of weed seeds
Wood Cellulose	500 lbs. 1000 lbs.	Slope less than 3/4:1 Slope greater than 3/4:1
Wood Pulp Fiber	500 lbs. 1000 lbs.	Slope less than 3/4:1 Slope greater than 3/4:1
Sericea Lespedeza Hay	3 Tons	Containing mature seeds
Pine Straw or Bark	3 inches thick	For bedding Not for seeding
Bituminous treated roving	See DOT specs.	Use on slopes, in ditches, or dry waterways.

1. Mulching is not required for temporary grassing.
2. Mulch shall be applied to cover 75% of the soil surface.
3. Sod does not require mulch.

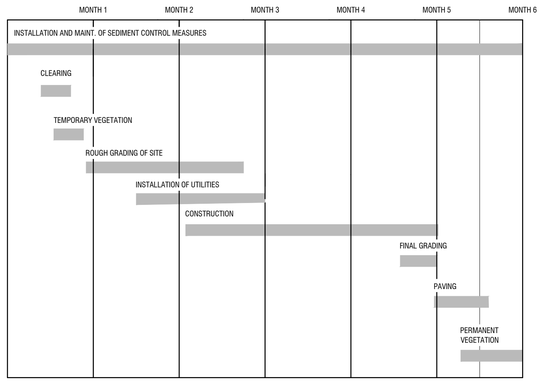
FERTILIZER REQUIREMENTS

TYPE OF SPECIES	YEAR	ANALYSIS OR EQUIVALENT	RATE	N TOP DRESSING RATE	LIME APPLICATION
Cool Season Grasses	First Second Maintenance	6 - 12 - 12 6 - 12 - 12 10 - 12 - 12	1500 lbs/ac 1000 lbs/ac 400 lbs/ac	50 - 100 lbs/ac 1*2* -- 30	2000 lbs/ac
Cool Season Grasses and Legumes	First Second Maintenance	6 - 12 - 12 0 - 10 - 10 0 - 10 - 10	1500 lbs/ac 1000 lbs/ac 400 lbs/ac	0 - 50 lbs/ac 1* -- --	2000 lbs/ac
Ground Covers	First Second Maintenance	10 - 10 - 10 10 - 10 - 10 10 - 10 - 10	1500 lbs/ac 1000 lbs/ac 400 lbs/ac	-- -- --	--
Pine Seedings	First	20 - 10 - 5	one 21-gram pallet per seeding placed in the closing hole	--	--
Shrub Leapedeza	First Maintenance	0 - 10 - 10 0 - 10 - 10	700 lbs/ac 700 lbs/ac 4*	--	--
Temporary Cover Crops Seeded Clone	First	10 - 10 - 10	500 lbs/ac	30 lbs/ac 5*	--
Warm Season Grasses	First Second Maintenance	6 - 12 - 12 6 - 12 - 12 10 - 10 - 12	1500 lbs/ac 800 lbs/ac 400 lbs/ac	50 - 100 lbs/ac 2*6* 50 - 100 lbs/ac 2*6* 30 lbs/ac	2000 lbs/ac
Warm Season Grasses and Legumes	First Second Maintenance	6 - 12 - 12 0 - 10 - 12 0 - 10 - 12	1500 lbs/ac 1000 lbs/ac 400 lbs/ac	50 lbs/ac 6*	2000 lbs/ac

1. Apply in spring following seeding.
2. Apply in split applications when high rates are used.
3. Apply in 3 split applications.
4. Apply when plants are pruned.
5. Apply to grass species only.
6. Apply when plants grow to height of 2 to 4 inches.

M.E. SACK ENGINEERING
515 NORTH MAIN ST. - P.O. BOX 649
HINESVILLE, GEORGIA 31313
912-368-5212

EAST SIDE PARK REMODEL
TENTATIVE SCHEDULING
FOR THE CITY OF ALAMO



*NO LDA'S WILL COMMENCE UNTIL AFTER THE INSTALLATION OF PERIMETER CONTROL/SEDIMENT STORAGE BMP'S PER PERMIT IV.D2.b PG30.

Ds1 DISTURBED AREA STABILIZATION (W/MULCHING ONLY)

SPECIFICATIONS
A. For temporary protection of critical areas without seeding.
This standard applies to grades or cleared areas which may be subjected to erosion for 6 months or less, where seeding may not have a suitable growing season to produce an erosion retardant cover, but which can be stabilized with a mulch cover.

Site Preparation
1. Grade, as needed and feasible, to permit the use of equipment for applying and anchoring mulch.
2. Install needed erosion control measures as required such as dikes, diversions, berms, terraces and sediment barriers.
3. As needed and feasible, loosen compact soil to a minimum depth of 3 inches.

Mulching Materials
1. Dry straw or hay - spread at a rate of 2 1/2 tons per acre.
2. Wood waste, chips, sawdust or bark - spread 2 to 3 inches deep (about 6 to 9 tons per acre).
3. Erosion control matting or netting, such as excelsior, jute, textile and plastic matting and netting - applied in accordance with manufacturers recommendations.
4. Polyethylene film - secured over banks or stockpiled soil material for temporary protection.

Applying and Anchoring Mulch
1. Apply straw or hay mulch uniformly by hand or mechanically. Anchor as appropriate and feasible. It may be pressed into the soil with a disk harrow with the disk set straight or with a special "packer disk." The disk may be smooth or serrated and should be 20 inches or more in diameter and 8 to 12 inches apart. the edges of the disk should be dull enough not to cut the mulch but press it into the soil leaving much of it in an erect position.
Straw hay mulch spread with special blower-type equipment may be anchored with emulsified asphalt (Grade AE-5 or SS-1). The asphalt emulsion must be sprayed onto the mulch as it is ejected from the machine. Use 100 gallons of water per
2. Spread wood waste uniformly on slopes that are 3:1 and flatter. No anchoring is needed.
3. Commercial matting and netting. Follow manufacturer's specification included with the material.

B. To conserve moisture and control weeds in nurseries, ornamental beds, around shrubs, and on bar areas on lawns.

Mulching Materials
Use one of the materials given below and apply at thickness indicated.

Material	Depth
1. Grain straw or grass hay	6" to 10"
2. Pine needle	4" to 6"
3. Wood waste (sawdust, bark, chips)	4" to 8"
4. Shredded residues (crops, leaves, etc.)	4" to 8"

5. Completely cover area with black polyethylene film and hold in place by placing soil on the outer edge.

When using organic mulches, apply 20-30 pounds of nitrogen in addition to the normal amount needed for plant growth to offset the tie up of N by decomposition of mulch.

Ds2 SPECIES AND PLANTING SCHEDULE

SPECIES	BROADCAST RATES (1) PER ACRE	PER 1000 Sq.Ft.	PLANTING DATES BY RESOURCE AREAS (3) J F M A M J J A S O N D	REMARKS
RYEGRASS, ANNUAL ALONE	40 lbs.	0.9 lbs.	M-L P C	227,000 SEED PER POUND. DENSE COVER. VERY COMPETITIVE AND IS NOT TO BE USED IN MIXTURES.

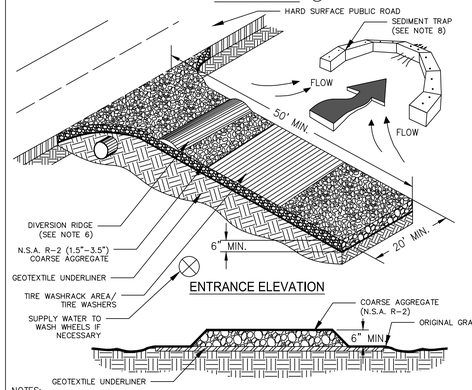
Ds3 SPECIES AND PLANTING SCHEDULE

SPECIES	BROADCAST RATES 1/ - PLS 2/ PER ACRE	PER 1000 S.F.	PLANTING DATES BY RESOURCE AREAS 3/ J F M A M J J A S O N D	SPECIFICATIONS
BERMUDA, COMMON (CYNODON DACTYLON) HULLED SEED ALONE	10 LBS.	0.2 LB.	P	1,787,000 SEED PER POUND. QUICK COVER. LOW GROWING AND SOD FORMING. FULL SUN. GOOD FOR ATHLETIC FIELDS.
WITH OTHER PERENNIALS	6 LBS.	0.1 LB.	P	
BERMUDA, COMMON (CYNODON DACTYLON) UNHULLED SEED WITH TEMPORARY COVER	10 LBS.	0.2 LB.	P	
WITH OTHER PERENNIALS	6 LBS.	0.1 LB.	P	PLANT WITH WINTER ANNUALS. PLANT WITH TALL FESCUE.
BERMUDA SPRIGS (CYNODON DACTYLON) COASTAL, COMMON, MIDLAND, OR TIFT 44	40 CU. FT. OR SOD PLUGS 3' x 3'	0.9 CU.FT.	P	A CUBIC FT. CONTAINS APPROXIMATLY 650 SPRIGS. A BUSHEL CONTAINS 1.25 C.F. OR APPROXIMATLY 800 SPRIGS.
COASTAL, COMMON, OR TIFT 44			P	
			P	SOUTHERN COASTAL PLAIN ONLY

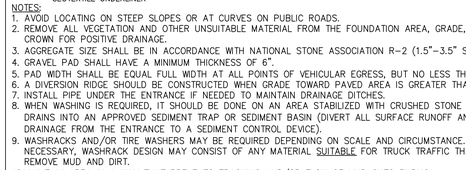
(1) BROADCAST RATES ARE IN PURE LIVE SEED (PLS)
(2) M-L REPRESENTS THE MOUNTAIN, BLUE RIDGE, AND RIDGES AND VALLEYS MLRA'S
P REPRESENTS THE SOUTHERN PIEDMONT MLRA
C REPRESENTS SOUTHERN COASTAL PLAIN, SAND HILLS, BLACK LANDS, AND ATLANTIC COAST FLATWOODS MLRA'S
(3) DARK LINES INDICATE OPTIMUM DATES, GRAY LINES INDICATE PERMISSIBLE BUT MARGINAL DATES.

CRUSHED STONE CONSTRUCTION EXIT

EXIT DIAGRAM

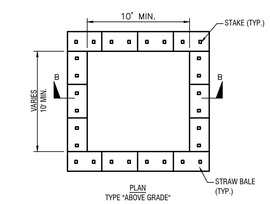


ENTRANCE ELEVATION

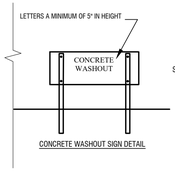


NOTES:
1. AVOID LOCATING ON STEEP SLOPES OR AT CURVES ON PUBLIC ROADS.
2. REMOVE ALL VEGETATION AND OTHER UNSUITABLE MATERIAL FROM THE FOUNDATION AREA, GRADE, AND CROWN FOR POSITIVE DRAINAGE.
3. AGGREGATE SIZE SHALL BE IN ACCORDANCE WITH NATIONAL STONE ASSOCIATION R-2 (1.5"-3.5" STONE).
4. GRAVEL PAD SHALL HAVE A MINIMUM THICKNESS OF 6".
5. PAD WIDTH SHALL BE EQUAL FULL WIDTH AT ALL POINTS OF VEHICULAR EGRESS, BUT NO LESS THAN 20'.
6. A DIVERSION RIDGE SHOULD BE CONSTRUCTED WHEN GRADE TOWARD PAVED AREA IS GREATER THAN 2%..
7. INSTALL PIPE UNDER THE ENTRANCE IF NEEDED TO MAINTAIN DRAINAGE DITCHES.
8. WHEN WASHING IS REQUIRED, IT SHOULD BE DONE ON AN AREA STABILIZED WITH CRUSHED STONE THAT DRAINS INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN (DIVERT ALL SURFACE RUNOFF AND DRAINAGE FROM THE ENTRANCE TO A SEDIMENT CONTROL DEVICE).
9. WASHRACKS AND/OR TIRE WASHERS MAY BE REQUIRED DEPENDING ON SCALE AND CIRCUMSTANCE. IF NECESSARY, WASHRACK DESIGN MAY CONSIST OF ANY MATERIAL SUITABLE FOR TRUCK TRAFFIC THAT REMOVE MUD AND DIRT.
10. MAINTAIN AREA IN A WAY THAT PREVENTS TRACKING AND/OR FLOW OF MUD ONTO PUBLIC RIGHTS-OF-WAYS. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT.

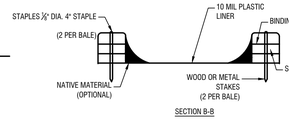
CONCRETE WASHOUT



PLAN
TYPE "ABOVE GRADE" WITH STRAW BALES



CONCRETE WASHOUT SIGN DETAIL



SECTION B-B

NOTES:
1. ACTUAL LAYOUT DETERMINED IN FIELD.
2. INSTALL CONCRETE WASHOUT SIGN (24"X24", MINIMUM) WITHIN 30' OF THE TEMPORARY CONCRETE WASHOUT FACILITY.
3. TEMPORARY WASHOUT AREA MUST BE AT LEAST 50' FROM A STORM DRAIN, CREEK BANK OR PERIMETER CONTROL.
4. CLEAN OUT CONCRETE WASHOUT AREA WHEN 50% FULL.
5. THE KEY TO FUNCTIONAL CONCRETE WASHOUTS IS WEEKLY INSPECTIONS, ROUTINE MAINTENANCE, AND REGULAR CLEAN OUT.
6. SILT FENCE SHALL BE INSTALLED AROUND PERIMETER OF CONCRETE WASHOUT AREA EXCEPT FOR THE SIDE UTILIZED FOR ACCESSING THE WASHOUT.
7. A ROCK CONSTRUCTION ENTRANCE MAY BE NECESSARY ALONG ONE SIDE OF THE WASHOUT TO PROVIDE VEHICLE ACCESS.

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
MARCUS E. SACK
GSWCC LEVEL II # 70248
EXPIRES: 06/14/2026
MARCUS@MESACK.COM

515 NORTH MAIN STREET
P.O. BOX 649
HINESVILLE, GA 31313
TEL: (912) 368-5212

GEORGIA REGISTERED PROFESSIONAL ENGINEER No. 36555 MARCUS E. SACK

DATE: 4/2/2025

ME SACK ENGINEERING 00

MUNICIPALITY:
CITY OF ALAMO

COUNTY:
WHEELER COUNTY

OWNER:
CITY OF ALAMO
5 WEST MAIN STREET
ALAMO, GA 30411
(912) 568-7153
MAYOR@CITYOFALAMO.US

24 HOUR CONTACT:
JEFFERY FLOYD
5 WEST MAIN STREET
ALAMO, GA 30411
(912) 568-7153
CITYMANAGER@CITYOFALAMO.US

EAST SIDE PARK REMODEL

EROSION CONTROL DETAILS

C501

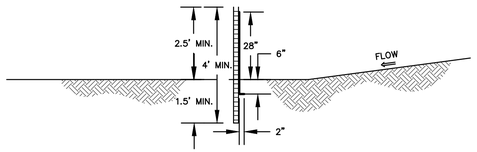
FILE NO: 2023-66

PLOT DATE: April 2, 2025

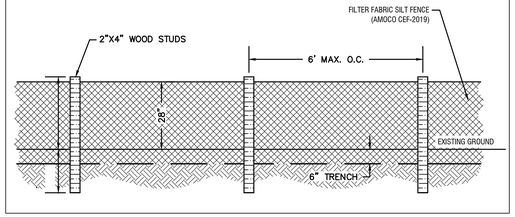
SILT FENCE - TYPE NON-SENSITIVE

Sd1-NS

SIDE VIEW

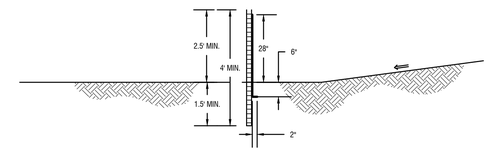


FRONT VIEW

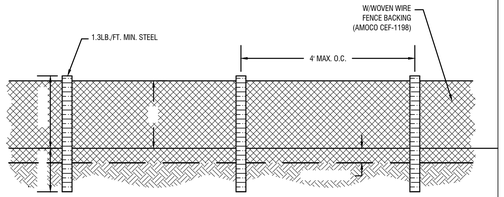


SILT FENCE - TYPE SENSITIVE

Sd1-S



SIDE VIEW

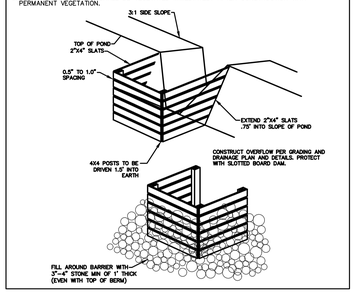


FRONT VIEW

SILT CONTROL GATE WITH SLOTTED BOARD DAM

RT-B

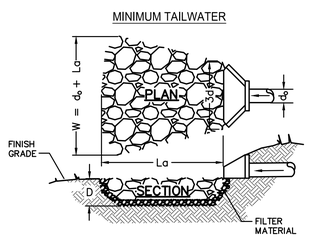
NOTES:
1. REFER TO APPROPRIATE CONSTRUCTION DETAIL FOR CONCRETE APRONS.
2. SLOTTED BOARD DAM SHALL BE INSTALLED WITH MINIMUM SIZE 4" X 4" POSTS.
3. BOARDS SHOULD HAVE A 0.5" TO 1"SPACE BETWEEN THEM AND MUST HAVE GROUND OR BOTTOM OF CONCRETE CONTACT.
4. MINIMUM SIZE 3--4" STONE FILTER SHALL BE INSTALLED AROUND THE UPSTREAM SIDE OF THE BOARD DAM.
5. POSTS FOR THE SILT CONTROL GATE SHALL BE 4: X 4" TREATED LUMBER AND FACE BOARDS SHALL BE 2" X 6" TREATED LUMBER WITH NO SPACING ALLOWED BETWEEN BOARDS.
6. AN APPROVED SILT FENCE FABRIC SHALL BE SECURELY FASTENED TO THE FRONT OF THE STRUCTURE USING STABLES (BE SURE TO HAVE SILT FENCE ON UPSTREAM SIDE OF STRUCTURE).
7. SEDIMENT SHALL BE REMOVED AND PROPERLY DISPOSED OF WHEN IT REACHES ONE-THIRD THE HEIGHT OF THE SILT GATE. FILTER FABRIC SHALL BE REPLACED WHEN DAMAGED AND/OR DETERIORATED.
8. ALL DISTURBED AREAS SHALL BE VEGETATED IMMEDIATELY AFTER CONSTRUCTION WITH PERMANENT VEGETATION.



STORM DRAIN OUTLET PROTECTION

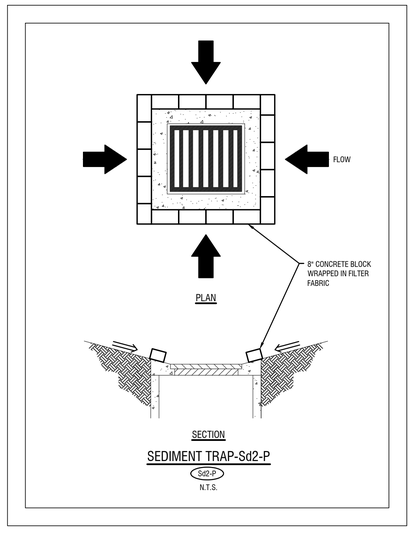
St

MINIMUM TAILWATER



NOTES:

1. REFER TO STORM DRAIN OUTLET PROTECTION CHART FOR DIMENSIONING INFORMATION.
2. d_o = DIAMETER OF OUTLET PIPE
3. d50 = MIN. STONE DIAMETER
4. d_{max} = MAX. STONE DIA. = (1.5 x d50)
5. D^{min} 1.5 TIMES THE MAXIMUM STONE DIAMETER, BUT NOT LESS THAN 6".
6. EXTEND THE RIPRAP APRON UP THE CHANNEL BANK TO THE TOP OF THE CHANNEL BANK.
7. A FILTER BLANKET OR FILTER FABRIC SHALL BE INSTALLED BETWEEN THE RIPRAP AND THE SOIL FOUNDATION. FILTER BLANKET SHALL CONSIST OF A GRADED GRAVEL LAYER OR SYNTHETIC FILTER CLOTH. FILTER BLANKET IS INTENDED TO PREVENT SOIL MOVEMENT THROUGH THE OPENINGS IN RIPRAP.
8. RIPRAP APRON AND FILTER MATERIAL SHALL BE WELL GRADED, LEVEL, AND EXTENDED TO ACHIEVE MAX. STABILITY.



SEDIMENT TRAP-Sd2-P

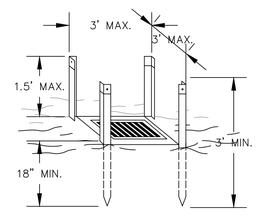
Sd2-P

N.T.S.

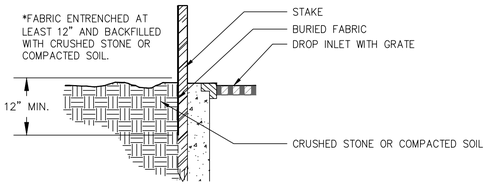
FABRIC AND SUPPORTING FRAME FOR INLET PROTECTION

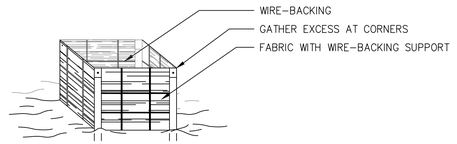
Sd2-P

STEEL FRAME AND SILT FENCE INSTALLATION



NOTES:
1. DESIGN IS FOR SLOPES NO GREATER THAN 5% (NOT DESIGNED FOR CONCENTRATED FLOWS).
2. THE STEEL POSTS SUPPORTING THE SILT FENCE MATERIAL SHOULD BE SPACED EVENLY AROUND THE PERIMETER OF THE INLET (MAXIMUM OF 3' APART).
3. THE STEEL POSTS SHOULD BE SECURELY DRIVEN AT LEAST 18" DEEP.
4. THE FABRIC SHOULD BE ENTRENCHED AT LEAST 12" AND THEN BACKFILLED WITH CRUSHED STONE OR COMPACTED SOIL.





DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
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COUNTY:
WHEELER COUNTY

OWNER:
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EAST SIDE PARK REMODEL

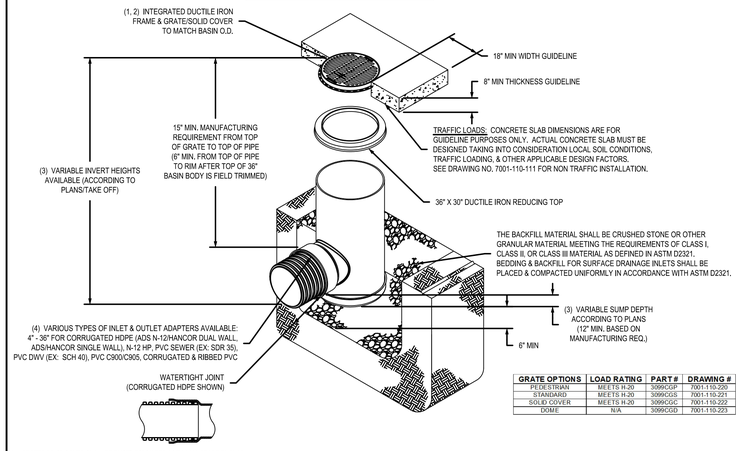
EROSION CONTROL DETAILS

C502

FILE NO: 2023-66

PLOT DATE: April 2, 2025

NYLOPLAST 36" DRAIN BASIN: 2836AG _ _X



GRATE OPTIONS	LOAD RATING	PART #	DRAWING #
PEDESTRIAN	MEETS H-20	3099CGP	7001-110-220
STANDARD	MEETS H-20	3099CGS	7001-110-221
SOLID COVER	MEETS H-20	3099CGC	7001-110-222
DOME	N/A	3099CGD	7001-110-223

1 - GRATES/SOLID COVER SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05.
2 - FRAMES & REDUCING TOP SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05.
3 - DRAIN BASIN TO BE CUSTOM MANUFACTURED ACCORDING TO PLAN DETAILS. RISERS ARE NEEDED FOR BASIN BODIES OVER 84" DUE TO SHIPPING RESTRICTIONS.
4 - DRAINAGE CONNECTION STUB JOINT TIGHTNESS SHALL CONFORM TO ASTM D3212 FOR CORRUGATED HDPE (ADS N-12/HANCOR DUAL WALL), N-12 HP, & PVC SEWER.
5 - ADAPTERS CAN BE MOUNTED ON ANY ANGLE 0° TO 360°. TO DETERMINE MINIMUM ANGLE BETWEEN ADAPTERS SEE DRAWING NO. 7001-110-012.

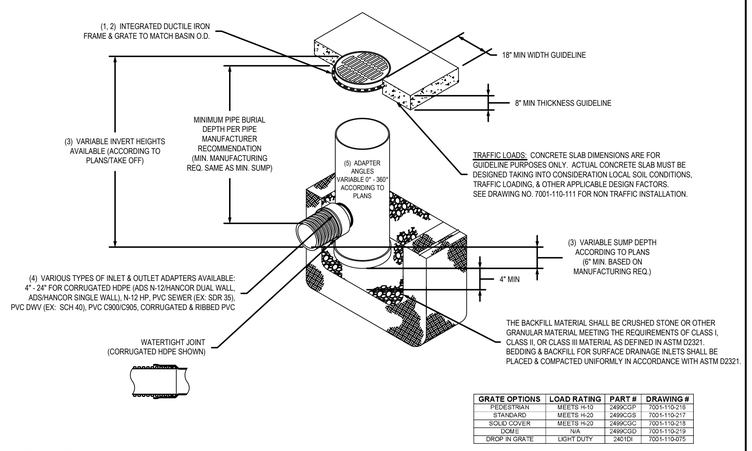
DRAWN BY	NMH	MATERIAL
DATE	9-21-18	
APPD BY	NMH	
DATE	9-21-18	
REVISED BY	NMH	PROJECT NO./NAME
DATE	10-04-21	
APPD BY	NMH	
DATE	10-04-21	
DWG SIZE	A	SCALE 1:40 SHEET 1 OF 1

ADS Nyloplast
3130 VERONA AVE
BUFORD, GA 30518
PHN (770) 932-2443
FAX (770) 932-2490
www.nyloplast-us.com

TITLE: 36" DRAIN BASIN QUICK SPEC INSTALLATION DETAIL

DWG NO. 7001-110-526 REV B

NYLOPLAST 24" DRAIN BASIN: 2824AG _ _X



GRATE OPTIONS	LOAD RATING	PART #	DRAWING #
PEDESTRIAN	MEETS H-10	2499CGP	7001-110-216
STANDARD	MEETS H-20	2499CGS	7001-110-217
SOLID COVER	MEETS H-20	2499CGC	7001-110-218
DOME	N/A	2499CGD	7001-110-219
DROP IN GRATE	LIGHT DUTY	24GTD	7001-110-075

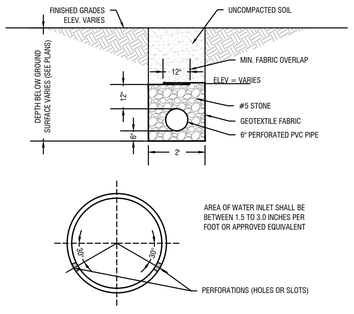
1 - GRATES/SOLID COVER SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05.
2 - FRAMES SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05.
3 - DRAIN BASIN TO BE CUSTOM MANUFACTURED ACCORDING TO PLAN DETAILS. RISERS ARE NEEDED FOR BASINS OVER 84" DUE TO SHIPPING RESTRICTIONS. SEE DRAWING NO. 7001-110-065.
4 - DRAINAGE CONNECTION STUB JOINT TIGHTNESS SHALL CONFORM TO ASTM D3212 FOR CORRUGATED HDPE (ADS N-12/HANCOR DUAL WALL), N-12 HP & PVC SEWER.
5 - ADAPTERS CAN BE MOUNTED ON ANY ANGLE 0° TO 360°. TO DETERMINE MINIMUM ANGLE BETWEEN ADAPTERS SEE DRAWING NO. 7001-110-012.

DRAWN BY	EBC	MATERIAL
DATE	04-03-06	
REVISED BY	NMH	PROJECT NO./NAME
DATE	03-14-16	
DWG SIZE	A	SCALE 1:40 SHEET 1 OF 1

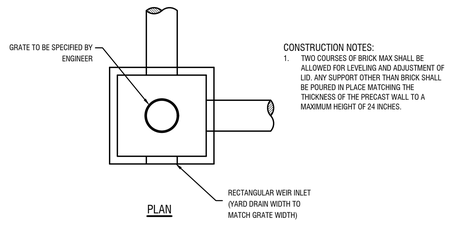
ADS Nyloplast
3130 VERONA AVE
BUFORD, GA 30518
PHN (770) 932-2443
FAX (770) 932-2490
www.nyloplast-us.com

TITLE: 24 IN DRAIN BASIN QUICK SPEC INSTALLATION DETAIL

DWG NO. 7001-110-192 REV E

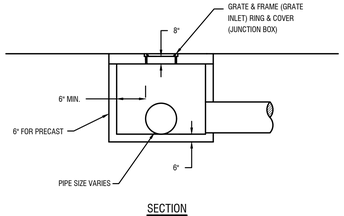


UNDERDRAIN TYPICAL DETAIL
N.T.S.



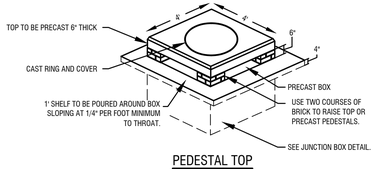
CONSTRUCTION NOTES:
1. TWO COURSES OF BRICK MAX SHALL BE ALLOWED FOR LEVELING AND ADJUSTMENT OF LID. ANY SUPPORT OTHER THAN BRICK SHALL BE POURED IN PLACE MATCHING THE THICKNESS OF THE PRECAST WALL TO A MAXIMUM HEIGHT OF 24 INCHES.

PLAN



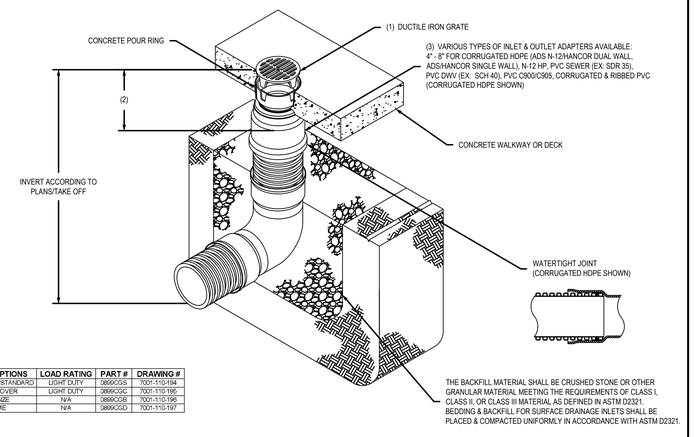
SECTION

JUNCTION BOX / GRATE INLET
N.T.S.



PEDESTAL TOP
N.T.S.

NYLOPLAST 8" INLINE DRAIN: 2708AG 0 8 N



GRATE OPTIONS	LOAD RATING	PART #	DRAWING #
PEDESTRIAN/STANDARD	LIGHT DUTY	0899CGS	7001-110-194
SOLID COVER	LIGHT DUTY	0899CGC	7001-110-195
BRONZE	N/A	0899CGB	7001-110-196
DOME	N/A	0899CGD	7001-110-197

1 - GRATES/SOLID COVERS SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05 WITH THE EXCEPTION OF THE BRONZE GRATE.
2 - FOR COMPLETE DESIGN DETAILS SEE DRAWING NO. 7003-110-000.
3 - DRAINAGE CONNECTION STUB JOINT TIGHTNESS SHALL CONFORM TO ASTM D3212 FOR CORRUGATED HDPE (ADS N-12/HANCOR DUAL WALL), & PVC SEWER.

DRAWN BY	EBC	MATERIAL
DATE	11-15-06	
REVISED BY	NMH	PROJECT NO./NAME
DATE	03-15-16	
DWG SIZE	A	SCALE 1:16 SHEET 1 OF 1

ADS Nyloplast
3130 VERONA AVE
BUFORD, GA 30518
PHN (770) 932-2443
FAX (770) 932-2490
www.nyloplast-us.com

TITLE: 8 IN INLINE DRAIN QUICK SPEC INSTALLATION DETAIL

DWG NO. 7003-110-033 REV F

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

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GEORGIA REGISTERED No. 36559 PROFESSIONAL ENGINEER MARCUS E. SACK

DATE: 4/2/2025

ME SACK ENGINEERING

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COUNTY:
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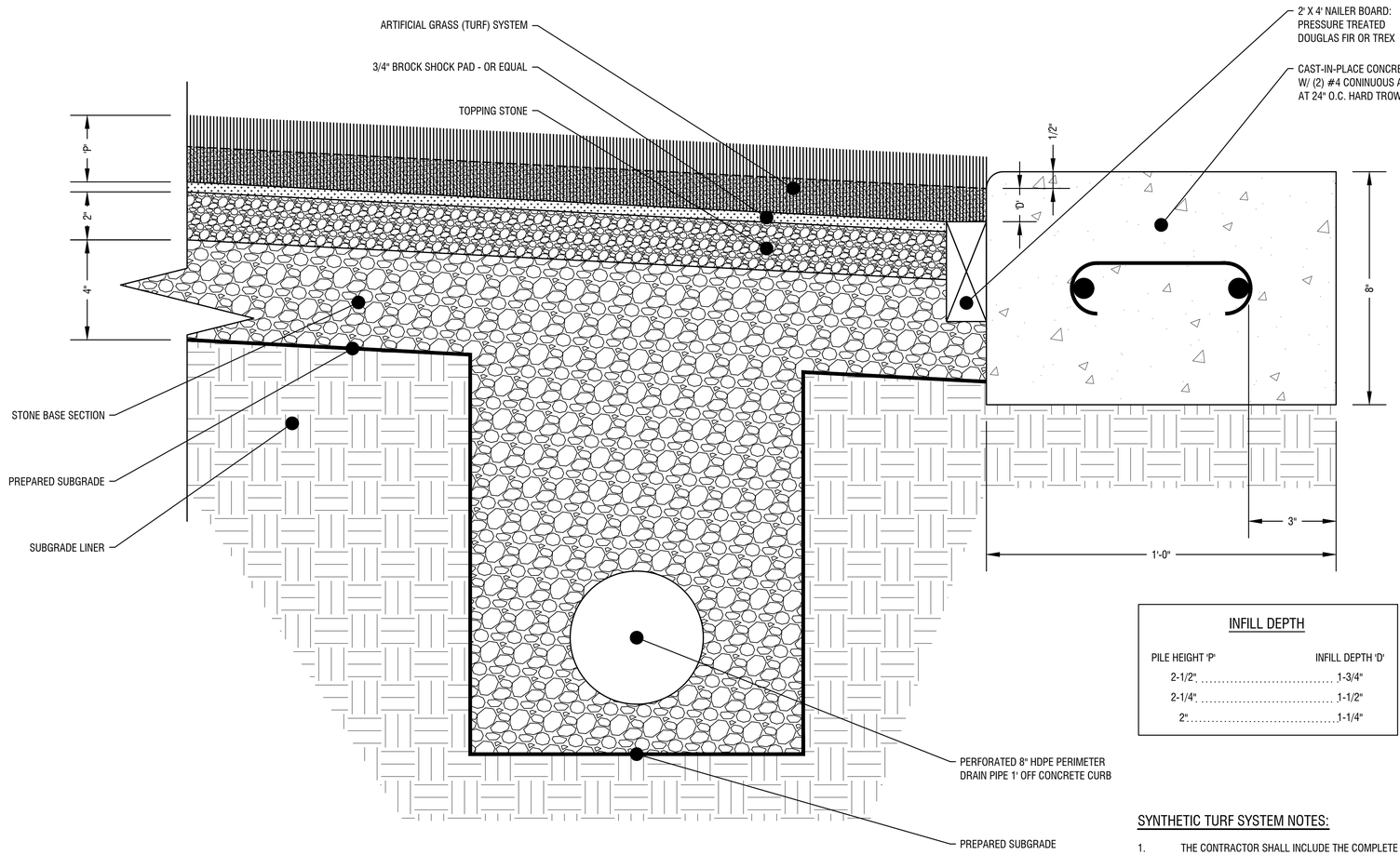
EAST SIDE PARK REMODEL

DRAINAGE DETAILS

C503

FILE NO: 2023-66

PLOT DATE: April 2, 2025



INFILL DEPTH	
PILE HEIGHT 'P'	INFILL DEPTH 'D'
2-1/2"	1-3/4"
2-1/4"	1-1/2"
2"	1-1/4"

SYNTHETIC TURF SYSTEM NOTES:

1. THE CONTRACTOR SHALL INCLUDE THE COMPLETE ARTIFICIAL GRASS SPORTS FIELD SYSTEM DESIGN IN PROJECT BID IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND PLANS.
2. THE DIMENSIONS, MATERIALS, AND COMPONENTS SHOWN IN THE DETAIL MAY VARY BASED ON THE MANUFACTURER SELECTED BY THE CONTRACTOR.
3. INSTALLATION TO BE DONE IN ACCORDANCE WITH THE PROJECT SPECIFICATION, AS WELL AS THE MANUFACTURER SPECIFICATIONS AND RECOMMENDATIONS.

TYPICAL ARTIFICIAL GRASS SPORTS FIELD - BASE SECTION, PERIMETER DRAIN, AND PERIMETER EDGING DETAIL

N.T.S.

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
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P.O. BOX 649
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GEORGIA REGISTERED No. 36559 PROFESSIONAL ENGINEER MARCUS E. SACK

DATE: 4/2/2025

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COUNTY:
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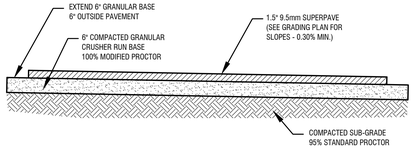
EAST SIDE PARK REMODEL

ARTIFICIAL GRASS FIELD DETAIL

C504

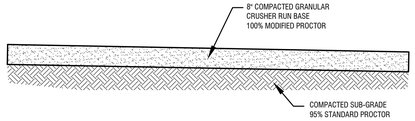
FILE NO: 2023-66

PLOT DATE: April 2, 2025



STANDARD DUTY PAVEMENT DETAIL

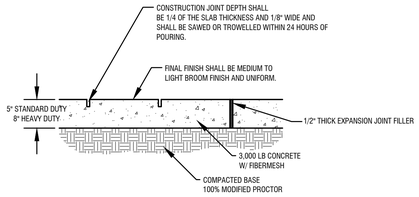
N.T.S.



ROCK ACCESS ROAD DETAIL

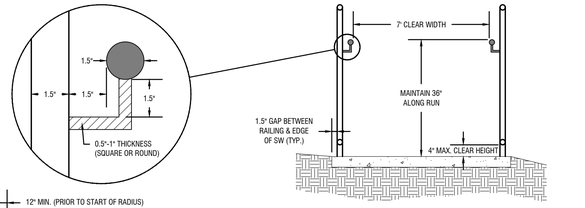
N.T.S.

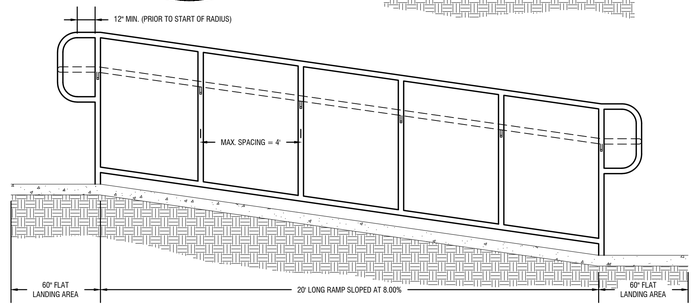
NOTES:
CONSTRUCTION JOINTS: SEE PLAN VIEW (12'-0" O.C. MAX) EXPANSION JOINTS: SEE PLAN VIEW (20'-0" O.C. MAX)



GENERAL CONCRETE DETAIL

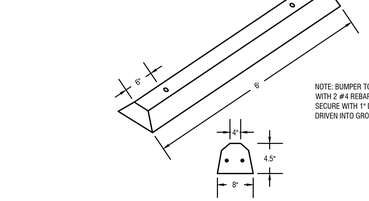
N.T.S.





ADA-ACCESSIBLE SIDEWALK RAMP & RAILING DETAIL

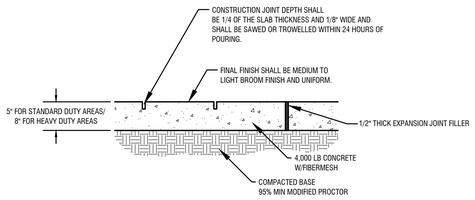
N.T.S.



NOTE: BUMPER TO BE 4500 PSI CONCRETE WITH 2 #4 REBAR REINFORCEMENT. SECURE WITH 1" DIA G.I. PIPE @ EA END DRIVEN INTO GROUND.

PARKING BUMPER

NTS

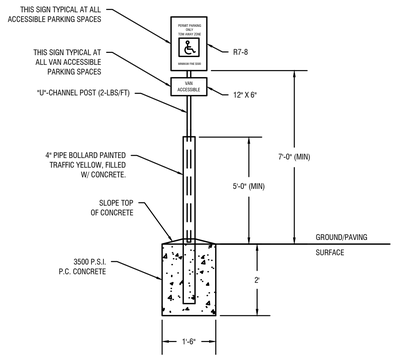


CONCRETE PAVING DETAIL

N.T.S.

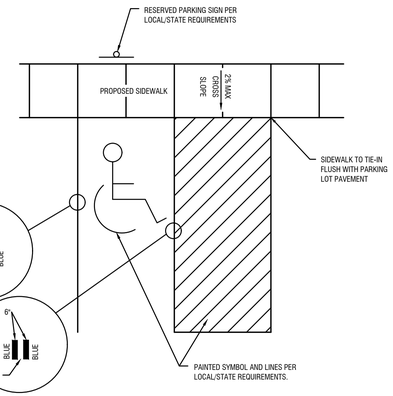
NOTES:

1. CONSTRUCTION JOINTS: 12' O.C. MAX
2. EXPANSION JOINTS: 20' O.C. MAX
3. THE CONCRETE SHOULD BE AIR ENTRAINED AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AFTER 28 DAYS OF LAB CURING PER ASTM C-31.
4. GRADED AGGREGATE BASE SHOULD CONFORM TO THE GDOT MATERIAL SPECIFICATION SECTION 815.
5. THE SELECT FILL SHOULD BE RELATIVELY CLEAN SANDS WITH PERCENT FINES LESS THAN 15%. THE FILL MATERIAL SHOULD BE COMPACTED TO A MINIMUM OF 95% OF THE SOIL'S MODIFIED PROCTOR MAXIMUM DRY DENSITY (ASTM D-1557).
6. IF SP OR SP-SM OR SM SOILS EXIST AT THE PROPOSED SUBGRADE ELEVATION EXTENDING TO A DEPTH AT LEAST 24" BELOW THE PROPOSED SUBGRADE LEVEL, THE IN-SITU SOILS CAN REPLACE THE SELECT FILL AND THE SUBGRADE SHOULD BE IMPROVED USING DENSIFICATION AS DISCUSSED IN THE EARTHWORKS SECTION OF THE GEOTECHNICAL REPORT.
7. CONCRETE JOINTS SHOULD BE SEALED PROPERLY TO AVOID INGRESS OF SURFACE WATER INTO THE SUBGRADE SOILS. PROPER SURFACE AND SUBGRADE DRAINAGE SYSTEM SHOULD BE INSTALLED TO MAINTAIN THE GROUNDWATER AT LEAST 2' BELOW THE TOP OF THE SUBGRADE.
8. SOME SUBGRADE SOIL UNDERCUTTING AND BACKFILLING WITH SUITABLE STRUCTURAL FILL WILL BE REQUIRED IF UNSTABLE SUBGRADE SOILS ARE ENCOUNTERED DURING SUBGRADE PREPARATION.



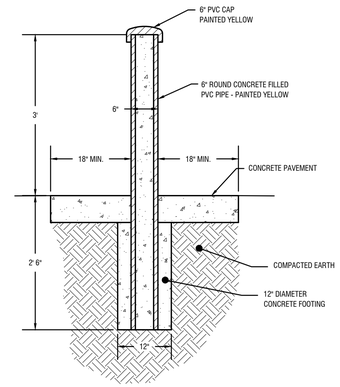
ACCESSIBLE PARKING SIGN

N.T.S.



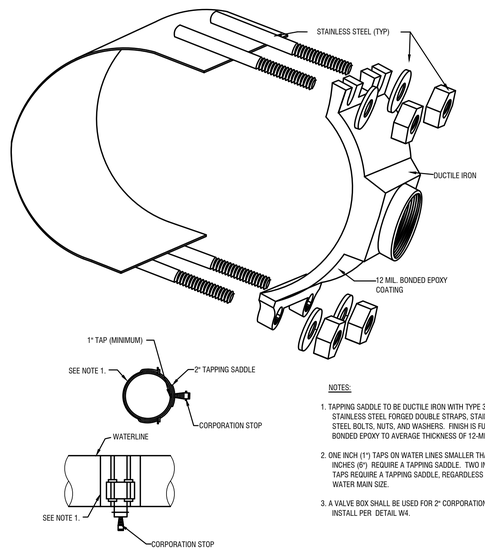
HANDICAP PARKING AND RAMP DETAIL

N.T.S.



PIPE BOLLARD DETAIL

N.T.S.



NOTES:

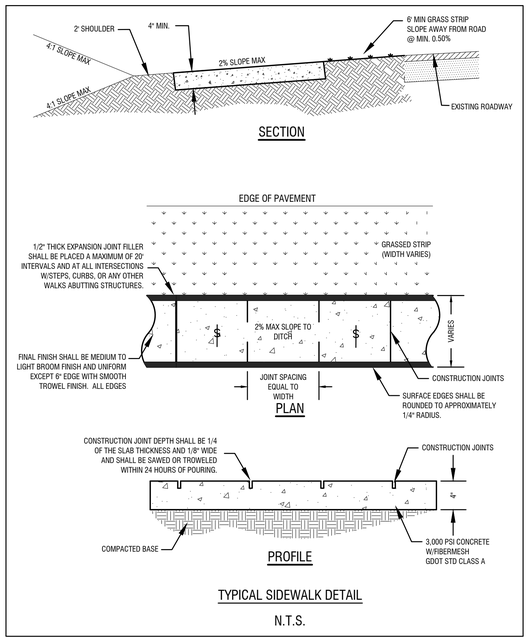
1. TAPPING SADDLE TO BE DUCTILE IRON WITH TYPE 304 STAINLESS STEEL FORGED DOUBLE STRAPS. STAINLESS STEEL BOLTS, NUTS, AND WASHERS. FINISH IS FUSION BONDED EPOXY TO AVERAGE THICKNESS OF 12-MILS.
2. ONE INCH (1") TAPS ON WATER LINES SMALLER THAN SIX INCHES (6") REQUIRE A TAPPING SADDLE. TWO INCH (2") TAPS REQUIRE A TAPPING SADDLE, REGARDLESS OF WATER MAIN SIZE.
3. A VALVE BOX SHALL BE USED FOR 2" CORPORATION STOPS. INSTALL PER DETAIL W4.

1" & 2" WATER TAPPING SADDLE

N.T.S.

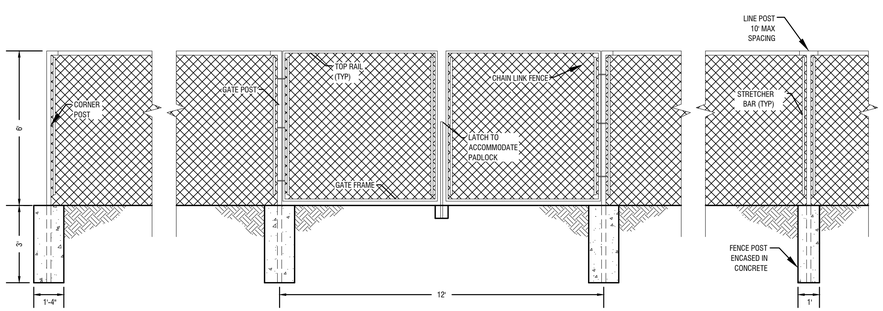
BLOW-OFF ASSEMBLY

N.T.S.



TYPICAL SIDEWALK DETAIL

N.T.S.



TYPICAL FENCE DETAIL

N.T.S.

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
MARCUS E. SACK
GSWCC LEVEL II # 70248
EXPIRES: 06/14/2026
MARCUS@MESACK.COM

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P.O. BOX 649
HINESVILLE, GA 31313
TEL: (912) 368-5212

GEORGIA REGISTERED No. 36559 PROFESSIONAL ENGINEER MARCUS E. SACK

DATE: 4/2/2025

ME SACK ENGINEERING

MUNICIPALITY:
CITY OF ALAMO

COUNTY:
WHEELER COUNTY

OWNER:
CITY OF ALAMO
5 WEST MAIN STREET
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(912) 568-7153
MAYOR@CITYOFALAMO.US

24 HOUR CONTACT:
JEFFERY FLOYD
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ALAMO, GA 30411
(912) 568-7153
CITYMANAGER@CITYOFALAMO.US

EAST SIDE PARK REMODEL

DETAILS

C505

FILE NO: 2023-66

PLOT DATE: April 2, 2025

PRE-DEVELOPED BASIN : ROAD 3 AREA = 0.07 AC

PRE-DEVELOPED BASIN : ROADSIDE DITCH 3 AREA = 0.90 AC

PRE-DEVELOPED BASIN : 2 AREA = 2.96 AC

PRE-DEVELOPED BASIN : ROAD 2 AREA = 0.06 AC

PRE-DEVELOPED BASIN : ROADSIDE DITCH 2 AREA = 0.54 AC

PRE-DEVELOPED BASIN : ROAD 1 AREA = 0.03 AC

PRE-DEVELOPED BASIN : ROADSIDE DITCH 1 AREA = 0.25 AC

PRE-DEVELOPED BASIN : 1 AREA = 2.70 AC

PRE-DEVELOPED BASIN : 3 AREA = 3.52 AC

STATON STREET (60' R/W)

WHEELER STREET (80' R/W)

UNNAMED DIRT ROAD

LEWIS STREET

N/F
LEWIS HARRY D

N/F
LEWIS LARRY D

SEABOARD COASTLINE RAILROAD (200' R/W)

SCALE BAR: 1"= 50'

50 0 25 50 100 200

DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
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DATE: 4/2/2025

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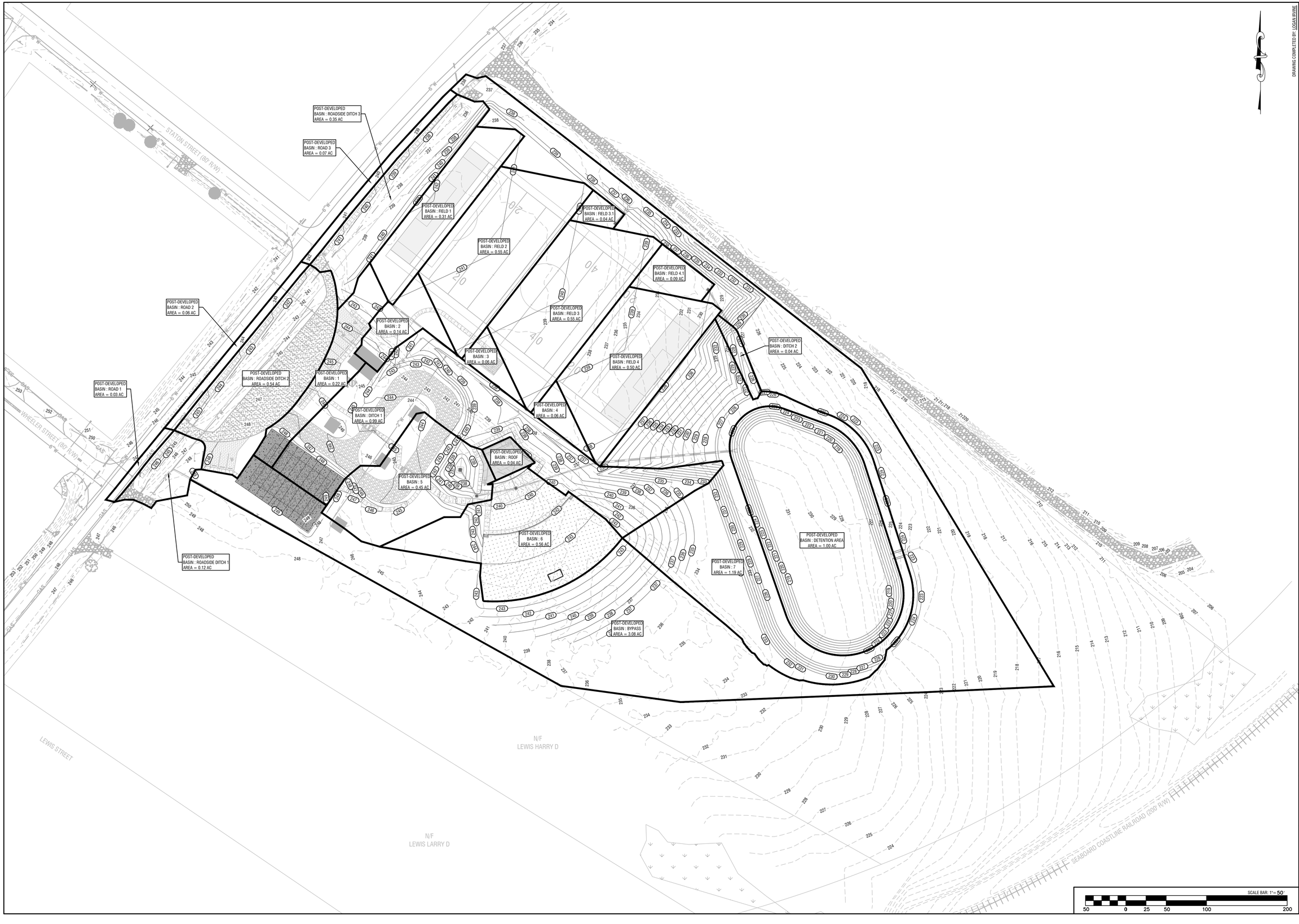
EAST SIDE PARK REMODEL

PRE DEVELOPED BASIN MAP

C600

FILE NO: 2023-66

PLOT DATE: April 2, 2025



DRAWING COMPLETED BY: LOGAN IRVINE

REVISIONS:

1.	ARCH/STRUC/ELEC ADJUSTMENTS

DESIGN PROFESSIONAL:
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GEORGIA REGISTERED No. 36559 PROFESSIONAL ENGINEER MARCUS E. SACK

DATE: 4/2/2025

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EAST SIDE PARK REMODEL

POST DEVELOPED BASIN MAP

C601

FILE NO: 2023-66

PLOT DATE: April 2, 2025

PROJECT CONSULTANTS

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email: logan@mesack.com

ELECTRICAL ENGINEER:
ELECTRICAL DESIGN CONSULTANTS, INC.
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(T) 912-225-4832
contact: BRIAN NEWLAND
email: bnewland@edc1973.com

ARCHITECT:
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(F) 912-447-8381
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contact: BRIAN SAPP
email: sappstructural.com

UTILITIES PROTECTION CENTER 1-800-282-7411 CALL BEFORE YOU DIG

SYMBOLS

DWG# / SHT# — BUILDING / WALL SECTION

ENLARGED DETAIL (DWG# / SHT#)

DWG# / SHT# — ELEVATIONS

ROOM NAME / 000 — ROOM TAG

DWG# / SHT# — INTERIOR ELEVATIONS

WN — WINDOW TAGS

DR# — DOOR TAG

GENERAL NOTES

1. GREENLINE ARCHITECTURE IS NOT RESPONSIBLE FOR INTERPRETING THE INTENT OF THESE CONSTRUCTION DOCUMENTS, INCLUDING MAKING MODIFICATIONS AS MAY BE NECESSARY DURING THE CONSTRUCTION PHASE. THE ABOVE NAMED COMPANY AND ARCHITECT OF RECORD ARE NOT LIABLE FOR THE WORK WHERE CHANGES TO THESE DOCUMENTS HAVE BEEN MADE.

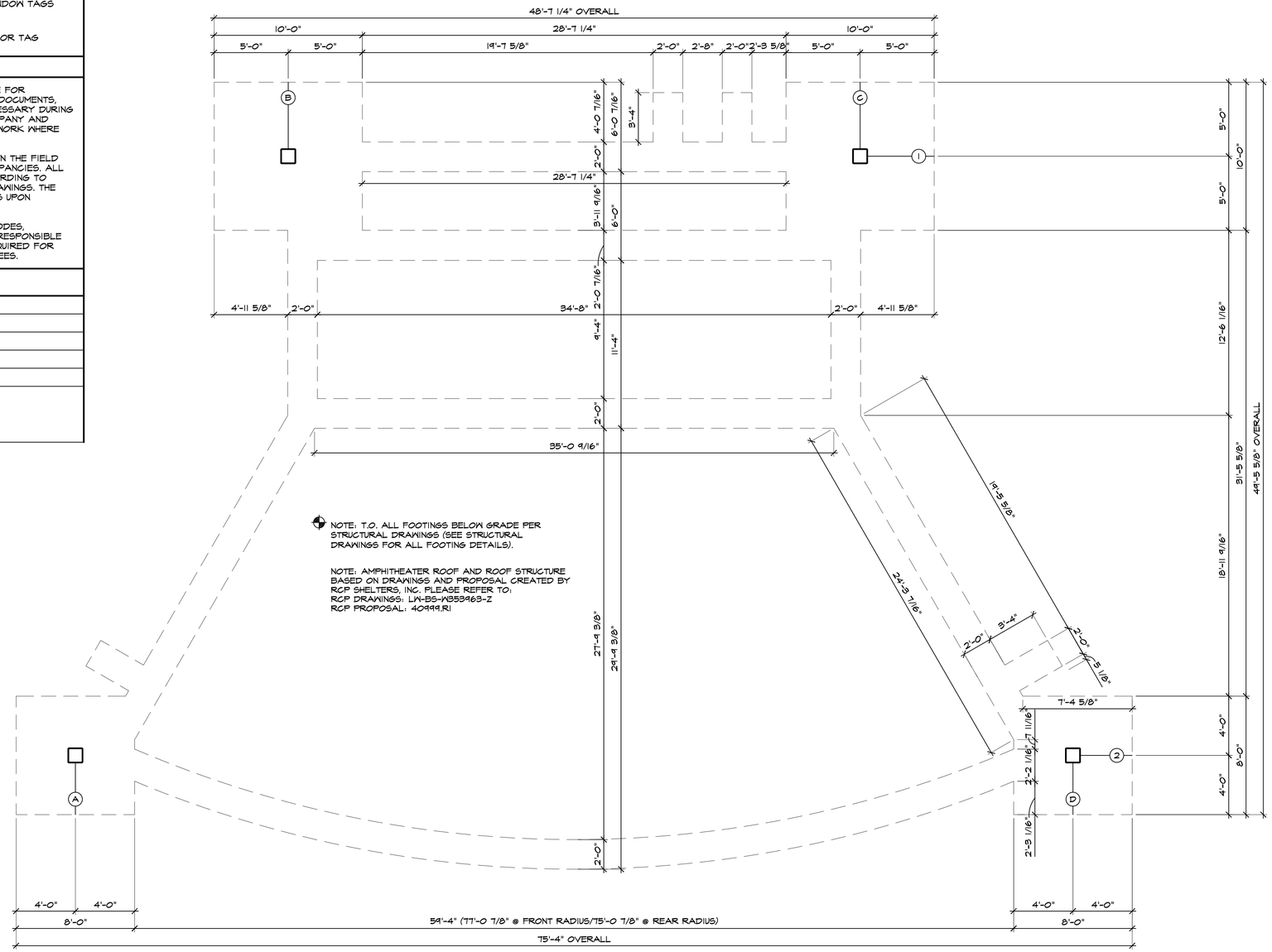
2. CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD AND SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES. ALL WORK REQUIRING MEASURING SHALL BE DONE ACCORDING TO FIGURES ON DRAWINGS AND NOT SCALED FROM DRAWINGS. THE ARCHITECT SHALL FURNISH ANY MISSING DIMENSIONS UPON REQUEST.

3. ALL WORK SHALL CONFORM TO PREVAILING CODES, ORDINANCES AND REQUIREMENTS. CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS REQUIRED FOR CONSTRUCTION AND SHALL PAY ALL APPLICABLE FEES.

CODE REVIEW DATA

PROPERTY ADDRESS	EAST RAILROAD STREET - ALAMO, GA
PROPERTY PIN #	XXXX-XXXX
ZONING DISTRICT	CONSERVATION - PARK
PARKING REQUIREMENTS	REFER TO CIVIL DRAWINGS
GROSS PROJECT AREA	8.86 ACRES

APPLICABLE CODES:
2018 INTERNATIONAL BUILDING CODE
2018 INTERNATIONAL MECHANICAL CODE
2018 INTERNATIONAL PLUMBING CODE
2018 INTERNATIONAL GAS CODE
2018 INTERNATIONAL FIRE CODE
2023 NATIONAL ELECTRIC CODE
2012 GEORGIA ACCESSIBILITY CODE



JOB NORTH

1 A1.1 FOUNDATION PLAN
SCALE: 1/4"=1'-0"

GREENLINE ARCHITECTURE
28 E 35TH ST
SAVANNAH, GA 31401
T 912.447.5665
F 912.447.8381
WWW.GREENLINEARCH.COM

EAST SIDE PARK AMPHITHEATER

OWNER: CITY OF ALAMO, GA
OWNER ADDRESS: 5 WEST MAIN STREET
ALAMO, GEORGIA 30411
OWNER TELEPHONE: (912) 568-7153
OWNER EMAIL: mayor@cityofalamo.us

REVISIONS
1 --/--/2025 ----

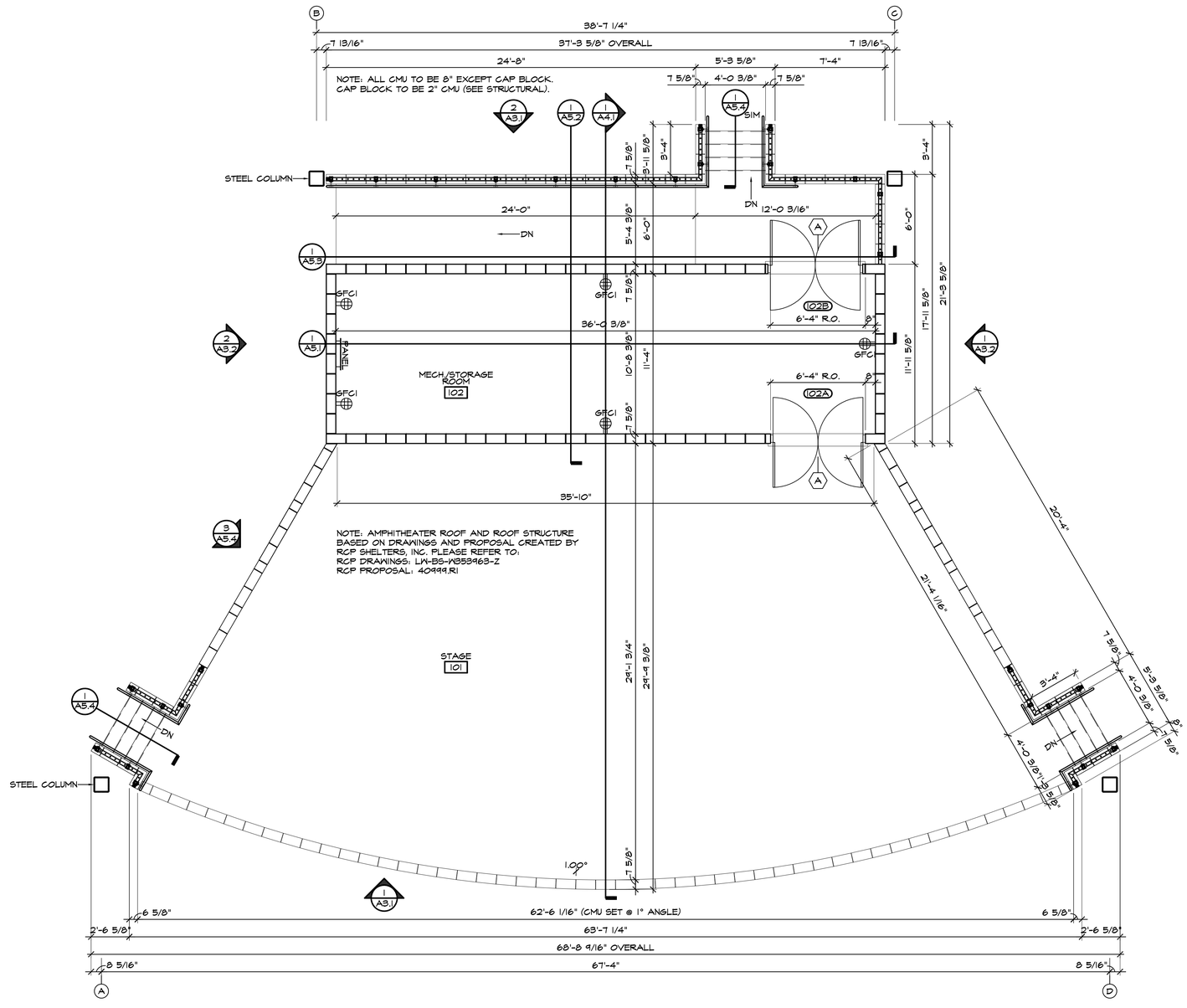
FOUNDATION PLAN

STATE OF GEORGIA
MONICA D. MASTRIANNI
CERTIFICATE NO. 035556
REGISTERED ARCHITECT

ISSUED FOR CONSTRUCTION

JOB NO: 24.023
ISSUE DATE: 01.22.2025
DRAWN: WJK

A1.1



JOB NORTH

1 A1.2 FLOOR PLAN
SCALE: 1/4"=1'-0"

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EAST SIDE PARK AMPHITHEATER

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ALAMO, GEORGIA 30411
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OWNER EMAIL: mayor@cityofalamo.us

REVISIONS

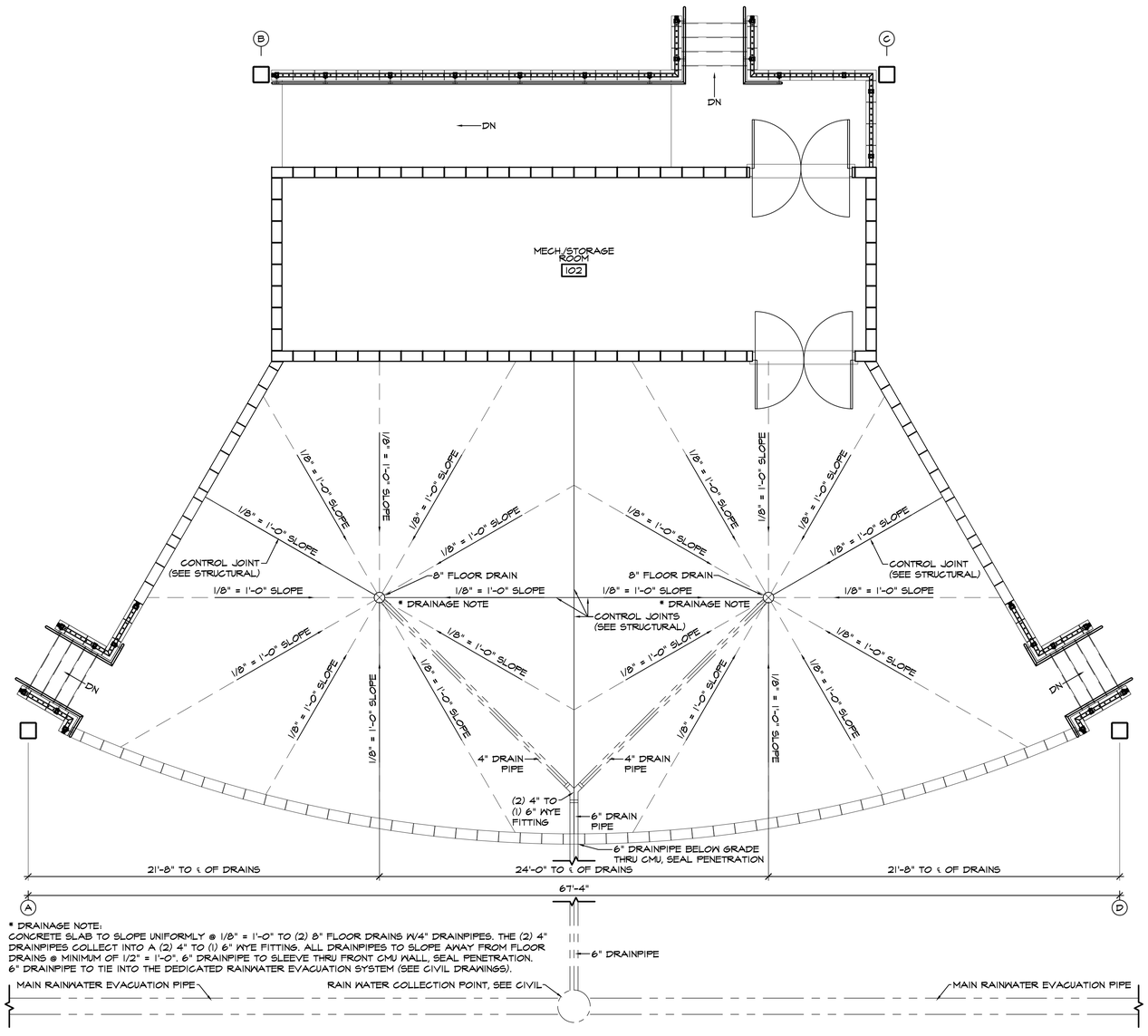
FLOOR PLAN

STATE OF GEORGIA
MONICA D. MASTRIANNI
CERTIFICATE NO. 030595
REGISTERED ARCHITECT

ISSUED FOR CONSTRUCTION

JOB NO: 24.023
ISSUE DATE: 01.22.2025
DRAWN: WJK

A1.2



* DRAINAGE NOTE:
CONCRETE SLAB TO SLOPE UNIFORMLY @ 1/8" = 1'-0" TO (2) 8" FLOOR DRAINS W/4" DRAINPIPES. THE (2) 4" DRAINPIPES COLLECT INTO A (2) 4" TO (1) 6" WYE FITTING. ALL DRAINPIPES TO SLOPE AWAY FROM FLOOR DRAINS @ MINIMUM OF 1/2" = 1'-0". 6" DRAINPIPE TO SLEEVE THRU FRONT CMU WALL, SEAL PENETRATION. 6" DRAINPIPE TO TIE INTO THE DEDICATED RAINWATER EVACUATION SYSTEM (SEE CIVIL DRAWINGS).

JOB NORTH

1 / A1.3 STAGE DRAINAGE PLAN
SCALE: 1/4"=1'-0"

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SAVANNAH, GA 31401
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EAST SIDE PARK AMPHITHEATER

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OWNER EMAIL: mayor@cityofalamo.us

REVISIONS

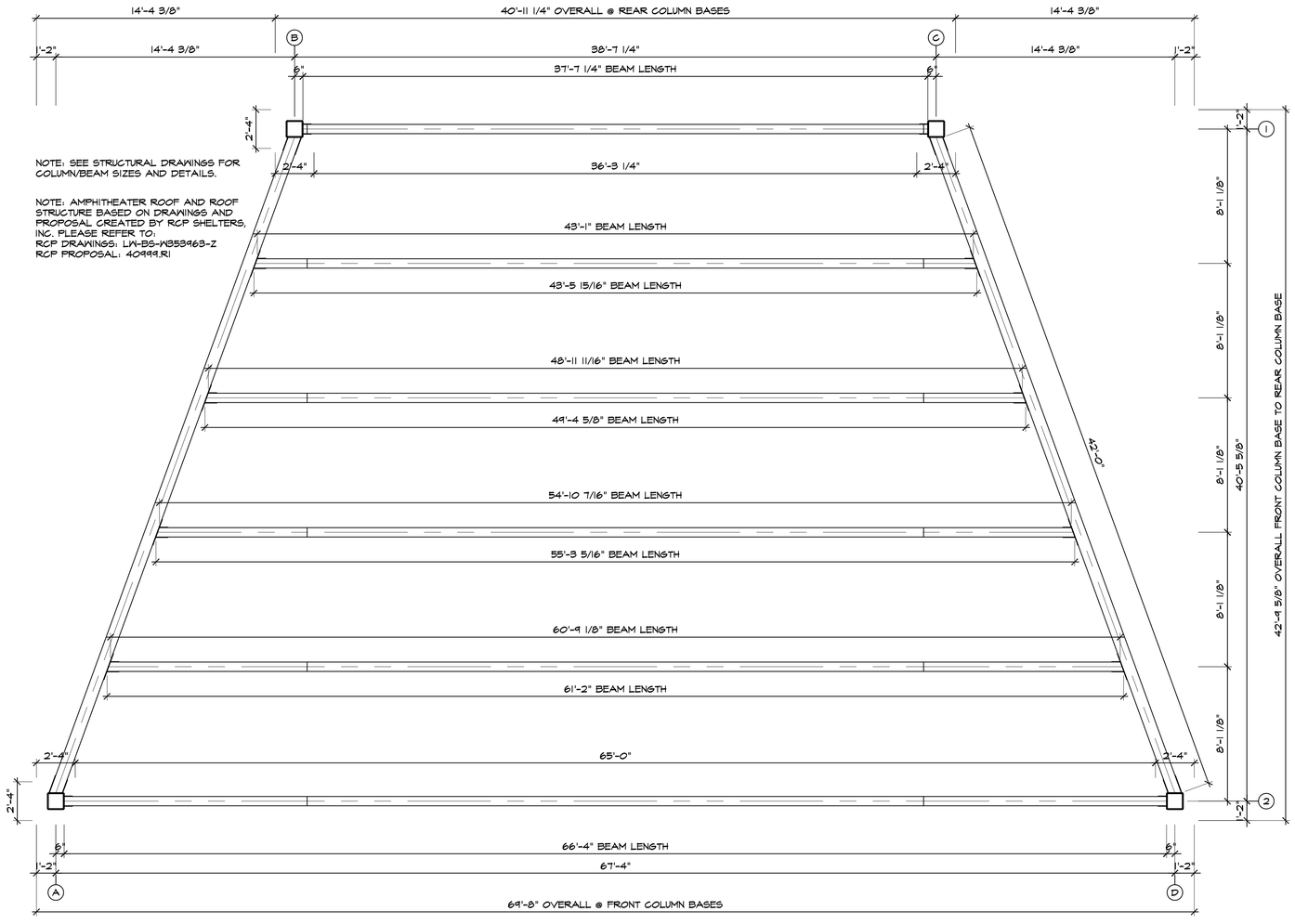
STAGE DRAINAGE PLAN

STATE OF GEORGIA
MONICA D. MASTRIANNI
CERTIFICATE NO. 031598
REGISTERED ARCHITECT

ISSUED FOR CONSTRUCTION

JOB NO: 24.023
ISSUE DATE: 01.22.2025
DRAWN: WJK

A1.3



JOB NORTH

1 / A1.4 ROOF STRUCTURE PLAN
SCALE: 1/4"=1'-0"

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28 E 35TH ST
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EAST SIDE PARK AMPHITHEATER

OWNER: CITY OF ALAMO, GA
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OWNER TELEPHONE: (912) 568-7153
OWNER EMAIL: mayor@cityofalamo.us

ROOF STRUCTURE PLAN

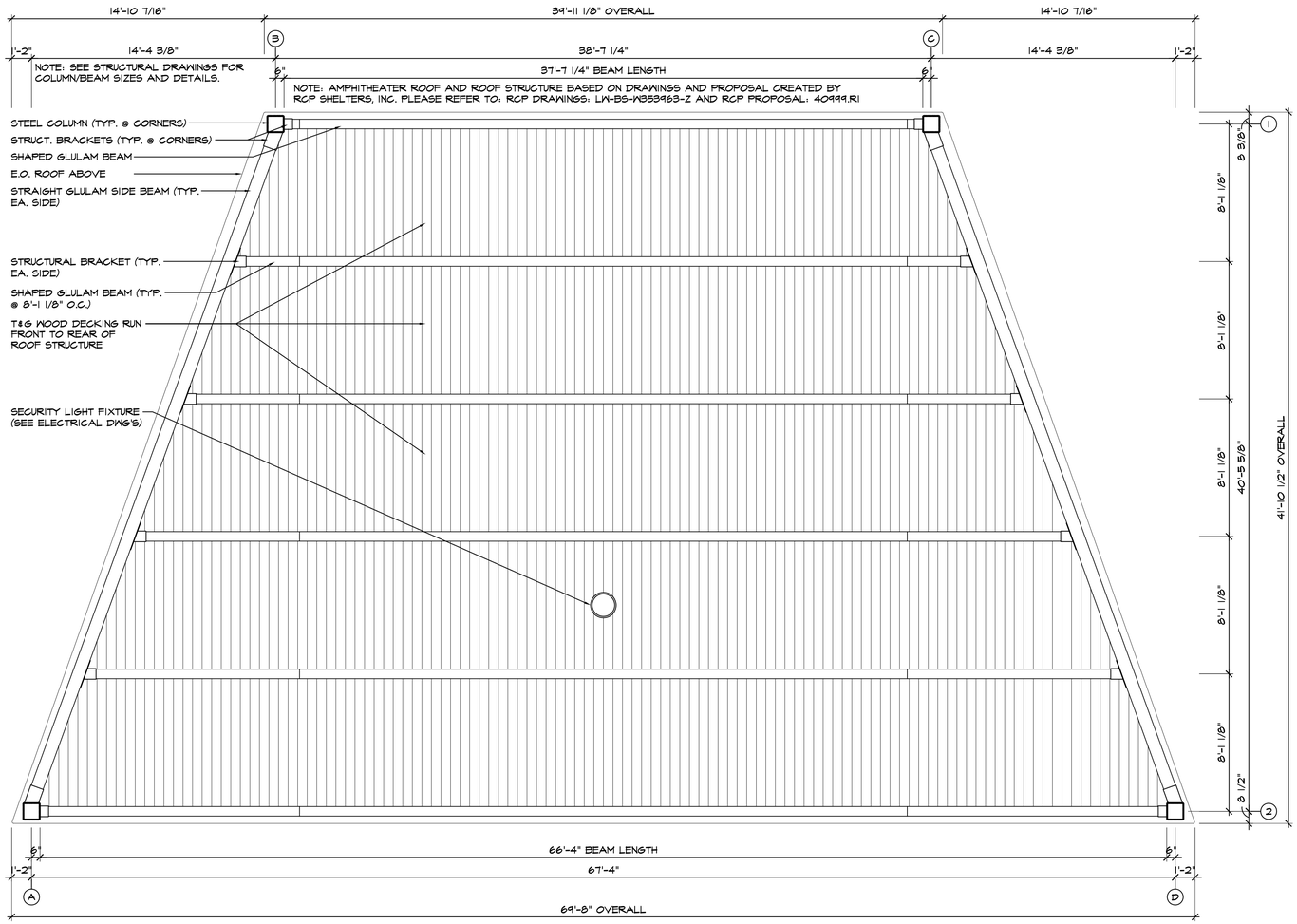
REVISIONS

STATE OF GEORGIA
MONICA D. MASTRIANNI
CERTIFICATE NO. 030595
REGISTERED ARCHITECT

ISSUED FOR CONSTRUCTION

JOB NO: 24.023
ISSUE DATE: 01.22.2025
DRAWN: WJK

A1.4



JOB NORTH

1 / A1.5 REFLECTED CEILING PLAN
SCALE: 1/4"=1'-0"

GREEN LINE ARCHITECTURE
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SAVANNAH, GA 31401
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EAST SIDE PARK AMPHITHEATER

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OWNER ADDRESS: 5 WEST MAIN STREET
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OWNER EMAIL: mayor@cityofalamo.us

REVISIONS

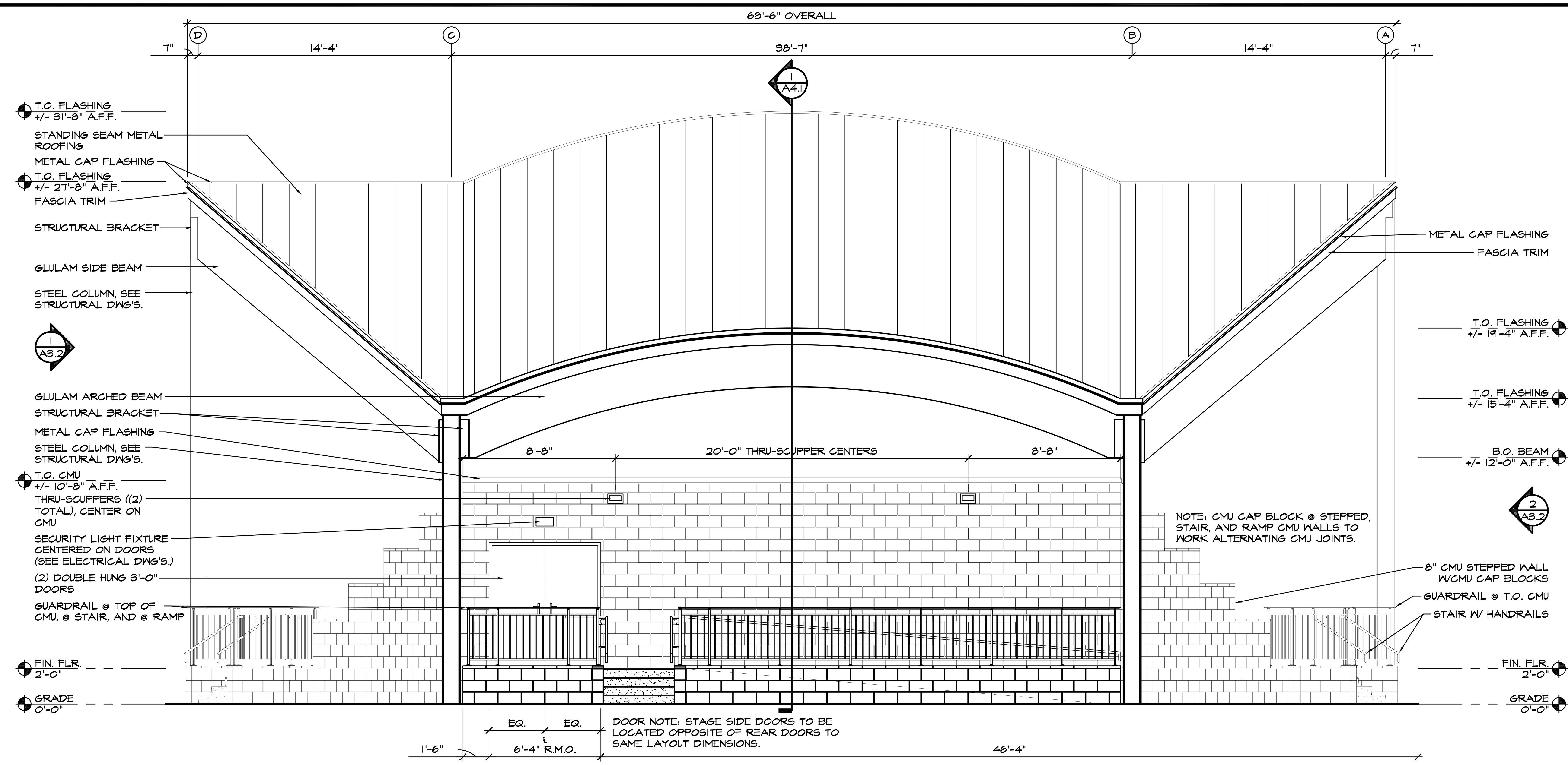
REFLECTED CEILING PLAN

STATE OF GEORGIA
MONICA D. MASTRIANNI
CERTIFICATE NO. 027695
REGISTERED ARCHITECT

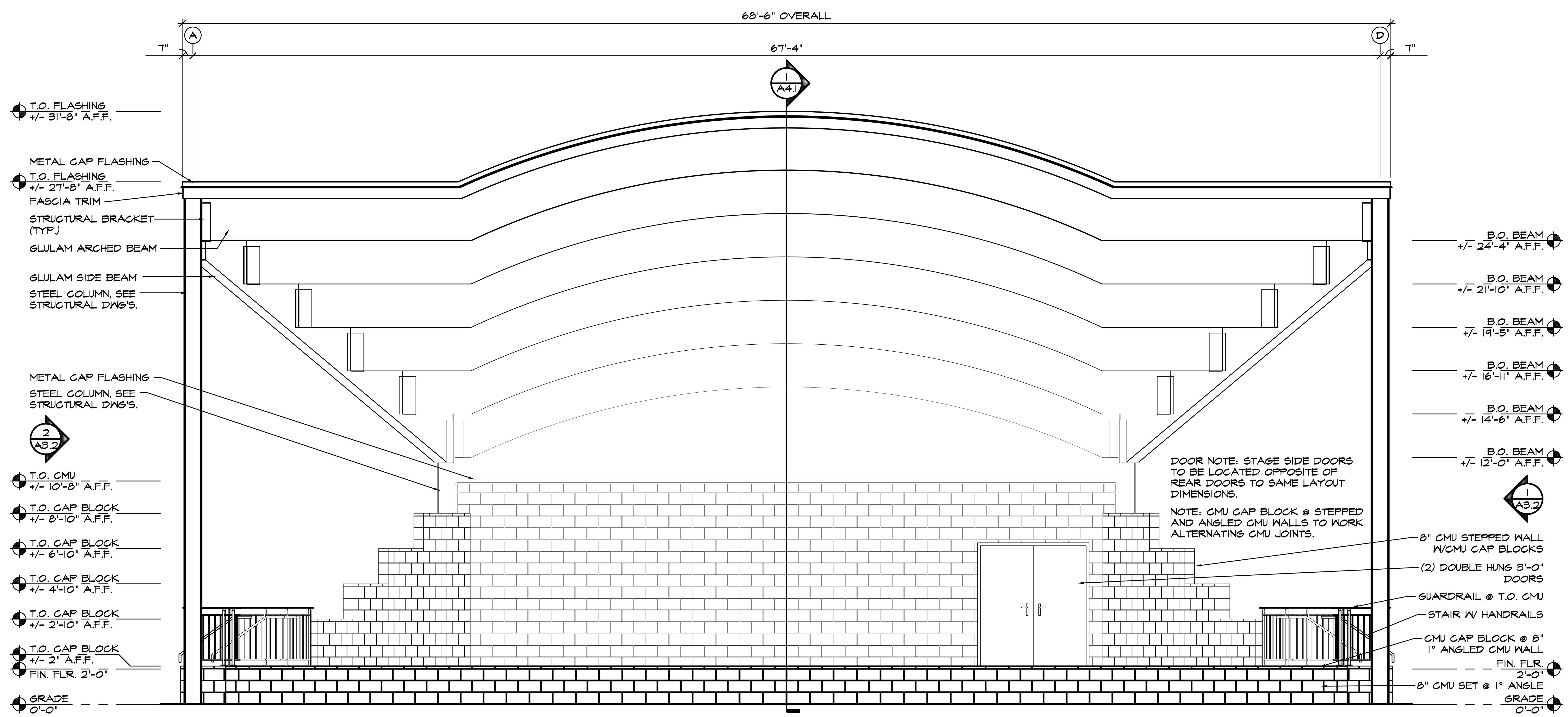
ISSUED FOR CONSTRUCTION

JOB NO: 24.023
ISSUE DATE: 01.22.2025
DRAWN: WJK

A1.5



2 / A3.1 NORTH ELEVATION
SCALE: 1/4"=1'-0"



1 / A3.1 SOUTH ELEVATION
SCALE: 1/4"=1'-0"

GREENLINE ARCHITECTURE
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EAST SIDE PARK AMPHITHEATER

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REVISIONS

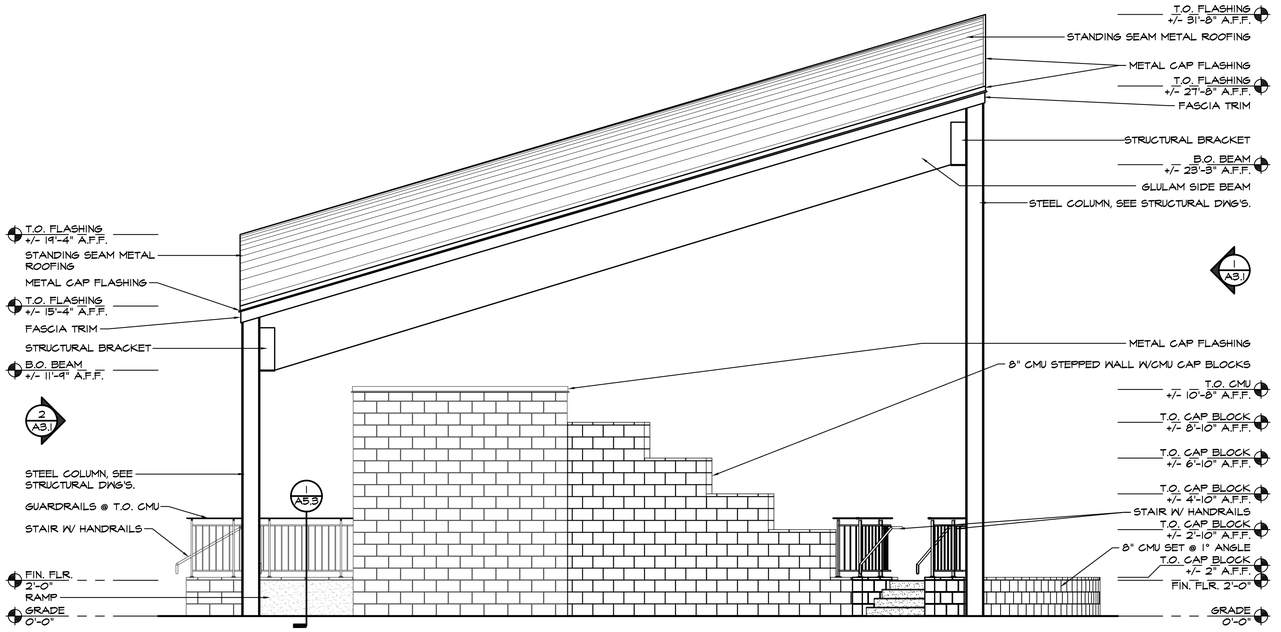
ELEVATIONS

STATE OF GEORGIA
MONICA D. MASTRIANNI
CERTIFICATE NO. 007695
REGISTERED ARCHITECT

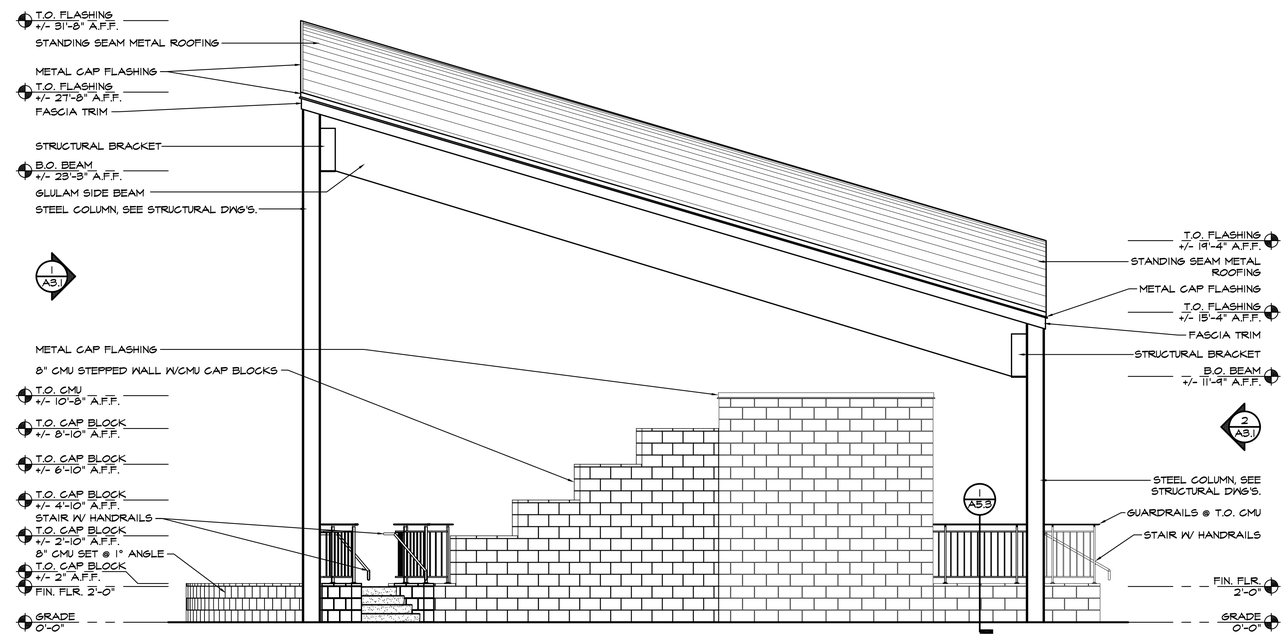
ISSUED FOR CONSTRUCTION

JOB NO: 24.023
ISSUE DATE: 01.22.2025
DRAWN: WJK

A3.1



2 A3.2 WEST ELEVATION
SCALE: 1/4"=1'-0"



1 A3.2 EAST ELEVATION
SCALE: 1/4"=1'-0"

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REVISIONS

ELEVATIONS

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ISSUED FOR CONSTRUCTION

JOB NO: 24.023
ISSUE DATE: 01.22.2025
DRAWN: WJK

A3.2

EXTERIOR FINISH SCHEDULE

CODE	DESCRIPTION	COLOR	MANUFACTURER CONTACT
EX1	2" AND 8" SPLIT-FACE CMU	T.B.D.	T.B.D.
	LOWER APRON CAPS/FULL BLOCK	ANTI-GRAFFITI (A/G) PAINT	
EX2	2" AND 8" SPLIT-FACE CMU	T.B.D.	T.B.D.
	UPPER CAPS/FULL BLOCK	ANTI-GRAFFITI (A/G) PAINT	
EX3	CMU MORTAR	T.B.D.	T.B.D.
	ALL CMU MORTAR JOINTS	A/G PAINT PER EX1 AND EX2	
EX4	CONCRETE	T.B.D.	T.B.D.
	SLABS AND STAIRS		
EX5	HANDRAIL SYSTEM	BRUSHED ALUMINUM	JULIUS BLUM & CO. - CARLSTADT
	GUARDRAIL/HANDRAIL/PICKET/POST		
EX6	DOORS/FRAMES	T.B.D.	T.B.D.
	----	ANTI-GRAFFITI (A/G) PAINT	
EX7	DOOR HARDWARE	T.B.D.	T.B.D.

EX8	THRU-SCUPPERS	T.B.D.	T.B.D.

EX9	METAL CAP FLASHING	T.B.D.	T.B.D.
	CMU AND ROOF TRIM FLASHING		
EX10	COLUMNS/BRACKETS/HARDWARE	T.B.D.	T.B.D.
	COLS./STRUCTURAL CONNECTIONS		
EX11	GLULAM BEAMS	T.B.D.	T.B.D.
	WAVEFORM AND SIDE BEAMS		
EX12	T&G WOOD PLANKS	T.B.D.	T.B.D.
	EXPOSED T&G WOOD CEILING		
EX13	FASCIA TRIM	T.B.D.	T.B.D.

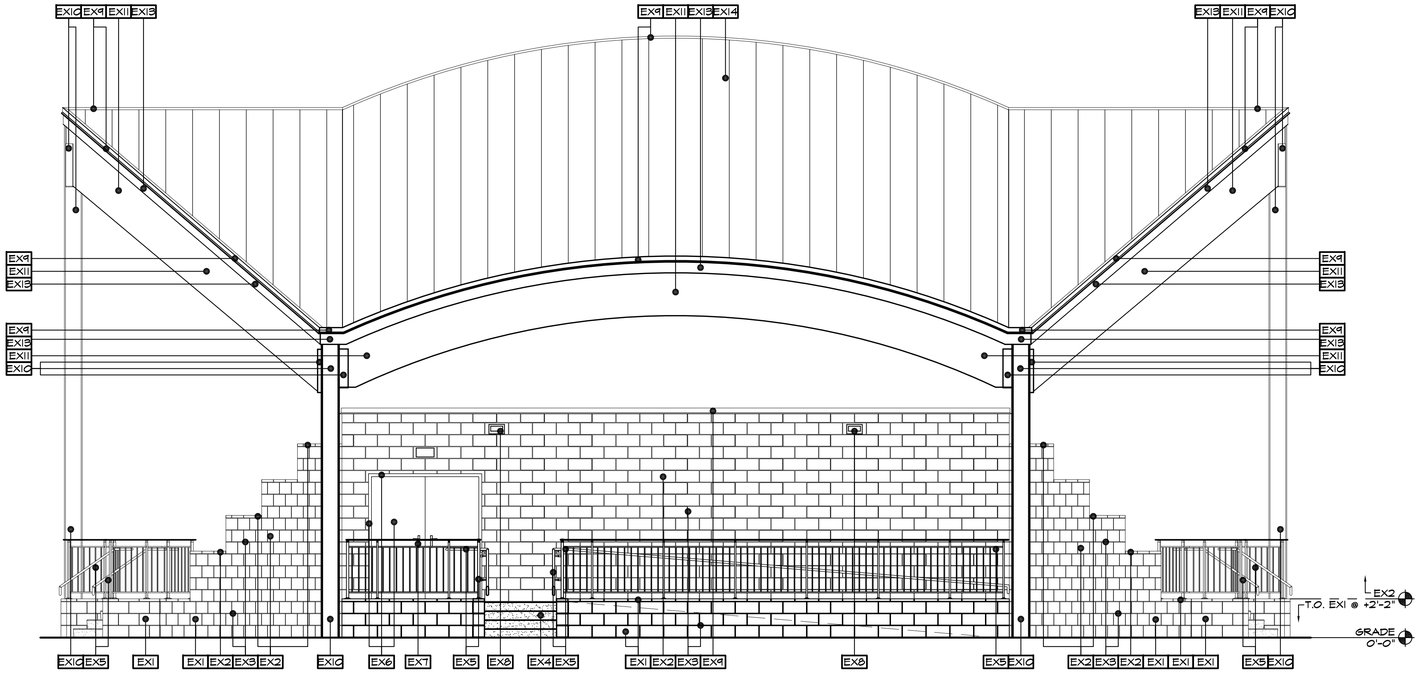
EX14	STANDING SEAM METAL ROOF	T.B.D.	T.B.D.

EXTERIOR GENERAL NOTES

1.) MATERIALS MAY NOT BE SUBSTITUTED WITHOUT PERMISSION FROM OWNER AND/OR ARCHITECT.

2.) GENERAL CONTRACTOR TO FIELD VERIFY ALL COLORS PRIOR TO CONSTRUCTION OR APPLICATION AND PROVIDE SAMPLES TO OWNER AND/OR ARCHITECT FOR REVIEW.

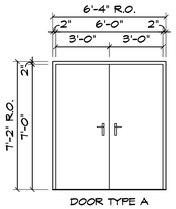
5 / A3.3 EXTERIOR FINISH SCHEDULE
SCALE: N.T.S.



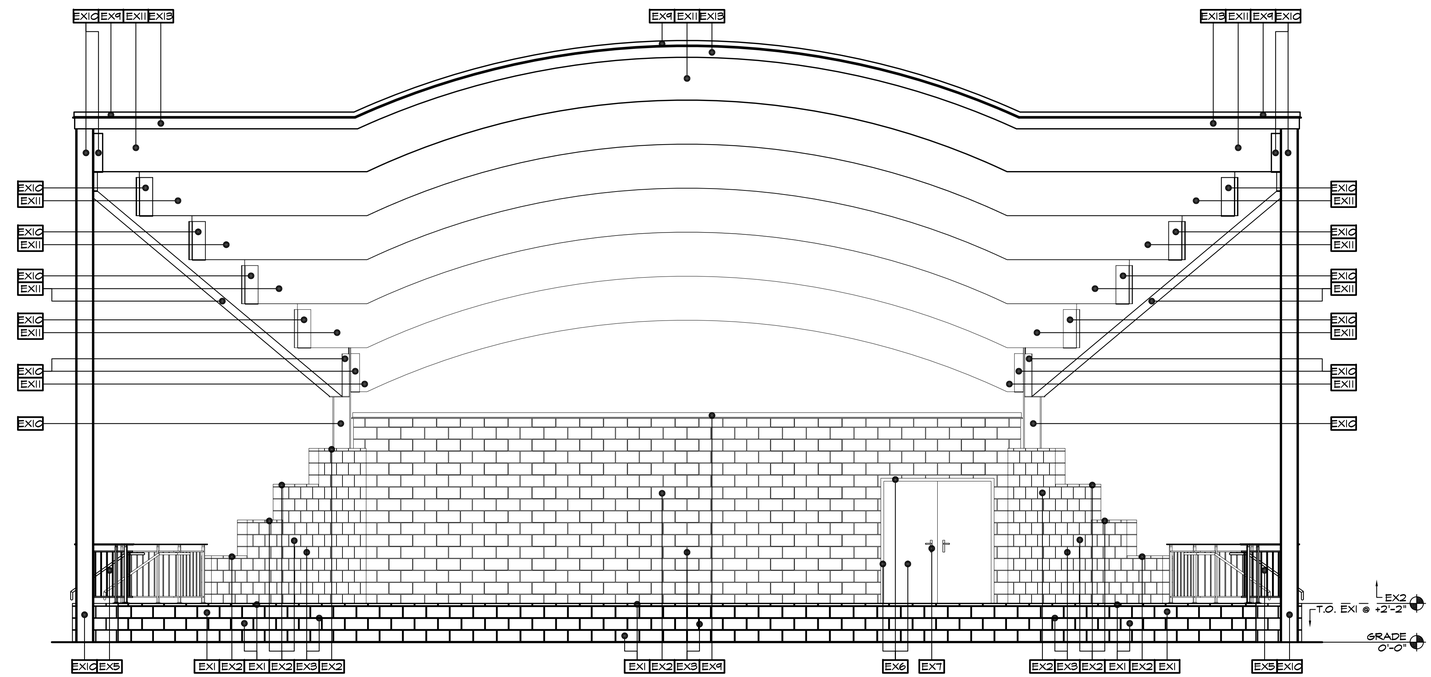
2 / A3.3 NORTH EXTERIOR FINISH ELEVATION
SCALE: 1/4"=1'-0"

DOOR SYMBOL		DOOR					FRAME	LOCKSET		
DOOR MARK	DOOR SPACE	WIDTH	HEIGHT	THICKNESS	MATERIAL	TYPE	MATERIAL	LOCK	KEYSIDE	REMARKS
102A	EXT	6'-0"	7'-0"	1-3/4"	STEEL	A	STEEL	-	EXT	PAIR OF 3'-0" DOUBLE HUNG DOORS (SWING IN/OUT)
102B	EXT	6'-0"	7'-0"	1-3/4"	STEEL	A	STEEL	-	EXT	PAIR OF 3'-0" DOUBLE HUNG DOORS (SWING IN/OUT)

4 / A3.3 DOOR SCHEDULE
SCALE: N.T.S.



3 / A3.3 DOOR FRAMES AND TYPES
SCALE: N.T.S.



1 / A3.3 SOUTH EXTERIOR FINISH ELEVATION
SCALE: 1/4"=1'-0"

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EAST SIDE PARK AMPHITHEATER

OWNER: CITY OF ALAMO, GA
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ALAMO, GEORGIA 30411
OWNER TELEPHONE: (912) 568-7153
OWNER EMAIL: mayor@cityofalamo.us

REVISIONS

EXTERIOR FINISH ELEVATIONS

STATE OF GEORGIA
MONICA D. MASTRIANNI
CERTIFICATE NO. 030595
REGISTERED ARCHITECT

ISSUED FOR CONSTRUCTION

JOB NO: 24.023
ISSUE DATE: 01.22.2025
DRAWN: WJK

A3.3

EXTERIOR FINISH SCHEDULE

CODE	DESCRIPTION	COLOR	MANUFACTURER CONTACT
EX1	2" AND 8" SPLIT-FACE CMU	T.B.D.	T.B.D.
	LOWER APRON CAPS/FULL BLOCK	ANTI-GRAFFITI (A/G) PAINT	
EX2	2" AND 8" SPLIT-FACE CMU	T.B.D.	T.B.D.
	UPPER CAPS/FULL BLOCK	ANTI-GRAFFITI (A/G) PAINT	
EX3	CMU MORTAR	T.B.D.	T.B.D.
	ALL CMU MORTAR JOINTS	A/G PAINT PER EX1 AND EX2	
EX4	CONCRETE	T.B.D.	T.B.D.
	SLABS AND STAIRS		
EX5	HANDRAIL SYSTEM	BRUSHED ALUMINUM	JULIUS BLUM & CO. - CARLSTADT
	GUARDRAIL/HANDRAIL/PICKET/POST		
EX6	DOORS/FRAMES	T.B.D.	T.B.D.
	----	ANTI-GRAFFITI (A/G) PAINT	
EX7	DOOR HARDWARE	T.B.D.	T.B.D.

EX8	THRU-SCUPPERS	T.B.D.	T.B.D.

EX9	METAL CAP FLASHING	T.B.D.	T.B.D.
	CMU AND ROOF TRIM FLASHING		
EX10	COLUMNS/BRACKETS/HARDWARE	T.B.D.	T.B.D.
	COLS./STRUCTURAL CONNECTIONS		
EX11	GLULAM BEAMS	T.B.D.	T.B.D.
	WAVEFORM AND SIDE BEAMS		
EX12	T&G WOOD PLANKS	T.B.D.	T.B.D.
	EXPOSED T&G WOOD CEILING		
EX13	FASCIA TRIM	T.B.D.	T.B.D.

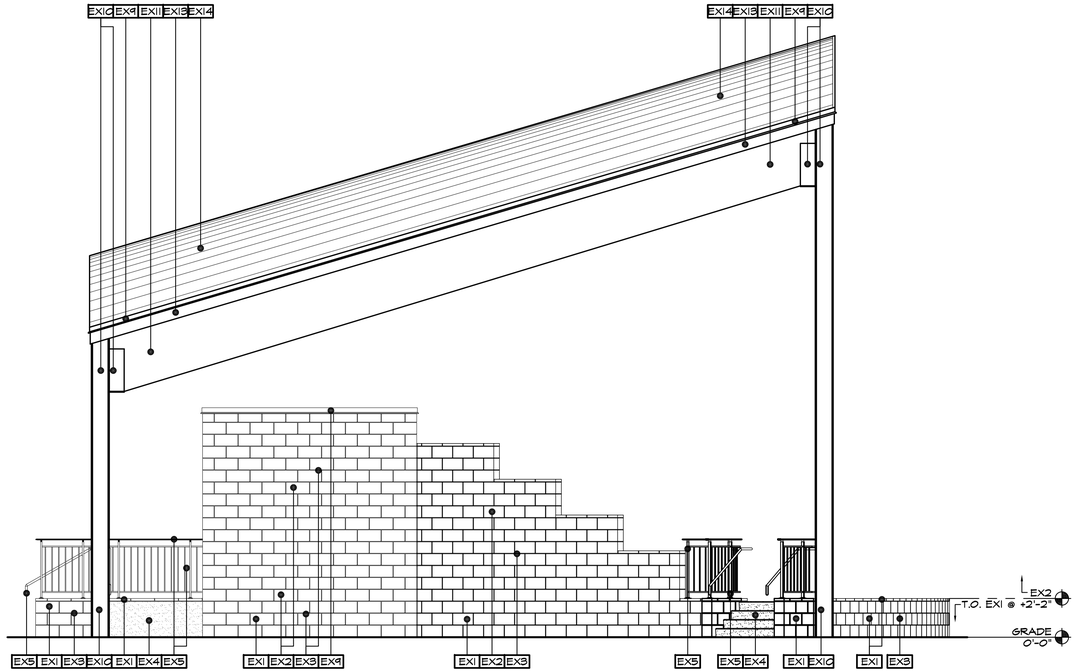
EX14	STANDING SEAM METAL ROOF	T.B.D.	T.B.D.

EXTERIOR GENERAL NOTES

1.) MATERIALS MAY NOT BE SUBSTITUTED WITHOUT PERMISSION FROM OWNER AND/OR ARCHITECT.

2.) GENERAL CONTRACTOR TO FIELD VERIFY ALL COLORS PRIOR TO CONSTRUCTION OR APPLICATION AND PROVIDE SAMPLES TO OWNER AND/OR ARCHITECT FOR REVIEW.

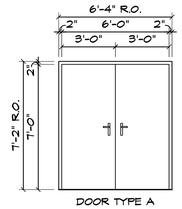
5 / A3.4 EXTERIOR FINISH SCHEDULE
SCALE: N.T.S.



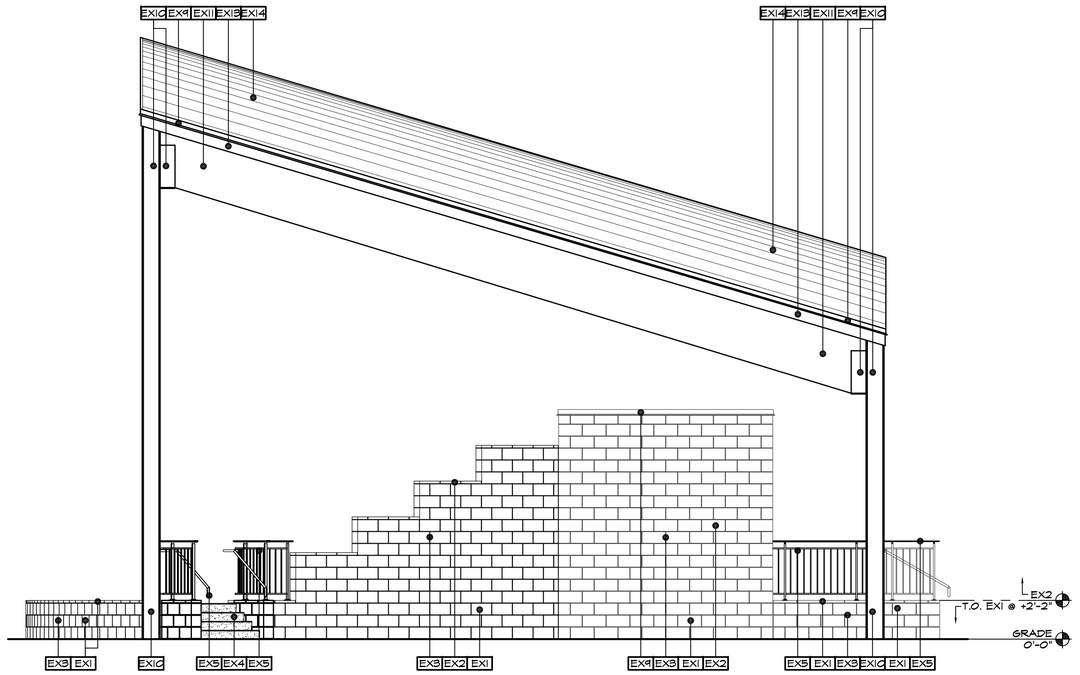
2 / A3.4 WEST EXTERIOR FINISH ELEVATION
SCALE: 1/4"=1'-0"

DOOR SYMBOL		DOOR					FRAME	LOCKSET		
DOOR MARK	DOOR SPACE	WIDTH	HEIGHT	THICKNESS	MATERIAL	TYPE	MATERIAL	LOCK	KEYSIDE	REMARKS
102A	EXT	6'-0"	7'-0"	1-3/4"	STEEL	A	STEEL	-	EXT	PAIR OF 3'-0" DOUBLE HUNG DOORS (SWING IN/OUT)
102B	EXT	6'-0"	7'-0"	1-3/4"	STEEL	A	STEEL	-	EXT	PAIR OF 3'-0" DOUBLE HUNG DOORS (SWING IN/OUT)

4 / A3.4 DOOR SCHEDULE
SCALE: N.T.S.



3 / A3.4 DOOR FRAMES AND TYPES
SCALE: N.T.S.



1 / A3.4 EAST EXTERIOR FINISH ELEVATION
SCALE: 1/4"=1'-0"

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EAST SIDE PARK AMPHITHEATER

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REVISIONS
1 --/--/2024 ----

EXTERIOR FINISH ELEVATIONS

STATE OF GEORGIA
MONICA D. MASTRIANNI
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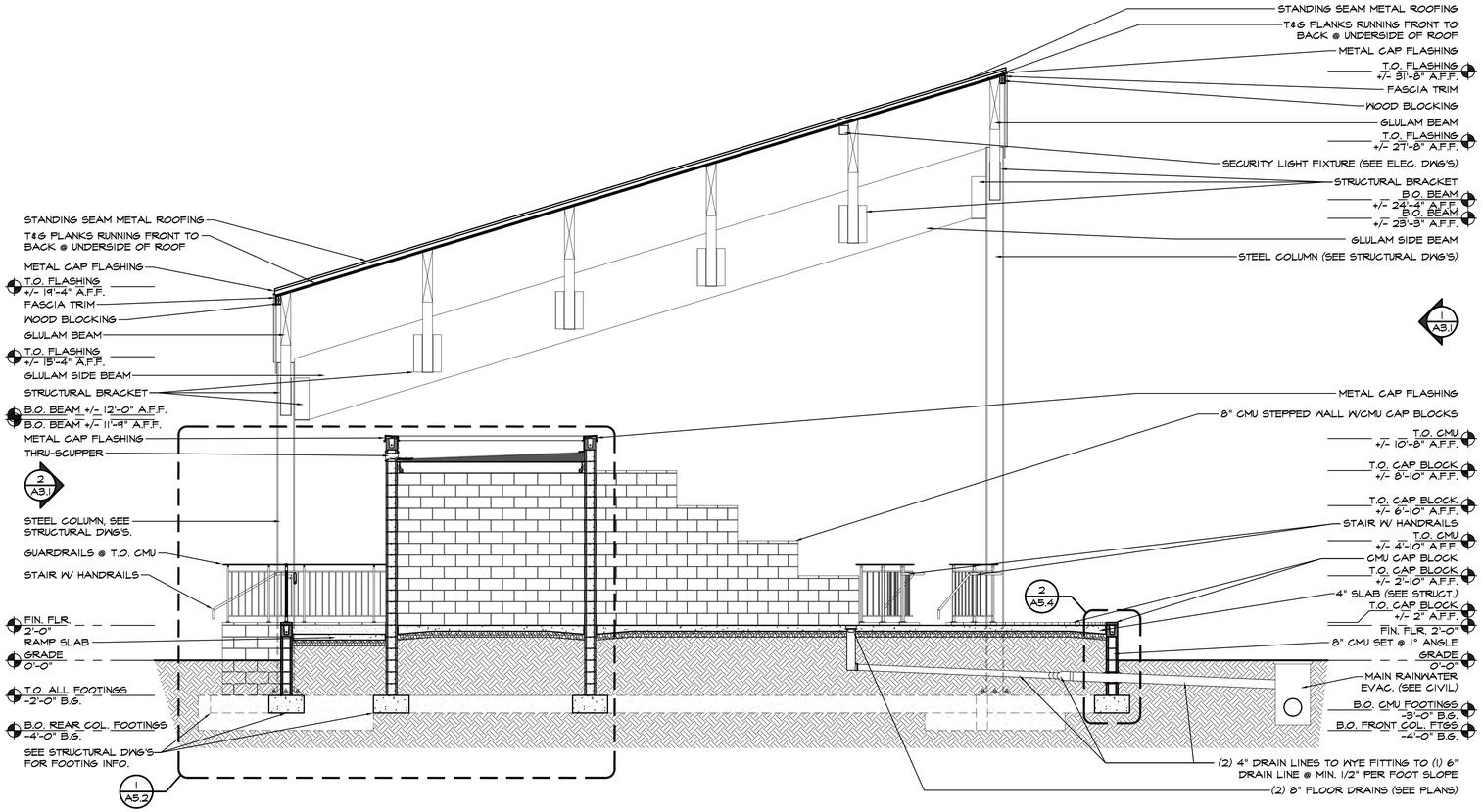

1 A4.1 LONGITUDIDINAL SECTION
SCALE: 1/4"=1'-0"

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BUILDING SECTION

REVISIONS
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STATE OF GEORGIA
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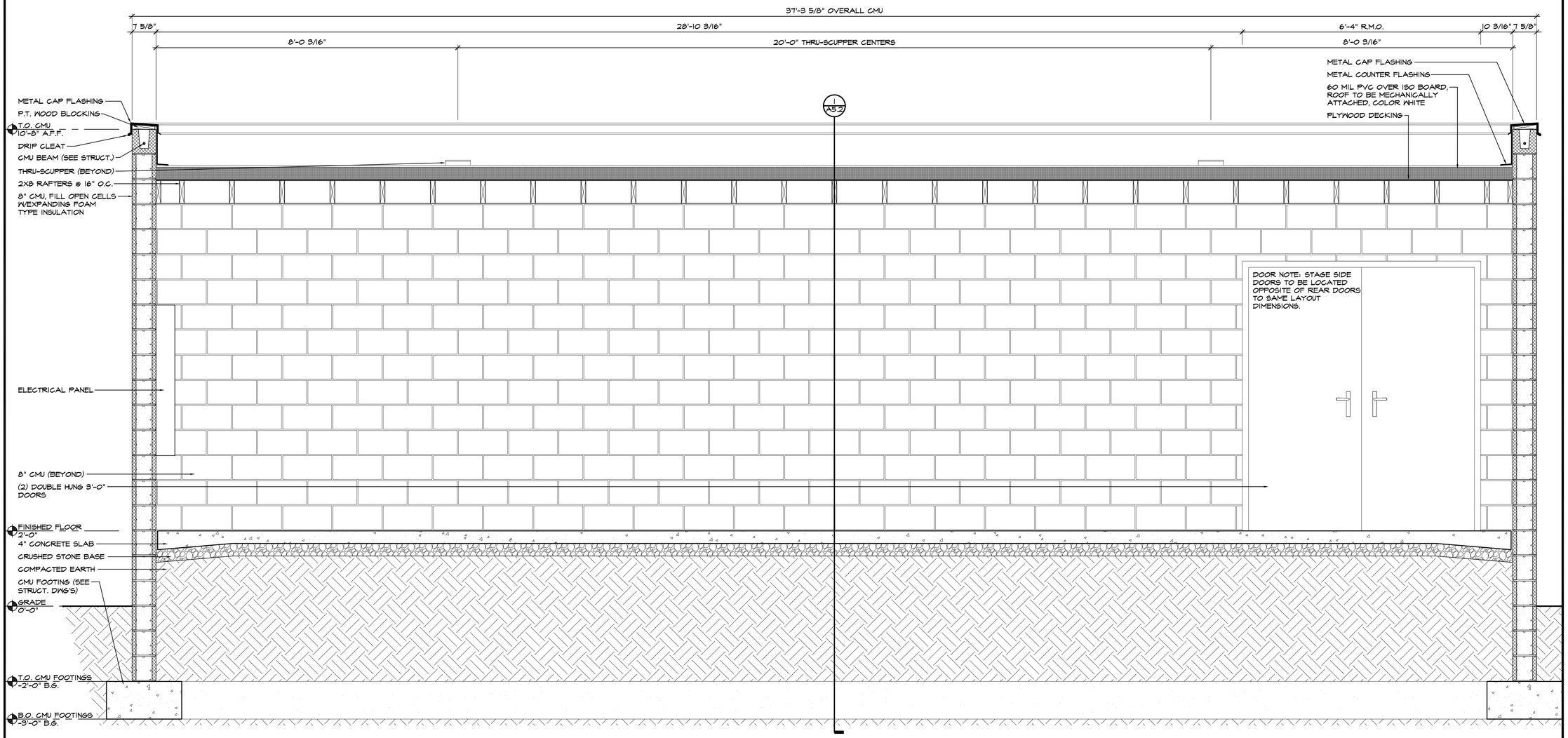
A4.1



1/A5.1 ENLARGED SECTION
SCALE: 3/4" = 1'-0"

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ENLARGED SECTIONS

STATE OF GEORGIA
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CERTIFICATE NO. 029595
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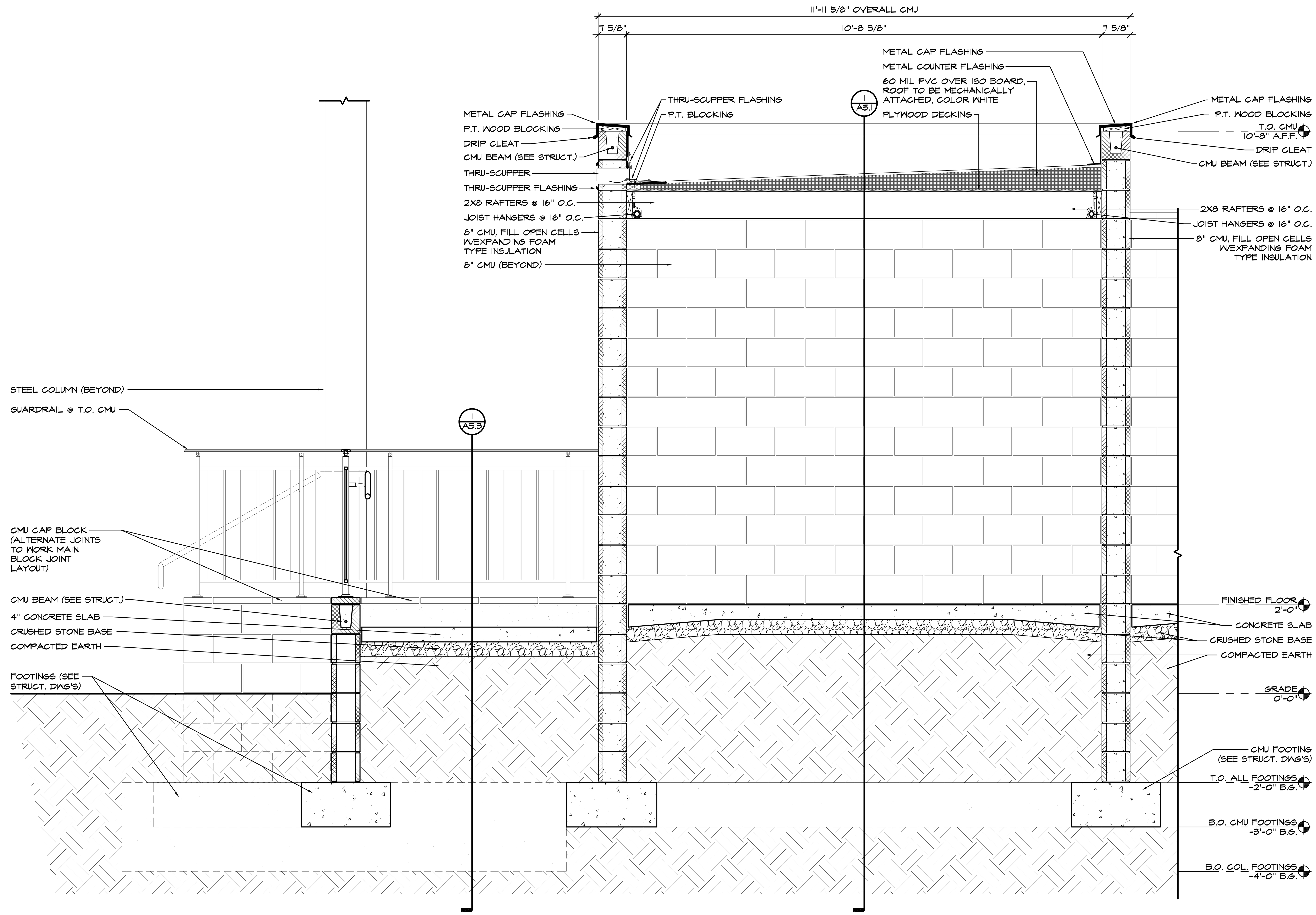
A5.1



1 A5.2 **ENLARGED SECTION**
SCALE: 3/4" = 1'-0"

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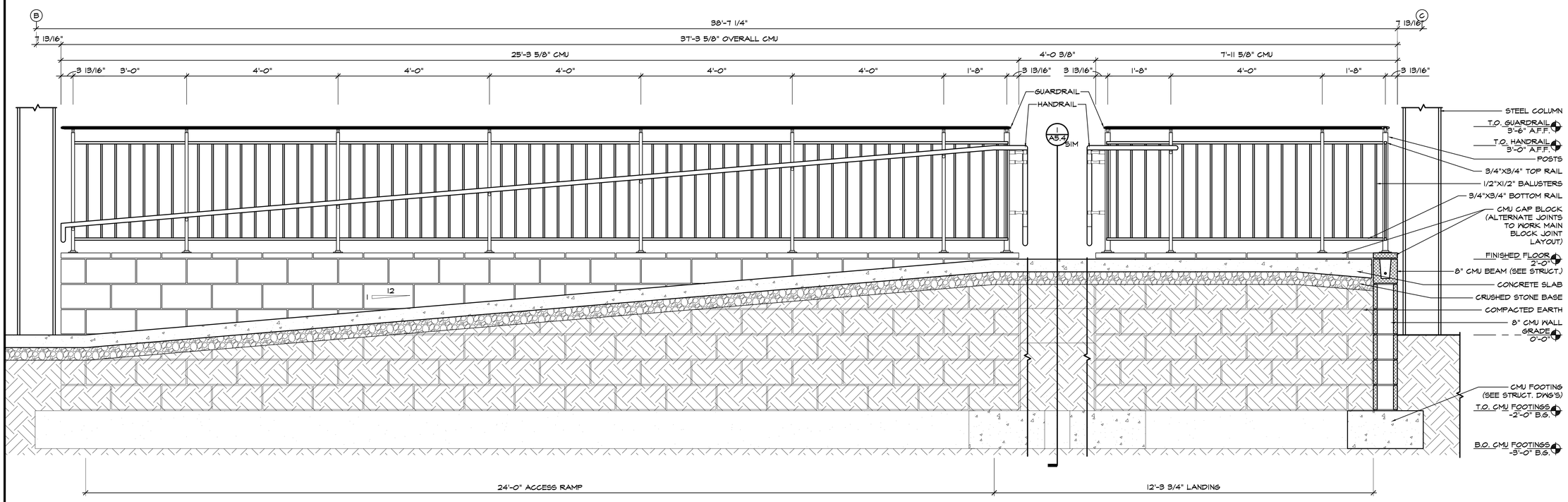
A5.2



1 / A5.3 ENLARGED SECTION
SCALE: 3/4" = 1'-0"

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ENLARGED SECTIONS

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REGISTERED ARCHITECT

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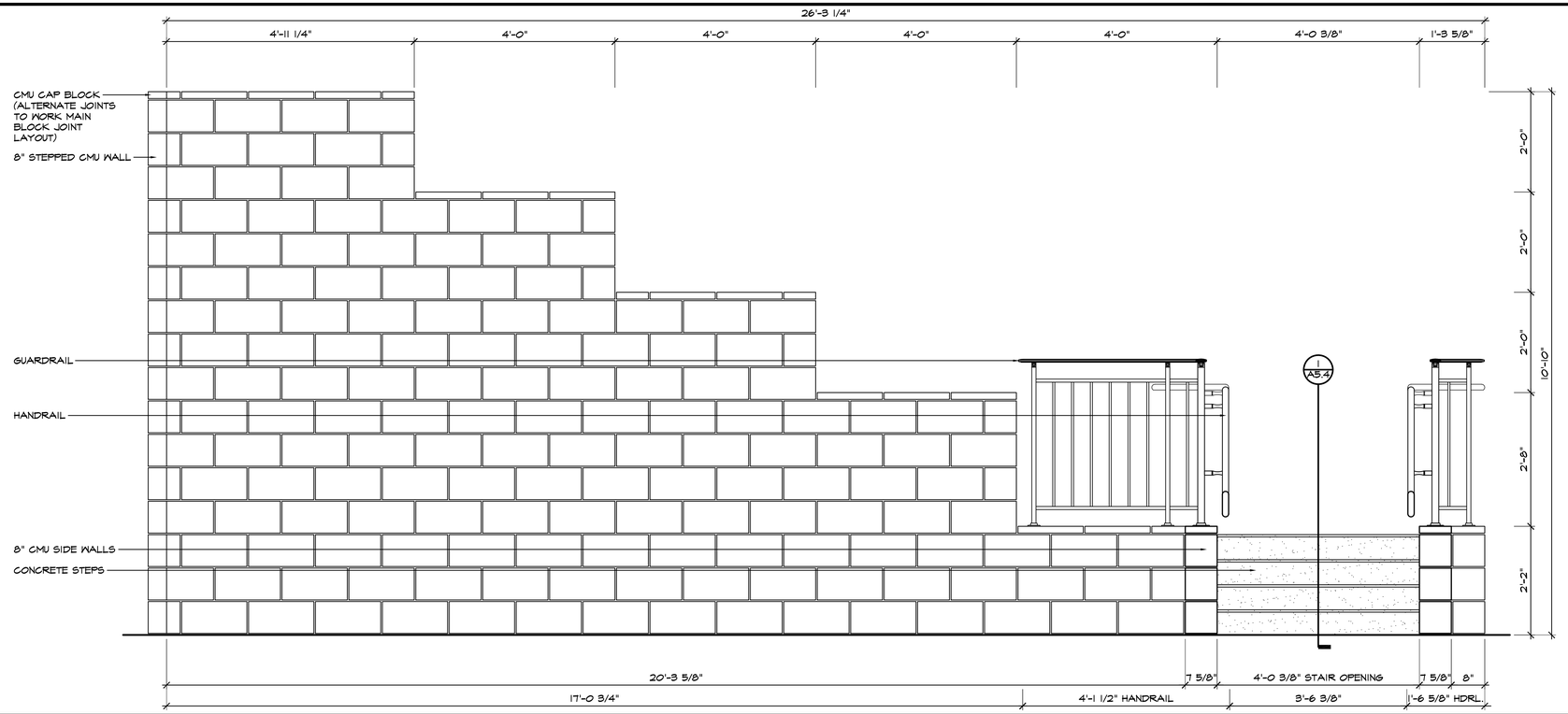
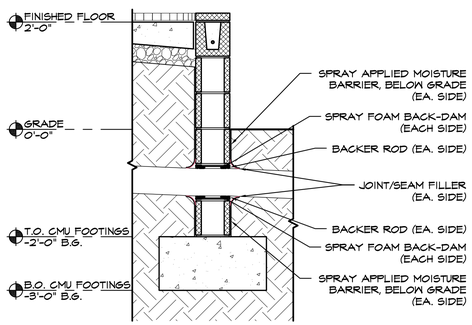
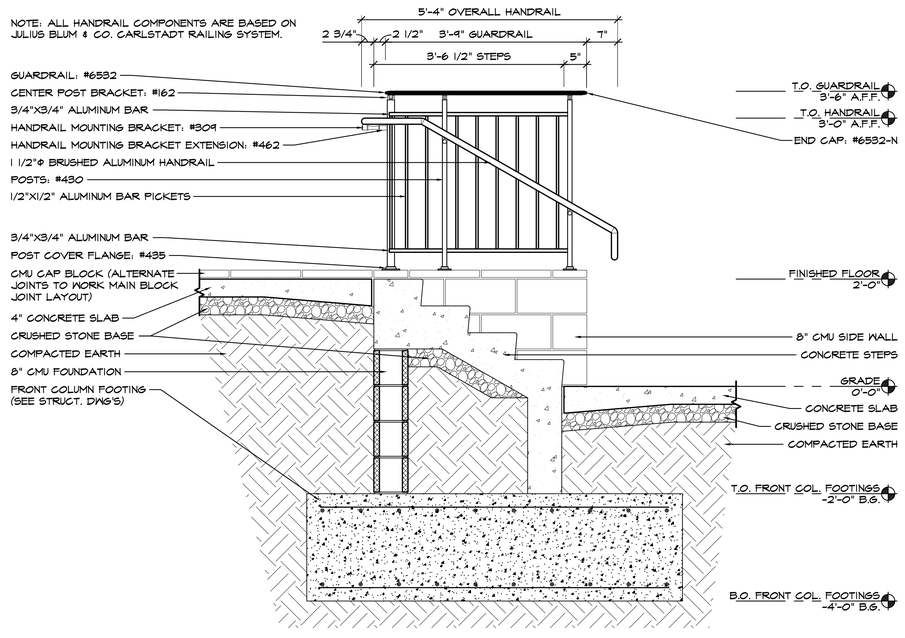
A5.3



3 A5.4 ENLARGED ELEVATION
SCALE: 3/4" = 1'-0"



2 A5.4 ENLARGED SECTION
SCALE: 3/4" = 1'-0"



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A5.4

STRUCTURAL DESIGN CRITERIA

BUILDING CODE
2018 INTERNATIONAL BUILDING CODE (IBC)

DEAD LOAD

DESIGN DEAD LOAD TABLE	
CONSTRUCTION	DEAD LOAD
ROOF	15 PSF

FLOOR LIVE LOAD

FLOOR LIVE LOAD TABLE		
FLOOR USE	UNIFORM LIVE LOADING	CONCENTRATED LIVE LOADING
ASSEMBLY AREAS: STAGE FLOORS	150 PSF	--
MECHANICAL/STORAGE AREAS	150 PSF	--
STAIRS AND EXITWAYS	100 PSF	--

ROOF LIVE LOAD

ROOF LIVE LOAD TABLE		
ROOF TYPE	UNIFORM LIVE LOADING	CONCENTRATED LIVE LOADING
ORDINARY FLAT AND PITCHED ROOF	20 PSF	300 LBS

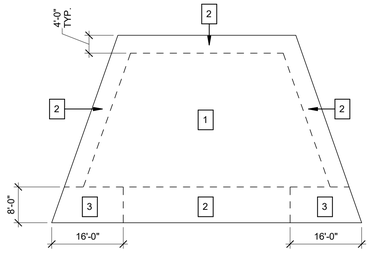
ROOF SNOW LOAD DATA

GROUND SNOW LOAD, p_g = 0 PSF

WIND DESIGN DATA

ULTIMATE DESIGN WIND SPEED, V_{ult} = 111 MPH
NOMINAL DESIGN WIND SPEED, V_{asd} = 86 MPH
RISK CATEGORY = II
WIND EXPOSURE = B
INTERNAL PRESSURE COEFFICIENT, (GCpi) = 0.0 (OPEN)
COMPONENTS AND CLADDING ULTIMATE WIND PRESSURES = (SEE TABLE BELOW)

COMPONENTS & CLADDING ULTIMATE WIND PRESSURES				
ELEMENT	ZONE	AREA (SQ. FT.)	p_{net} (PSF)	
			POSITIVE	NEGATIVE
ROOF	1	64	16.0	-16.7
		128	16.0	-16.7
		256	16.0	-16.7
	2	64	24.3	-25.2
		128	24.3	-25.2
		256	15.9	-16.7
	3	64	31.7	-50.0
		128	24.3	-25.2
		256	16.0	-16.7



TYPICAL ROOF PLAN

EARTHQUAKE DESIGN DATA

RISK CATEGORY = II
SEISMIC IMPORTANCE FACTOR, I_e = 1.00
SITE CLASS = D
S_S = 0.164g
S_1 = 0.075g
S_{DS} = 0.175g
S_{D1} = 0.119g
T_L = 8 sec
SEISMIC DESIGN CATEGORY = B

SEISMIC FORCE RESISTING SYSTEM					
SEISMIC FORCE RESISTING SYSTEM	DETAILING SECTION	R	Ω_0	C_d	h_n LIMIT
G2. STEEL ORDINARY CANTILEVER COLUMN SYSTEMS	14.1	1 1/4	1 1/4	1 1/4	SDC B = 35FT

RESPONSE MODIFICATION COEFFICIENT, R = 1 1/4
ANALYSIS PROCEDURE UTILIZED = EQUIVALENT LATERAL FORCE PROCEDURE (ASCE 7-16 12.8)
SEISMIC RESPONSE COEFFICIENT, C_s = 0.14
SEISMIC BASE SHEAR, V = 4.9 KIPS

GEOTECHNICAL INFORMATION

PROJECT GEOTECHNICAL REPORT = TERRACON, PROJECT NO. ES245082, DATED MAY 1, 2024
ALLOWABLE VERTICAL BEARING PRESSURE = 2,500 PSF (ISOLATED); 2,500 PSF (CONTINUOUS)
MODULUS OF SUBGRADE REACTION = 120 PSI/IN BELOW NATURAL GRADE

FLOOD DESIGN DATA

FLOOD ZONE = X

GENERAL REQUIREMENTS

1. THE INTENT OF THESE DRAWINGS IS TO SHOW ALL ITEMS NECESSARY TO COMPLETE THE FOUNDATION FOR THE GLUED-LAMINATED TIMBER AMPHITHEATER. FOR ITEMS, METHODS AND/OR MATERIALS NOT SHOWN; THE MINIMUM REQUIREMENTS OF THE 2018 INTERNATIONAL BUILDING CODE SHALL GOVERN, AS AMENDED BY THE STATE AND LOCAL GOVERNING AGENCIES OF THE PROJECT LOCATION.
2. ALL DRAWINGS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO START OF CONSTRUCTION SO THAT A CLARIFICATION CAN BE PROVIDED.
3. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT THEIR OWN EXPENSE AND AT NO EXPENSE TO THE OWNER, ARCHITECT OR ENGINEER.
4. DRAWINGS ARE NOT TO BE SCALED. WRITTEN DIMENSIONS SHALL GOVERN CONSTRUCTION. THE CONTRACTOR SHALL VERIFY DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS AND THE SITE CONDITIONS PRIOR TO CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND THE ENGINEER SO THAT CLARIFICATION CAN BE PROVIDED.
5. THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OR SEQUENCE OF CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR AND PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES AND SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO (NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS). THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND IMPLEMENTATION OF ALL SCAFFOLDING, BRACING AND SHORING.
6. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT.
7. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.
8. ANY DELEGATED ENGINEERING DESIGN TO BE PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL MEET THE CRITERIA HEREIN, AND SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT.
9. ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW.
10. NO STRUCTURAL MEMBER SHALL BE CUT OR NOTCHED OR OTHERWISE REDUCED IN STRENGTH UNLESS APPROVED BY THE STRUCTURAL ENGINEER.

STRUCTURAL SPECIAL INSPECTIONS

1. SPECIAL STRUCTURAL TESTS AND INSPECTIONS SHALL BE PERFORMED ON THIS PROJECT IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE (IBC). THE FOLLOWING DOCUMENTS HAVE BEEN PREPARED FOR THIS PROJECT AS A PART OF THESE CONSTRUCTION DOCUMENTS:
   a. STATEMENT OF SPECIAL INSPECTIONS
   b. SCHEDULE OF SPECIAL INSPECTIONS
   c. STATEMENT OF SPECIAL INSPECTIONS REQUIREMENTS FOR WIND RESISTANCE
   d. STATEMENT OF SPECIAL INSPECTIONS REQUIREMENTS FOR SEISMIC RESISTANCE
2. SPECIAL STRUCTURAL TESTS AND INSPECTIONS SHALL BE PERFORMED BY AN AGENCY SELECTED BY THE CONTRACTOR AND APPROVED BY THE ENGINEER OF RECORD. THE AGENCY SHALL MEET ALL OF THE REQUIREMENTS FOR QUALIFICATIONS, INDICATED IN IBC SECTION 1703.1. SPECIAL INSPECTORS SHALL BE QUALIFIED PERSONS WHO SHALL DEMONSTRATE COMPETENCE TO THE SATISFACTION OF THE BUILDING OFFICIAL FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
3. THE CONTRACTOR SHALL COORDINATE THE INSPECTION SERVICES IN ACCORDANCE WITH THE PROGRESS OF THE WORK. THE CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE TO THE INSPECTOR TO ALLOW PROPER SCHEDULING OF PERSONNEL.
4. ALL REPORTS AND SHOP CERTIFICATION OF SPECIAL INSPECTIONS TO BE PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP SHALL BE SUBMITTED TO THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DISTRIBUTING THESE REPORTS TO THE SPECIAL INSPECTOR, THE ARCHITECT, AND THE ENGINEER OF RECORD IN A TIMELY MANNER.
5. THE COSTS OF THE SPECIAL INSPECTOR'S SERVICES SHALL BE PAID FOR BY THE OWNER.SPECIAL INSPECTIONS REPORTS AND A FINAL REPORT IN ACCORDANCE WITH IBC SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.
6. REPORTS SHALL INDICATE THAT THE WORK WAS PERFORMED AND CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. WORK NOT IN CONFORMANCE SHALL BE IDENTIFIED IN THE REPORT AND SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR.
7. A FINAL REPORT OF INSPECTIONS DOCUMENTING REQUIRED SPECIAL INSPECTIONS, INCLUDING ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL, THE ARCHITECT, AND THE ENGINEER OF RECORD PRIOR TO COMPLETION OF THE STRUCTURAL SYSTEMS BUT AT A FREQUENCY NOT TO EXCEED 60 DAYS.

SUBGRADE PREPARATION

1. CONTRACTOR SHALL FOLLOW SITE WORK RECOMMENDATIONS LISTED IN THE PROJECT GEOTECHNICAL REPORT BY TERRACON DATED MAY 1, 2024.

FOUNDATIONS

1. THE FOUNDATION IS DESIGNED BASED UPON THE RECOMMENDATIONS AND DESIGN PARAMETERS INCLUDED IN THE PROJECT GEOTECHNICAL REPORT PREPARED BY TERRACON DATED MAY 1, 2024
2. SOIL PRESSURES USED FOR FOUNDATION DESIGN:
   a. ALLOWABLE BEARING PRESSURE = 2,500 PSF
   b. MODULUS OF SUBGRADE REACTION = 120 PSI/IN
3. ALL FOUNDATIONS SHALL BE PLACED ON COMPACTED SUBGRADE. SEE SUBGRADE PREPARATION NOTES.
4. THE BOTTOM OF ALL EXTERIOR FOUNDATIONS SHALL BE A MINIMUM OF 18 INCHES BELOW FINISHED GRADE UNLESS NOTED OTHERWISE.
5. REMOVE ALL WATER SOFTENED SOILS FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE. FILL REMAINING VOIDS WITH ADDITIONAL CONCRETE.
6. ALL FOUNDATION REINFORCEMENT SHALL BE PROPERLY TIED IN PLACE PRIOR TO PLACEMENT OF CONCRETE.
7. WHERE FINISHED GRADES DIFFER ON OPPOSITE SIDES OF FOUNDATION WALLS, PROVIDE TEMPORARY BRACING TO PREVENT LATERAL MOVEMENT UNTIL ALL ADJACENT FILL, COMPACTION, FLOOR SLABS, AND FRAMING AT NEXT LEVEL HAS BEEN COMPLETED..

REINFORCING STEEL

1. FABRICATING, PLACING, AND SUPPORTING REINFORCEMENT SHALL COMPLY WITH CRSI'S "MANUAL OF STANDARD PRACTICE."
2. REINFORCING BARS SHALL BE ASTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE.
3. REINFORCING BARS IN WELDED CONDITIONS, WHERE PERMITTED, SHALL BE ASTM A 706, DEFORMED.
4. STEEL WELDED-WIRE REINFORCEMENTS SHALL BE ASTM A 1064 WITH 70 KSI MINIMUM YIELD STRENGTH.
5. NO REINFORCEMENT SHALL BE FLAME-CUT OR BENT IN FIELD WITHOUT GUIDANCE FROM STRUCTURAL ENGINEER.
6. REINFORCING STEEL SHALL HAVE COVER PROTECTION AS FOLLOWS:

CONCRETE COVER PROTECTION TABLE	
CONDITION	MINIMUM COVER
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:	
WALL PANELS, SLABS, JOISTS	1 INCH
OTHER MEMBERS	1½ INCHES
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS	¾ INCHES
BEAMS, COLUMNS:	
PRIMARY REINFORCEMENT	1½ INCHES
TIES, STIRRUPS, SPIRALS	1 INCH

SLABS ON GRADE

1. ALL SLABS ON GRADE SHALL BE ON COMPACTED SUBGRADE WITH 4 INCHES MINIMUM OF POROUS FILL MATERIAL. SEE SUBGRADE PREPARATION NOTES.
   ALL SLABS ON GRADE SHOULD BE SUPPORTED ON A MINIMUM OF 4-INCHES OF GRANULAR, FREE-DRAINING POROUS FILL WITH A VAPOR BARRIER AS A CAPILLARY LAYER BETWEEN THE SLAB AND THE SUBGRADE.
2. THE CONTRACTOR SHALL COORDINATE ALL LIMITS AND DEPTHS OF DEPRESSIONS FOR FLOOR FINISHES WITH ARCHITECTURAL DRAWINGS AND SCHEDULES. LIMITS SHOWN ON STRUCTURAL DRAWINGS ARE SCHEMATIC.
3. THE USE OF POLYPROPYLENE FIBERS IN LIEU OF WELDED WIRE FABRIC IS PROHIBITED WITHOUT THE WRITTEN AUTHORIZATION OF THE ENGINEER.
4. THE FINISH TOLERANCE OF ALL SLABS SHALL BE IN ACCORDANCE WITH ACI 301, TYPE A.
5. SLABS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE FOLLOWING FLATNESS / LEVELNESS REQUIREMENTS:
   a. FF = 35
   b. FL = 25
7. FF AND FL TOLERANCES SHALL BE TESTED IN ACCORDANCE WITH ASTM E 1155. ACTUAL OVERALL F-NUMBERS SHALL BE CALCULATED USING THE INFERIOR / SUPERIOR AREA METHOD. ALL FLOOR TOLERANCE MEASUREMENTS SHALL BE MADE WITHIN 48 HOURS AFTER SLAB INSTALLATION. IN ALL CASES, TOLERANCE MEASUREMENTS SHALL PRECEDE THE REMOVAL OF SHORES AND FORMS. RESULTS OF ALL FLOOR PROFILE TESTS INCLUDING A RUNNING TABULATION OF THE OVERALL FF AND FL VALUES FOR ALL OF THE RANDOM TRAFFIC SLABS INSTALLED TO DATE SHALL BE PROVIDED TO THE CONTRACTOR WITHIN 72 HOURS AFTER EACH SLAB INSTALLATION.
8. WALKWAYS AND OTHER EXTERIOR SLABS ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS. SEE THE SITE PLAN AND ARCHITECTURAL DRAWINGS FOR LOCATIONS, DIMENSIONS, ELEVATIONS, JOINTING DETAILS AND FINISH DETAILS.
9. SAW-CUT CONTRACTION JOINTS SHALL BE CUT AS SOON AS THE CONCRETE CAN BE CUT WITHOUT RAVELING. CONVENTIONAL CAW-CUT JOINTS SHOULD BE RUN WITHIN 4-12 HOURS AFTER THE CONCRETE HAS BEEN FINISHED. JOINTS PLACED WITH AN EARLY ENTRY SAW MAY BE CUT 1-4 HOURS AFTER THE SLAB HAS BEEN FINISHED.
10. SLAB JOINTS SHALL BE FILLED WITH APPROVED MATERIAL. THIS SHOULD TAKE PLACE AS LATE AS POSSIBLE PRIOR TO FILLING, REMOVE ALL DEBRIS FROM THE SLAB JOINTS, THEN FILL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AS FOLLOWS:
    a. CONTROL JOINT SEALANT = EUCLID CHEMICAL DURAL 340 SL OR APPROVED EQUAL
    b. EXPANSION/ISOLATION JOINT SEALANT = EUCLID CHEMICAL EUCOLASTIC 1 SL OR APPROVED EQUAL
11. CONCRETE FINISH FLOORS SHALL HAVE A HARD STEEL TROWELED FINISH UNLESS INDICATED OTHERWISE ON THE DRAWINGS. PLACE, STRIKE OFF, CONSOLIDATE, LEVEL AND FLOAT TO THE PROPER ELEVATION. TROWELING SHALL BEGIN AFTER SURFACE HAS RECEIVED A FLOAT FINISH. THE SLAB DRYING MUST PROCEED NATURALLY AND MUST NOT BE HASTENED BY THE DUSTING ON OF DRY CEMENT OR SAND. LIGHTLY TOOL ALL EDGES AT CONSTRUCTION JOINTS AND EXERCISE CARE THAT SLAB EDGES ARE NOT DEPRESSED ALONG BULKHEADS DURING FINISHING OPERATIONS, PARTICULARLY HAND TROWELING. EXTERIOR SLABS, SIDEWALKS, PADS AND RAMPS SHALL HAVE A LIGHT BROOM FINISH UNLESS INDICATED OTHERWISE ON THE DRAWINGS. PROVIDE STANDARD TROWEL FINISH AT ALL SUB-SLABS.
12. PROVIDE 1/2" PREMOLDED EXPANSION JOINT (P.E.J.) FILLER AROUND PERIMETER OF SLABS WHERE THEY ABUT VERTICAL SURFACES AND AT COLUMN ISOLATION JOINTS AS DETAILED.

CAST-IN-PLACE CONCRETE

1. ALL CAST-IN-PLACE CONCRETE CONSTRUCTION SHALL COMPLY WITH THE FOLLOWING ACI PUBLICATIONS:
   a. ACI 301-14 - GENERAL CONSTRUCTION REQUIREMENTS
   b. ACI 117-14 - TOLERANCES FOR CONCRETE CONSTRUCTION
2. CONCRETE SHALL BE NORMAL-WEIGHT CONCRETE (145 PCF) WITH MIXES MEETING THE FOLLOWING CRITERIA:
   a. FOUNDATION ELEMENTS & SLAB ON GRADE
      - EXPOSURE CLASS = F0
      - MINIMUM 28-DAY COMPRESSIVE STRENGTH = 4000 PSI
      - MAXIMUM WATER-TO-CEMENTITOUS MATERIALS RATIO = 0.50
      - SLUMP LIMIT = 5 INCHES (±1 INCH)
      - NOMINAL MAXIMUM AGGREGATE SIZE = 1 INCH
      - AIR CONTENT = 4.5% (+/-1.5%)
3. ACCEPTABLE CEMENTIOUS MATERIALS:
   a. PORTLAND CEMENT - ASTM C 150, TYPE II
   b. FLY ASH - ASTM C 618 (NOT PERMITTED FOR TILTUP WALL PANELS)
   c. SLAG CEMENT - ASTM C989
   d. BLENDED HYDRAULIC CEMENT - ASTM C 595, TYPE IS OR TYPE IP
4. ALL EXPOSED CONCRETE EDGES SHALL HAVE A 3/4-INCH CHAMFER.
5. OPENINGS, POCKETS, ETC., LARGER THAN 6" SHALL NOT BE PLACED IN CONCRETE SLABS, DECKS, WALLS, UNLESS SPECIALLY DETAILED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW OPENINGS, POCKETS, ETC., LARGER THAN 6" NOT SHOWN ON THE STRUCTURAL DRAWINGS, BUT WHICH ARE LOCATED IN STRUCTURAL MEMBERS. FOR ANY FURTHER RESTRICTIONS ON OPENINGS IN STRUCTURAL ELEMENTS, SEE APPLICABLE SECTIONS BELOW.
6. PIPES LARGER THAN 1 ½" DIAMETER SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE EXCEPT WHEN WHERE SPECIFICALLY APPROVED. NO CONDUITS SHALL BE PLACED IN CONCRETE FILL OVER METAL DECKING.
7. REQUIRED CAST-IN-PLACE CONCRETE SUBMITTALS TO ENGINEER:
   a. PRODUCT DATA - SUBMIT TECHNICAL PRODUCT DATA FOR ANY ADMIXTURES OR CONCRETE-RELATED CONSTRUCTION PRODUCTS.
   b. DESIGN MIXTURES - THE FOLLOWING ITEMS ARE REQUIRED:
      - MIX IDENTIFICATION BY MEANS OF CLASS OR LOCATION WHERE MIX WILL BE USED.
      - STRENGTH OF CONCRETE.
      - TARGET SLUMP, WATER-TO-CEMENT RATIO, DENSITY, AND AIR CONTENT.
      - LIST OF ALL MATERIALS, ADMIXTURES, AND ADDITIVES ALONG WITH THEIR PROPORTIONS.
      - NOMINAL MAXIMUM AGGREGATE SIZE AND COMBINED AGGREGATE GRADATION.
      - CALCULATIONS AND TEST RESULTS REQUIRED BY ACI 318-14 CHAPTER 26
      - TEST RESULTS OF TOTAL CHLORIDE CONTENT.
      - INFORMATION ON CONCRETE MATERIALS AS PER ACI 301-14 SECTION 26.4
      - TEST RESULTS PER ASTM C33, INCLUDING THE CLEANNESS VALUE, SAND EQUIVALENT, AND ALKALI-SILICA REACTIVITY (ASR) POTENTIAL AND MITIGATION, IF REQUIRED.
      - MILL CERTIFICATE FOR THE CEMENT INDICATING THE SOURCE OF THE CEMENT AND COMPLIANCE WITH THE PROJECT SPECIFICATION.
      - MILL ANALYSIS FOR SUPPLEMENTARY CEMENTITIOUS MATERIALS (INCLUDING FLY ASH AND SLAG CEMENT) AND AGGREGATES FROM THE MANUFACTURER.
      - CERTIFICATION BY THE MANUFACTURERS THAT THE ADMIXTURES CONFORM TO THE SPECIFIED STANDARDS.
      - WHETHER MIX IS APPROPRIATE FOR PUMPING.
      - THERMAL CONTROL PLAN, INCLUDING HOT WEATHER AND COLD WEATHER PLACEMENT.
   c. STEEL REINFORCEMENT SHOP DRAWINGS - PLACING DRAWINGS THAT DETAIL FABRICATION, BENDING, AND PLACEMENT OF REINFORCEMENT.
   d. 28-DAY CONCRETE ACCEPTANCE TEST REPORT AS REQUIRED BY ACI 318-14 SECTION 26.13.2

CONCRETE UNIT MASONRY

1. ALL CONCRETE UNIT MASONRY CONSTRUCTION SHALL COMPLY WITH THE FOLLOWING TMS PUBLICATIONS:
   a. TMS 402-16 - BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
   b. TMS 602-16 - SPECIFICATION FOR MASONRY STRUCTURES
2. CONCRETE MASONRY UNITS SHALL BE ASTM C 90 AND MEET THE FOLLOWING THE FOLLOWING CRITERIA:
   a. UNIT COMPRESSIVE STRENGTH = 1900 PSI
   b. DENSITY CLASSIFICATION = LIGHTWEIGHT
   c. RUNNING BOND PLACEMENT
3. MORTAR SHALL COMPLY WITH ASTM C 270 AND THE FOLLOWING CRITERIA:
   a. TYPE = S
   b. ACCEPTABLE MORTAR CEMENT:
      - PORTLAND CEMENT - ASTM C 150, TYPE II
      - HYDRATED LIME - ASTM C 207, TYPE S
      - PORTLAND CEMENT-LIME MIX
      - MASONRY CEMENT - ASTM C 91
   c. BEDDING = FULL
   d. MORTAR AGGREGATE, ASTM C 144
4. GROUT FOR UNIT MASONRY SHALL COMPLY WITH ASTM C 476 AND THE FOLLOWING CRITERIA:
   a. GROUT TYPE = COARSE
   b. GROUT DENSITY = 140 PCF
   c. COMPRESSIVE STRENGTH = 3000 PSI
   d. GROUT AGGREGATE = 1/2-INCH MAX COARSE AGGREGATE, ASTM C 404
   e. TARGET SLUMP = 9 INCHES (1 INCH)
   f. NO ADMIXTURES UNLESS APPROVED BY ENGINEER
5. MASONRY-JOINT REINFORCEMENT, WHERE INDICATED, SHALL COMPLY WITH ASTM A 951 AND WITH THE FOLLOWING CRITERIA:
   a. HOT-DIP GALVANIZED STEEL
   b. WIRE SIZE FOR SIDE RODS = 0.148 INCHES
   c. WIRE SIZE RODS FOR CROSS RODS = 0.148 INCHES
   d. SPACING OF CROSS RODS = 16 INCHES
   e. PROVIDE IN LENGTHS OF NOT LESS THAN 10 FEET WITH PREFABRICATED CORNER AND TEE UNITS
   f. CONTINUOUS THROUGH VERTICALLY-REINFORCED CELLS
6. ALL CELLS BELOW GROUND FLOOR SLAB SHALL BE FILLED SOLID WITH GROUT.
7. CONCRETE MASONRY UNITS SHALL BE FILLED IN 4-FEET LIFTS MAXIMUM.
8. PLAIN END TWO CELL UNITS SHALL BE USED FOR BLOCKS THAT ARE TO HAVE CELLS REINFORCED OR FILLED.
9. PROVIDE A 4-INCH BY 4-INCH CLEAN-OUT OPENING AT BOTTOM COURSE OF EACH LIFT AT EACH REINFORCED CELL EXCEPT WHERE HOLE OR PATCH CANNOT BE CONCEALED BY BRICK OR WALL FINISH.
10. SEE ARCHITECTURAL DRAWINGS FOR THE EXTENT AND LOCATION OF MASONRY WALLS.
11. ALL MASONRY WALLS SHOWN HEREIN HAVE BEEN DESIGNED TO RESIST THE REQUIRED VERTICAL AND LATERAL FORCES IN THE FINAL CONFIGURATION ONLY. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO ADEQUATELY BRACE THE WALLS FOR VERTICAL AND LATERAL LOADS THAT COULD POSSIBLY BE APPLIED PRIOR TO COMPLETION OF LATERAL SUPPORT BY CONNECTIONS AT FLOOR OR ROOF FRAMING LOCATIONS.
12. REQUIRED CONCRETE UNIT MASONRY SUBMITTALS TO ENGINEER:
   a. PRODUCT DATA - SUBMIT TECHNICAL PRODUCT DATA FOR ANY CONCRETE MASONRY-RELATED CONSTRUCTION PRODUCTS.
   b. MATERIAL CERTIFICATES - FOR EACH TYPE AND SIZE OF UNIT, SUBMIT DATA ON MATERIAL PROPERTIES AND MATERIAL TEST REPORTS TO SUBSTANTIATE COMPLIANCE WITH REQUIREMENTS.
   c. DESIGN MIXTURES - THE FOLLOWING ITEMS ARE REQUIRED:
      - TEST REPORTS FOR MORTAR MIXES REQUIRED TO COMPLY WITH PROPERTY SPECIFICATION. TEST ACCORDING TO ASTM C 109 FOR COMPRESSIVE STRENGTH, ASTM C 1506 FOR WATER RETENTION, AND ASTM C 91 FOR AIR CONTENT.
      - TEST REPORTS FOR GROUT MIXES REQUIRED TO COMPLY WITH COMPRESSIVE STRENGTH REQUIREMENT ACCORDING TO ASTM C 1019.
   d. STEEL REINFORCEMENT SHOP DRAWINGS - PLACING DRAWINGS THAT DETAIL FABRICATION, BENDING, AND PLACEMENT OF REINFORCEMENT.
   e. CONTROL JOINT LOCATION PLAN AND DETAIL

POST-INSTALLED REBAR, ANCHORS, AND FASTENERS

1. THE PRODUCTS BELOW ARE THE DESIGN BASIS FOR THIS PROJECT. PRODUCT DIAMETER AND EMBEDMENT SHALL BE AS SHOWN IN THE DETAILS. INSTALL PRODUCTS IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). CONTRACTOR SHALL CONTACT MANUFACTURER'S REPRESENTATIVE FOR PRODUCT INSTALLATION TRAINING AND A LETTER SHALL BE SUBMITTED TO THE ENGINEER OF RECORD INDICATING THAT TRAINING HAS TAKEN PLACE. REFER TO THE PROJECT BUILDING CODE AND/OR EVALUATION REPORT FOR SPECIAL INSPECTIONS AND PROOF LOAD REQUIREMENTS. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW MAY BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER OF RECORD FOR REVIEW. SUBSTITUTIONS WILL ONLY BE CONSIDERED FOR PRODUCTS HAVING A RESEARCH REPORT RECOGNIZING THE PRODUCT FOR THE APPROPRIATE APPLICATION UNDER THE PROJECT BUILDING CODE. SUBSTITUTION REQUESTS SHALL INCLUDE CALCULATIONS THAT DEMONSTRATE THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE EQUIVALENT PERFORMANCE VALUES OF THE DESIGN BASIS PRODUCT.

FOR ANCHORING INTO SOLID GROUTED CONCRETE MASONRY:

1. MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC01 OR ICC-ES AC106. PRE-APPROVED PRODUCTS INCLUDE:
   SCREW ANCHORS
   a. SIMPSON STRONG-TIE TITEN-HD (ICC-ES ESR-1056)
   b. DEWALT SCREW-BOLT+ (ICC-ES ESR-4042)
   EXPANSION ANCHORS
   c. SIMPSON STRONG-TIE STRONG-BOLT 2 (IAPMO-UES ER-240)
   d. DEWALT POWER-STUD+ SD1 (ICC-ES ESR-2966)
   e. HILTI KWIK BOLT-3 (ICC-ES ESR-1385)
2. ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC58. PRE-APPROVED PRODUCTS INCLUDE:
   a. SIMPSON STRONG-TIE AT-XP (IAPMO-UES ER-263)
   b. SIMPSON STRONG-TIE SET-XP (IAPMO-UES ER-2508)
   c. DEWALT AC100+ GOLD (ICC-ES ESR-3200)
   d. HILTI HIT-HY 70 (ICC-ES ESR-2682)
3. POWER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED PRODUCTS INCLUDE:
   a. SIMPSON STRONG-TIE GAS ACTUATED PINS (ICC-ES ESR-2811)
   b. SIMPSON STRONG-TIE POWDER ACTUATED PINS (ICC-ES ESR-2138)
   c. DEWALT GAS ACTUATED FASTENERS (ICC-ES ESR-3275)
   d. DEWALT POWDER ACTUATED FASTENERS (ICC-ES ESR-2024)
   e. HILTI X-U (ICC-ES ESR-2269)

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GEORGIA REGISTERED 2/13/25 No. SE000802 STRUCTURAL ENGINEER BRIAN K. SAPP

REVISIONS

#	DATE	DESCRIPTION

M.E. SACK ENGINEERING

EAST SIDE PARK AMPHITHEATER

5 WEST MAIN STREET, ALAMO, GEORGIA 30411

PROJECT NO.	24.076
DATE	02/13/25
DRAWN BY	MPD
CHECKED BY	BKS

STRUCTURAL NOTES

S0.1

ROUGH CARPENTRY AND GLULAM

1. ALL WOOD FRAMING SHALL COMPLY WITH THE NATIONAL FOREST PRODUCTS ASSOCIATION'S NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS).
2. ALL WOOD FRAMING MEMBERS ARE DESIGNED TO ACT AS A SYSTEM ONCE CONSTRUCTION IS COMPLETE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE SAFETY AND STABILITY OF WOOD FRAMING SYSTEMS (I.E. TEMPORARY BRACING) DURING CONSTRUCTION AS A RESULT OF CONSTRUCTION METHODS AND SEQUENCES.
3. WOOD FRAMING MATERIALS:
   a. DIMENSIONAL LUMBER SHALL BE #2 SOUTHERN YELLOW PINE.
   b. GLUED-LAMINATED TIMBER BEAMS AND GIRDERS SHALL BE FABRICATED USING SOUTHERN PINE AND SHALL COMPLY WITH A 24F-1.7E STRESS CLASSIFICATION.
   c. TONGUE AND GROOVE DECK SHALL BE MINIMUM #2 SOUTHERN PINE.
4. GLUED-LAMINATED TIMBER MANUFACTURER SHALL BE AITC- OR APA-EWS-LICENSED.
5. DELIVERY, STORAGE, AND HANDLING OF GLUED-LAMINATED TIMBER ELEMENTS SHALL COMPLY WITH THE PROVISIONS OF AITC 111.
6. ALL GLUED-LAMINATED TIMBER ELEMENTS SHOULD BE INDIVIDUALLY WRAPPED USED PLASTIC-COATED PAPER WITH WATER-RESISTANT SEAMS PRIOR TO INSTALLATION.
7. ALL GLUED-LAMINATED TIMBER ELEMENTS SHALL COMPLY WITH ANSI A190.1 AND ANSI 117 OR RESEARCH/EVALUATION REPORTS ACCEPTABLE TO AUTHORITIES HAVING JURISDICTION.
8. EACH PIECE OF GLUED-LAMINATED TIMBER SHOULD BE FACTORY MARKED WITH AN AITC QUALITY MARK OR APA-EWS TRADEMARK ON A SURFACE THAT WILL NOT BE EXPOSED IN THE COMPLETED WORK.
9. ALL WOOD FRAMING MATERIAL SHALL BE SURFACED DRY AND USED AT 19% MAXIMUM MOISTURE CONTENT.
10. GLUED-LAMINATED TIMBER ELEMENTS SHOULD BE FABRICATED WITH WET-USE ADHESIVE COMPLYING WITH ANSI A190.1.
11. GLUED LAMINATED TIMBER ELEMENTS SHALL BE FABRICATED WITH A BALANCED LAY-UP.
12. APPEARANCE GRADE FOR GLUED-LAMINATED TIMBER ELEMENTS SHALL BE ARCHITECTURAL GRADE COMPLYING WITH AITC 110 UNLESS SPECIFIED OTHERWISE IN ARCHITECTURAL DRAWINGS.
13. GLUED-LAMINATED TIMBER ELEMENTS SHALL BE PROPERLY SEALED BY A TRANSPARENT AND PENETRATING WOOD SEALER THAT IS COMPATIBLE WITH THE FINISH INDICATED ON THE ARCHITECTURAL DRAWINGS.
5. ALL DIMENSIONAL LUMBER EXPOSED TO EXTERIOR ENVIRONMENT OR IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED TO A MINIMUM RETENTION OF 0.25 lbs. OF ACQ PER CUBIC FOOT OF WOOD, AND EACH PIECE SHALL BEAR THE THIRD PARTY QUALITY MARK, "ABOVE GRADE USE". ALL LUMBER IN CONTACT WITH THE GROUND SHALL BE PRESSURE TREATED TO A MINIMUM RETENTION OF 0.40 lbs. OF ACQ PER CUBIC FOOT OF WOOD, AND EACH PIECE SHALL BEAR THE THIRD PARTY QUALITY MARK, "GROUND CONTACT USE". REFERENCE STANDARD AWPA C2 AND ASTM D1760 FOR PRESSURE TREATMENT OF TIMBER PRODUCTS.
6. WHERE POSSIBLE ALL CUTS AND HOLES SHOULD BE COMPLETED BEFORE TREATMENT. CUTS AND HOLES DUE TO ON-SITE FABRICATION SHALL BE BRUSHED WITH 2 COATS OF COPPER NAPHTHENATE SOLUTION CONTAINING A MINIMUM OF 2% METALLIC COPPER IN SOLUTION (PER AWPA STD. M4).
7. THE CONTRACTOR SHALL CAREFULLY SELECT LUMBER TO BE USED IN LOAD BEARING APPLICATIONS. THE LENGTH OF SPLIT ON THE WIDE FACE OF 2" NOMINAL LOAD BEARING FRAMING SHALL BE LIMITED TO LESS THAN ½ OF THE WIDE FACE DIMENSION. THE LENGTH OF SPLIT ON THE WIDE FACE OF 3" (NOMINAL) AND THICKER LUMBER SHALL BE LIMITED TO ⅓ OF THE NARROW FACE DIMENSIONS.
8. LOCATION, NUMBER, AND DIMENSIONS OF FRAMING MEMBERS SHOW GENERAL ARRANGEMENT ONLY. ACTUAL SPANS, SPACINGS, ETC. SHALL BE DETERMINED FROM ARCHITECTURAL DETAILS.
9. SEE ARCHITECTURAL PLANS AND DETAILS FOR ALL NON-STRUCTURAL FRAMING AND TRIM.
10. FOR ALL JOISTS AND RAFTERS, PROVIDE FULL-DEPTH BLOCKING AT ENDS, AT MIDSPAN, AND AT A MAXIMUM SPACING OF 8 FEET IN BETWEEN.
11. ALL ENGINEERED WOOD PRODUCTS SHALL BE BRIDGED, BLOCKED, AND BRACED IN CONFORMANCE WITH THE MANUFACTURERS RECOMMENDATION.
12. NO CUTS, HOLES, OR COPES IN STRUCTURAL WOOD FRAMING SHALL BE PERMITTED WITHOUT PRIOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER AND ARCHITECT.
13. ALL BOLTS, CARRIAGE BOLTS, LAG SCREWS, EXPANSION BOLTS AND EPOXY BOLTS SHALL BE INSTALLED WITH STANDARD CUT WASHERS UNDER THE BOLT HEADS AND NUTS THAT BEAR DIRECTLY ON THE WOOD. ALL NUTS SHALL BE TIGHTENED AT THE TIME OF INSTALLATION AND RE-TIGHTENED IF NECESSARY, DUE TO WOOD SHRINKAGE, PRIOR TO CLOSE-IN OR A THE COMPLETION OF THE PROJECT. BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1. WOOD SCREWS SHALL CONFORM TO B18.6.1. ALL BOLTS SHALL CONFORM TO ASTM A307 GRADE A UNLESS NOTED OTHERWISE. THE MINIMUM STRENGTHS FOR LAG SCREWS AND WOOD SCREWS SHALL BE AS FOLLOWS:

SCREW YIELD STRENGTH		
SCREW DIAMETER	SCREW TYPE	YIELD STRENGTH
#6 (0.138 IN.)	WOOD SCREW	100 KSI
#7 (0.151 IN.)	WOOD SCREW	90 KSI
#8 (0.164 IN.)	WOOD SCREW	90 KSI
#9 (0.177 IN.)	WOOD SCREW	90 KSI
#10 (0.190 IN.)	WOOD SCREW	80 KSI
#12 (0.216 IN.)	WOOD SCREW	80 KSI
#14 (0.246 IN.)	WOOD SCREW	70 KSI
¼ IN.	LAG SCREW	70 KSI
5/16 IN.	LAG SCREW	60 KSI
⅜ IN. AND GREATER	LAG SCREW	45 KSI

14. BOLT HOLES SHALL BE CAREFULLY CENTERED AND DRILLED NOT MORE THAN 1/16" LARGER THAN THE BOLT DIAMETER. BOLTED CONNECTIONS SHALL BE SNUG TIGHT BUT NOT TO THE EXTENT OF CRUSHING WOOD UNDER WASHERS.
15. ALL PLATES, ANCHORS, BOLTS, NUTS, WASHERS, AND OTHER MISCELLANEOUS HARDWARE SHALL BE HOT DIP GALVANIZED IN PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOODS.
16. BOLTS, SCREWS, AND PINS USED IN CONJUNCTION WITH A MANUFACTURED CONNECTOR SHALL FOLLOWER MANUFACTURER'S WRITTEN INSTRUCTIONS REGARDING INSTALLATION.
18. ALL NAILS FOR STRUCTURAL WORK SHALL BE COMMON WIRE NAILS UNLESS NOTED OR DETAILED OTHERWISE MEETING ASTM F1667. HOLES SHALL BE PRE-DRILLED WHERE NECESSARY TO PREVENT SPLITTING. NAILS SHALL HAVE THE MINIMUM PROPERTIES SPECIFIED IN THE TABLE BELOW.

NAIL YIELD STRENGTH	
NAIL SIZE	YIELD STRENGTH
6d (0.133 IN. x 2 IN.)	100 KSI
8d (0.131 IN. x 2½ IN.)	90 KSI
10d (0.148 IN. x 3 IN.)	90 KSI
12d (0.148 IN. x 3¼ IN.)	90 KSI
16d (0.162 IN. x 3½ IN.)	80 KSI
20d (0.192 IN. x 4 IN.)	80 KSI

20. CUTTING AND NOTCHING OF SAWN LUMBER JOISTS, SAWN LUMBER RAFTERS AND STUDS SHALL BE IN CONFORMANCE WITH THE FOLLOWING CRITERIA:
   a. RAFTERS - NOTCHING AT THE ENDS OF RAFTERS OR CEILING JOISTS SHALL NOT EXCEED 1/5TH THE DEPTH. NOTCHES IN THE TOP OR BOTTOM OF THE RAFTER OR CEILING JOIST SHALL NOT EXCEED 1/6TH THE DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE 1/3RD OF THE SPAN, EXCEPT THAT A NOTCH NOT EXCEEDING 1/3RD OF THE DEPTH IS PERMITTED IN THE TOP OF THE RAFTER OR CEILING JOIST NOT FURTHER FROM THE FACE OF THE SUPPORT THAN THE DEPTH OF THE MEMBER. HOLES BORED IN RAFTERS OR CEILING JOISTS SHALL NOT BE 2½ INCHES OF THE TOP AND BOTTOM AND THEIR DIAMETER SHALL NOT EXCEED 1/4TH THE DEPTH OF THE MEMBER.

WOOD STRUCTURAL PANELS

1. ALL WOOD STRUCTURAL PANELS SHALL BE APA TRADEMARKED STRUCTURAL-USE PANELS QUALIFIED AND MANUFACTURED IN ACCORDANCE WITH APA PRP-108 (PERFORMANCE STANDARDS AND QUALIFICATION POLICY FOR STRUCTURAL-USE PANELS) AND U.S. DEPARTMENT OF COMMERCE VOLUNTARY PRODUCT STANDARDS PS 1-09 (STRUCTURAL PLYWOOD) AND PS 2-04 (PERFORMANCE STANDARD FOR WOOD-BASED STRUCTURAL-USE PANELS) AND SHALL BE IDENTIFIED BY THE MARK OF AN APPROVED TESTING AND GRADING AGENCY.

WOOD STRUCTURAL PANELS					
USAGE	THICKNESS	CONSTRUCTION	BOND CLASSIFICATION	PANEL GRADE	SPAN RATING
ROOF	½ IN.	OSB/PLYWOOD	EXP. 1	SHEATHING	32/16

2. INSTALL ALL PANELS AT ROOF WITH THE LONG DIMENSIONS OF THE PANEL ACROSS SUPPORTS AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. STAGGER PANEL END JOINTS. ALLOW 1/8" SPACING AT PANEL ENDS AND EDGES UNLESS OTHERWISE RECOMMENDED BY THE PANEL MANUFACTURER
5. ALL NAILS FOR PANEL ATTACHMENT SHALL NOT BE OVERDRIVEN.
6. WHERE EITHER 2" OR 2 1/2" FASTENER SPACINGS ARE USED FOR WOOD STRUCTURAL PANELS USED AT ROOF OR FLOOR, THE FRAMING MEMBER AT THE ADJOINING PANEL SHALL BE 3" NOMINAL WIDTH AND THE NAILS AT PANEL EDGES SHALL BE STAGGERED IN TWO LINES.
7. NAILS AT ABUTTING PANEL EDGES MUST PENETRATE THE SAME PIECE OF FRAMING OR BLOCKING.

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GEORGIA REGISTERED 2/13/25 No. SE000802 STRUCTURAL ENGINEER BRIAN K. SAPP

REVISIONS

#	DATE	DESCRIPTION

M.E. SACK ENGINEERING

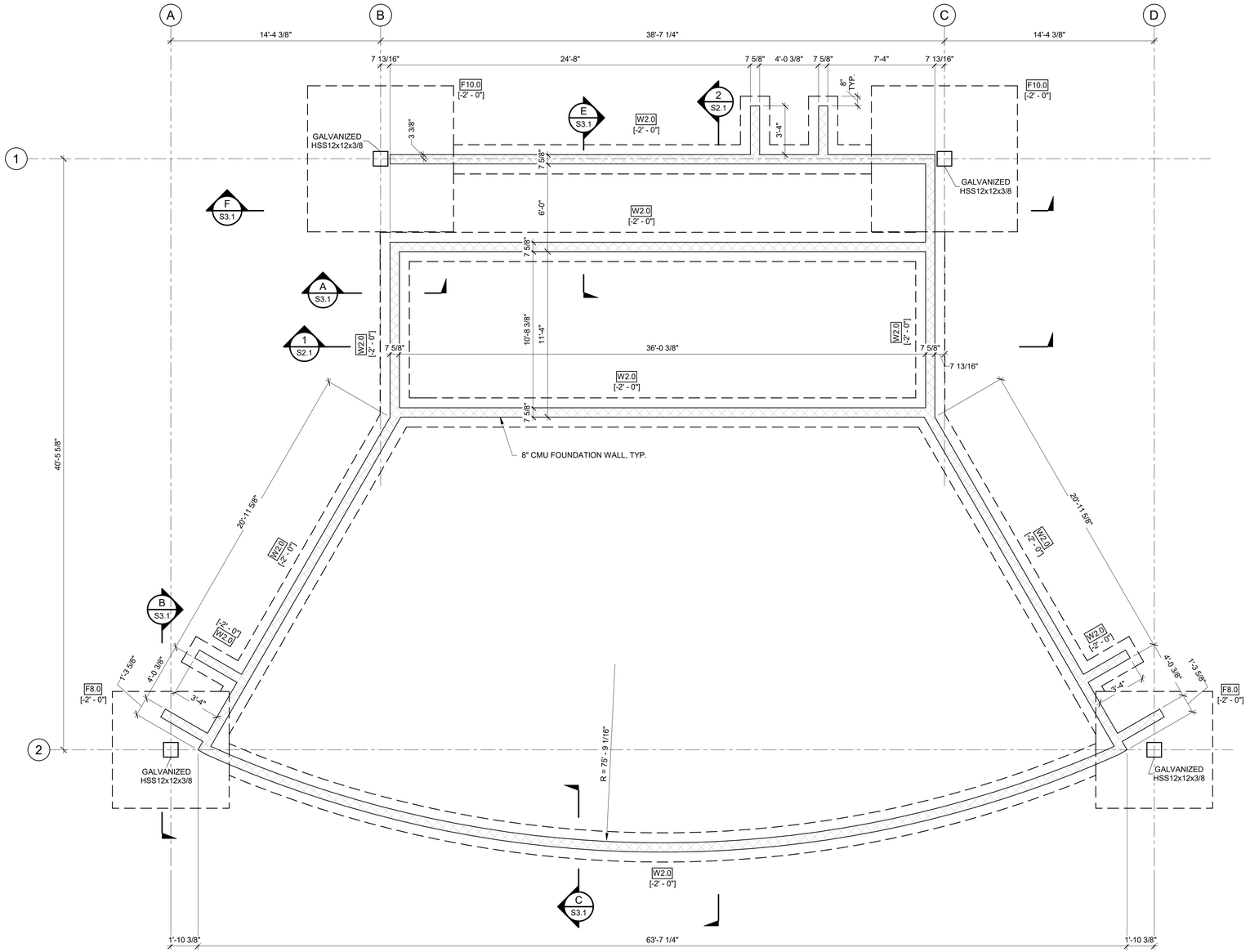
EAST SIDE PARK AMPHITHEATER

5 WEST MAIN STREET, ALAMO, GEORGIA 30411

PROJECT NO.	24.076
DATE	02/13/25
DRAWN BY	MPD
CHECKED BY	BKS

STRUCTURAL NOTES

S0.2



FOUNDATION PLAN NOTES:

1. T/SLAB ELEV. (2'-0") IS FOR REFERENCE ONLY. COORDINATE ELEVATION DATUM WITH CIVIL AND ARCHITECTURAL DRAWINGS.
2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS AND ELEVATIONS, DOOR AND WINDOW OPENING LOCATIONS NOT SHOWN.

WALL FOUNDATION SCHEDULE		
TAG	FOOTING SIZE	REINFORCEMENT
W2.0	2'-0" x CONT. x 12" THICK.	(3) #5 CONT. & #5 @ 12" O.C., BOTTOM

COLUMN FOUNDATION SCHEDULE		
TAG	FOOTING SIZE	REINFORCEMENT
F8.0	8'-0" x 8'-0" x 24" THICK.	(8) #6 EW T&B
F10.0	10'-0" x 10'-0" x 24" THICK.	(10) #6 EW T&B

PLAN LEGEND

— — — —	FOUNDATION EXTENTS
F#.# [±FT-IN]	INDICATES SHALLOW COLUMN FOUNDATION - SEE FOUNDATION SCHEDULE
W#.# [±FT-IN]	INDICATES SHALLOW WALL STRIP FOUNDATION - SEE FOUNDATION SCHEDULE
±FT-IN TARGET	ELEVATION INDICATOR RELATIVE TO REFERENCE ELEV.
CMU SIZE	CMU WALL - CONTINUES HIGHER THICKNESS AS MARKED

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REVISIONS

#	DATE	DESCRIPTION

M.E. SACK ENGINEERING

EAST SIDE PARK AMPHITHEATER

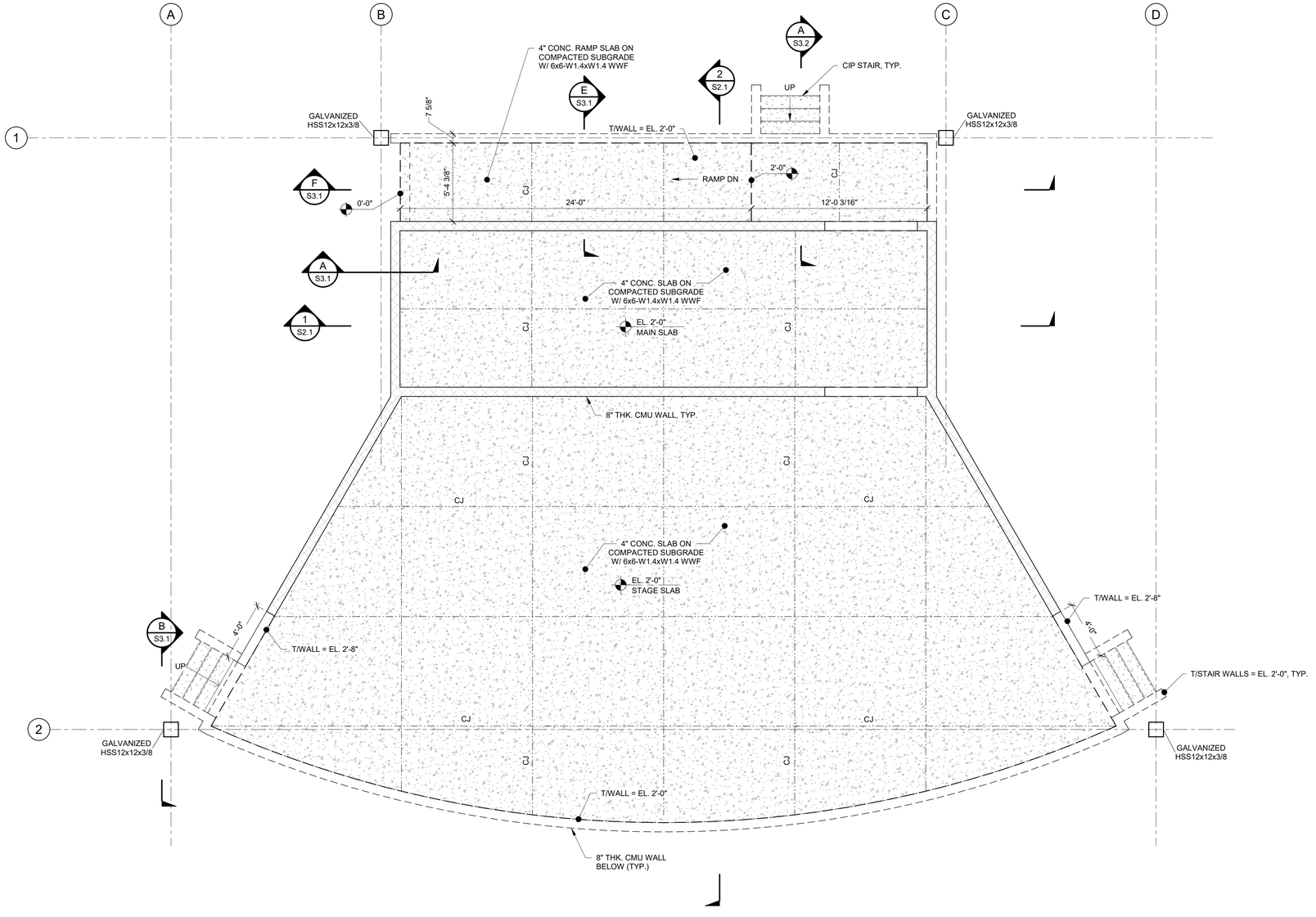
5 WEST MAIN STREET,
ALAMO, GEORGIA 30411

PROJECT NO.	24.076
DATE	02/13/25
DRAWN BY	RP
CHECKED BY	BKS

FOUNDATION PLAN

S1.1

1 FOUNDATION PLAN
1/4" = 1'-0"



FIRST FLOOR SLAB PLAN NOTES:
1. T/SLAB ELEV. (2'-0") IS FOR REFERENCE ONLY. COORDINATE ELEVATION DATUM WITH CIVIL AND ARCHITECTURAL DRAWINGS.
2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS AND ELEVATIONS, DOOR AND WINDOW OPENING LOCATIONS NOT SHOWN.

PLAN LEGEND

CMU WALL BELOW	CMU WALL BELOW - SIZE AS INDICATED
±FT-IN TARGET	ELEVATION INDICATOR RELATIVE TO REFERENCE ELEV.
CJ	SLAB CONTROL JOINT
CMU SIZE	CMU WALL CONTINUES HIGHER - THICKNESS AS MARKED
	HSS COLUMN - SIZE AS MARKED ON OTHERS PLANS

1 FIRST FLOOR SLAB PLAN
1/4" = 1'-0"

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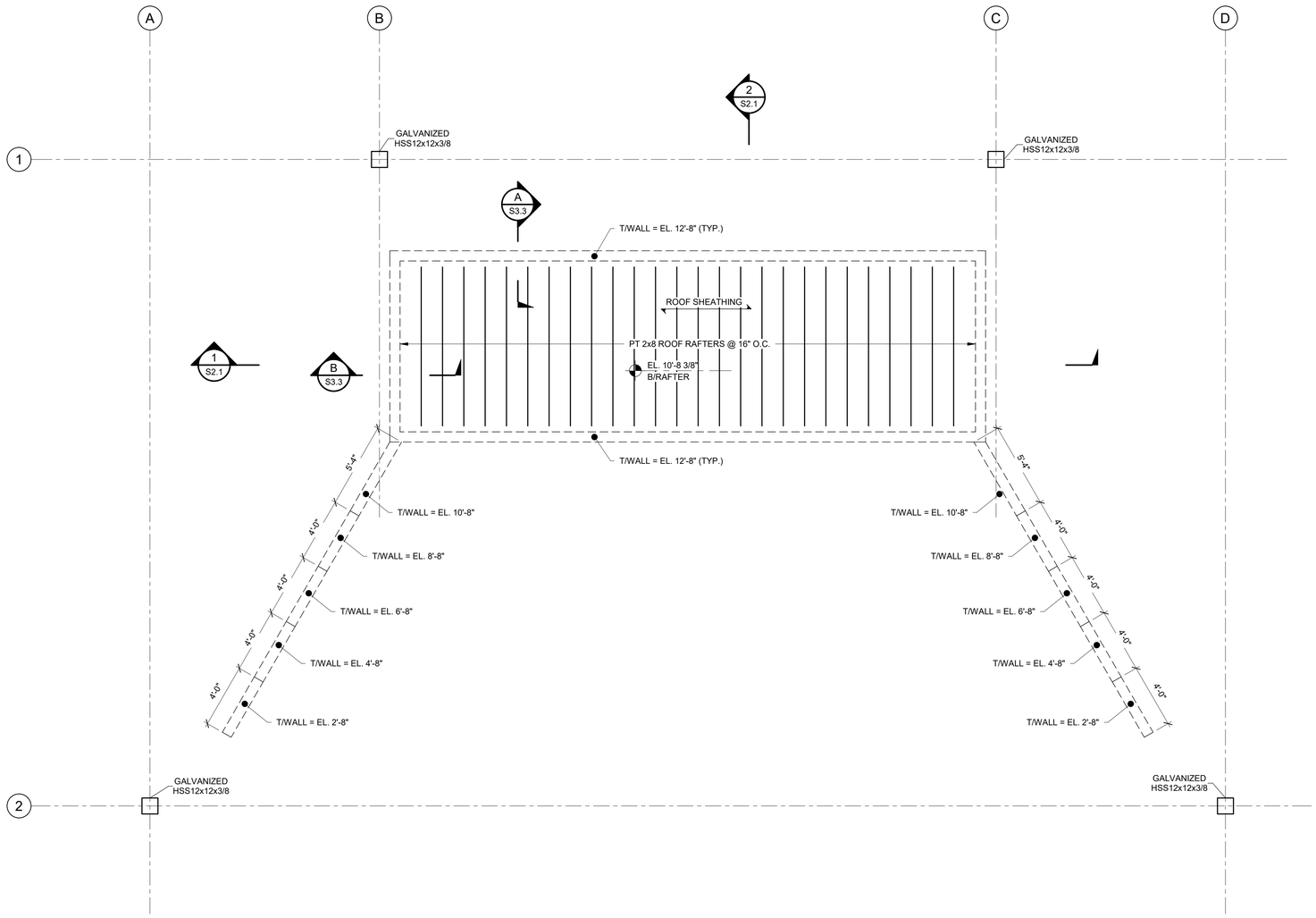
FIRST FLOOR SLAB PLAN

S1.2



SECOND FLOOR FRAMING PLAN NOTES:

1. T/SLAB ELEV. (2'-0") IS FOR REFERENCE ONLY. COORDINATE ELEVATION DATUM WITH CIVIL AND ARCHITECTURAL DRAWINGS.
2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS AND ELEVATIONS, DOOR AND WINDOW OPENING LOCATIONS NOT SHOWN.

PLAN LEGEND

	CMU WALL BELOW - SIZE AS INDICATED
	ROOF RAFTER - SIZE AS NOTED
	BEAM/GIRDER - SIZE AS MARKED
	HSS COLUMN - SIZE AS MARKED ON OTHERS PLANS
	SHEATHING SPAN INDICATOR

1 STORAGE ROOF FRAMING PLAN
1/4" = 1'-0"

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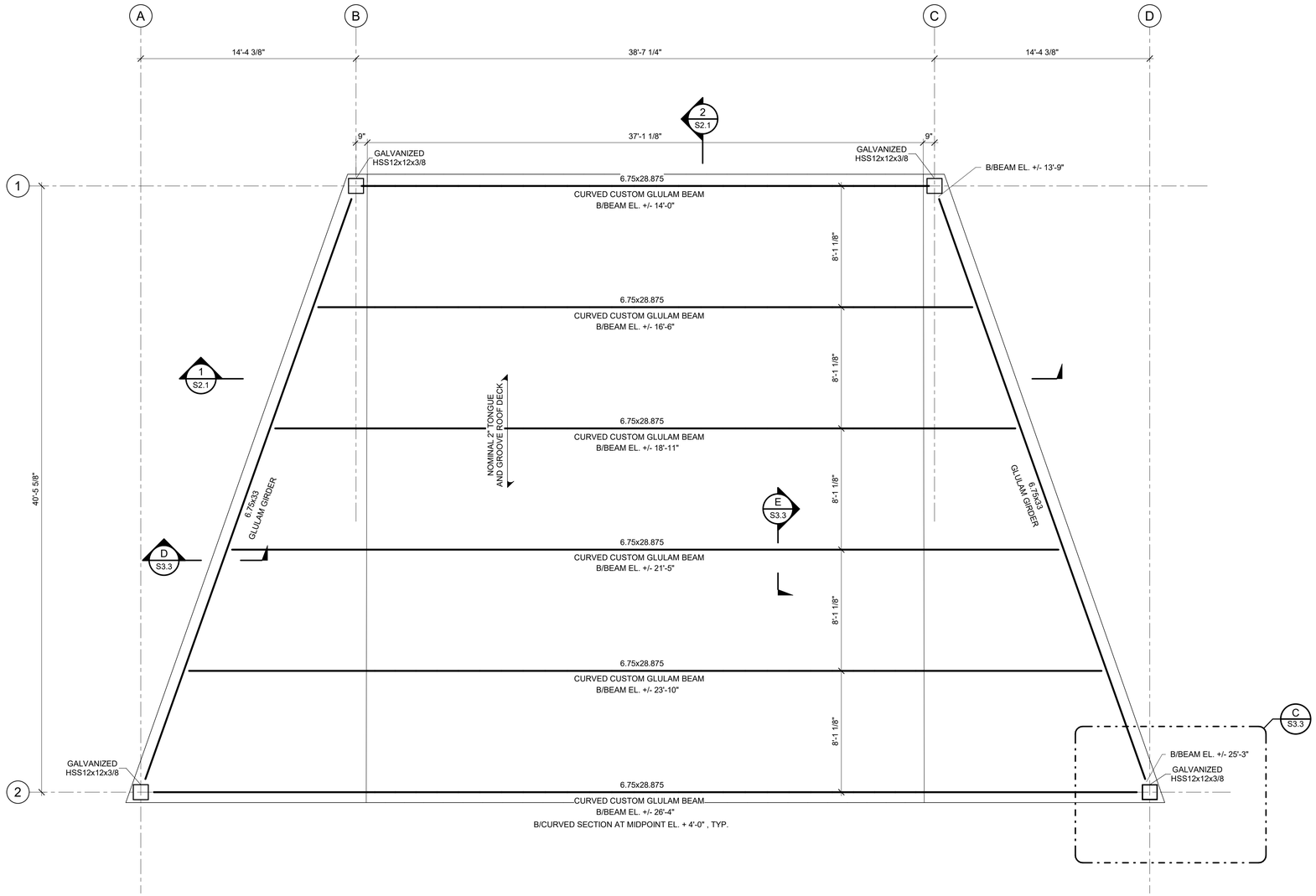
STORAGE ROOF FRAMING PLAN

S1.3



ROOF FRAMING PLAN NOTES:

1. T/SLAB ELEV. (2'-0") IS FOR REFERENCE ONLY. COORDINATE ELEVATION DATUM WITH CIVIL AND ARCHITECTURAL DRAWINGS.
2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS AND ELEVATIONS. DOOR AND WINDOW OPENING LOCATIONS NOT SHOWN.
3. VERIFY ALL CURVED SECTION DIMENSIONS WITH ARCHITECT.

PLAN LEGEND

- BEAM/GIRDER - SIZE AS MARKED
- HSS COLUMN - SIZE AS MARKED ON PLANS
- DECK SPAN INDICATOR

1 ROOF FRAMING PLAN
1/4" = 1'-0"

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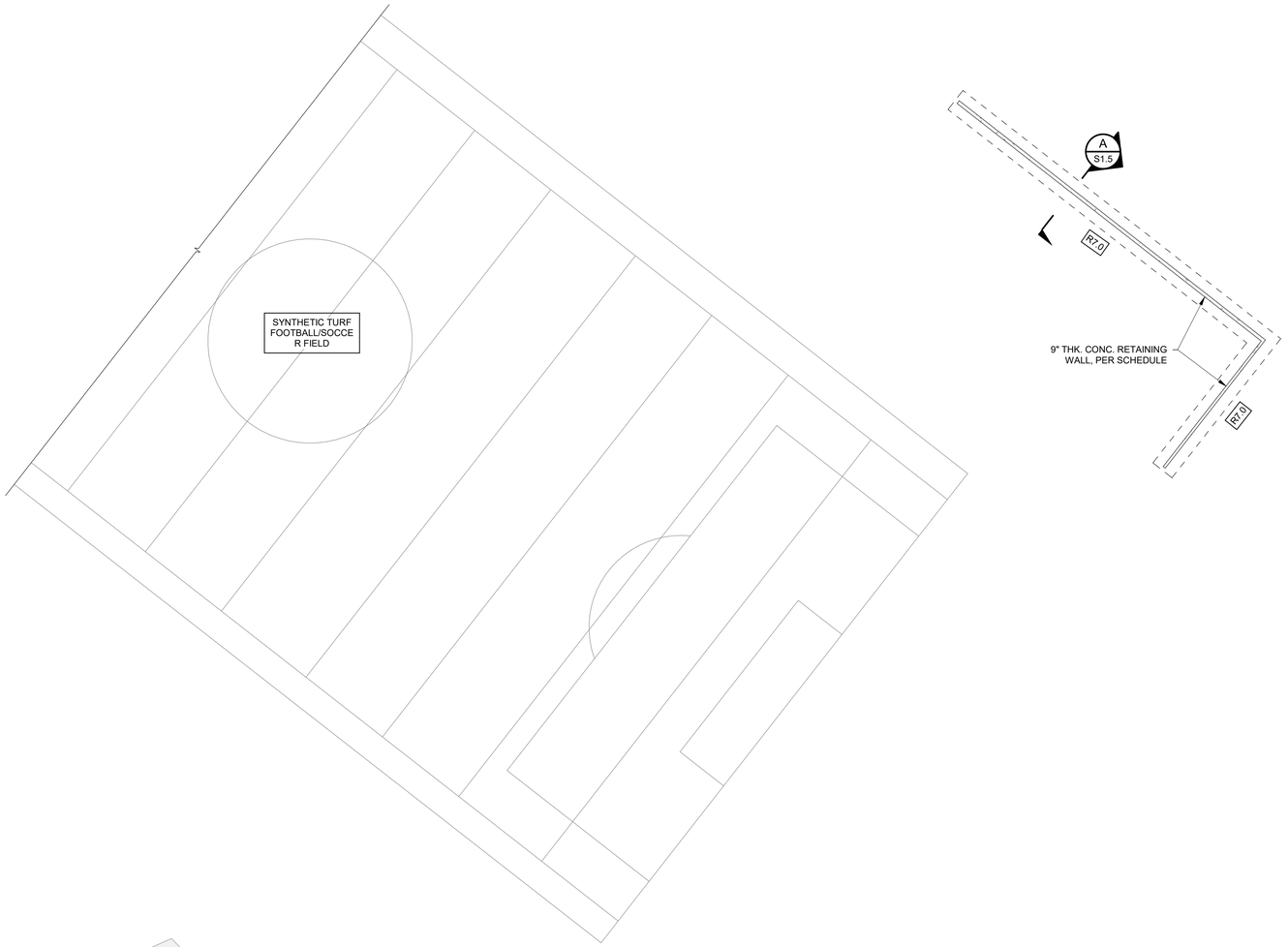
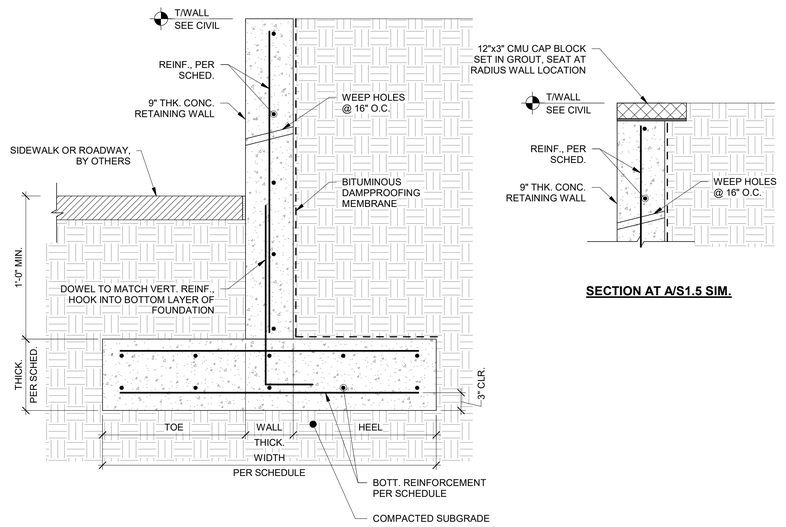
ROOF FRAMING PLAN

S1.4



1 RETAINING WALL PLAN
1" = 20'-0"

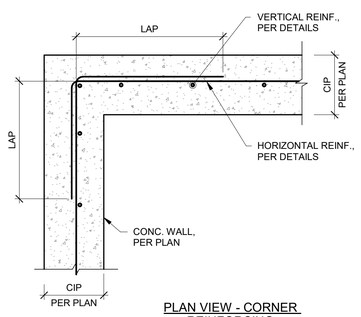


SECTION AT A/S1.5 SIM.

RETAINING WALL SCHEDULE

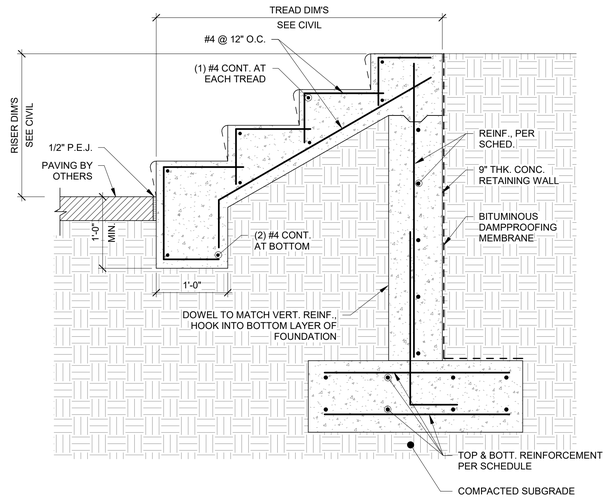
MAXIMUM RETAINED HEIGHT	FOOTING MARK	DIMENSIONS				REINFORCEMENT				
						FOOTING			STEM WALL	
		TOE	WALL THICK.	HEEL	FTG. THICK.	BASE TOP FACE	BASE BOTT. FACE	BASE TRANSV.	HORIZ.	REAR FACE VERT.
3'-0"	R3.0	1'-1 1/2"	9"	1'-1 1/2"	1'-0"	#6 @ 12" O.C.	#6 @ 12" O.C.	#6 @ 12" O.C.	#6 @ 12" O.C. CENTER	#6 @ 12" O.C. CENTER
8'-0"	R7.0	3'-0"	9"	3'-3"	1'-3"	#6 @ 12" O.C.	#6 @ 12" O.C.	#6 @ 12" O.C.	#6 @ 12" O.C. EA. FACE	#6 @ 12" O.C. EA. FACE

A SITE RETAINING WALL DETAIL
1" = 1'-0"



PLAN VIEW - CORNER REINFORCING

C REINFORCEMENT AT CONC. WALL CORNERS
1" = 1'-0"



B STAIR SECTION AT RETAINING WALL
1" = 1'-0"

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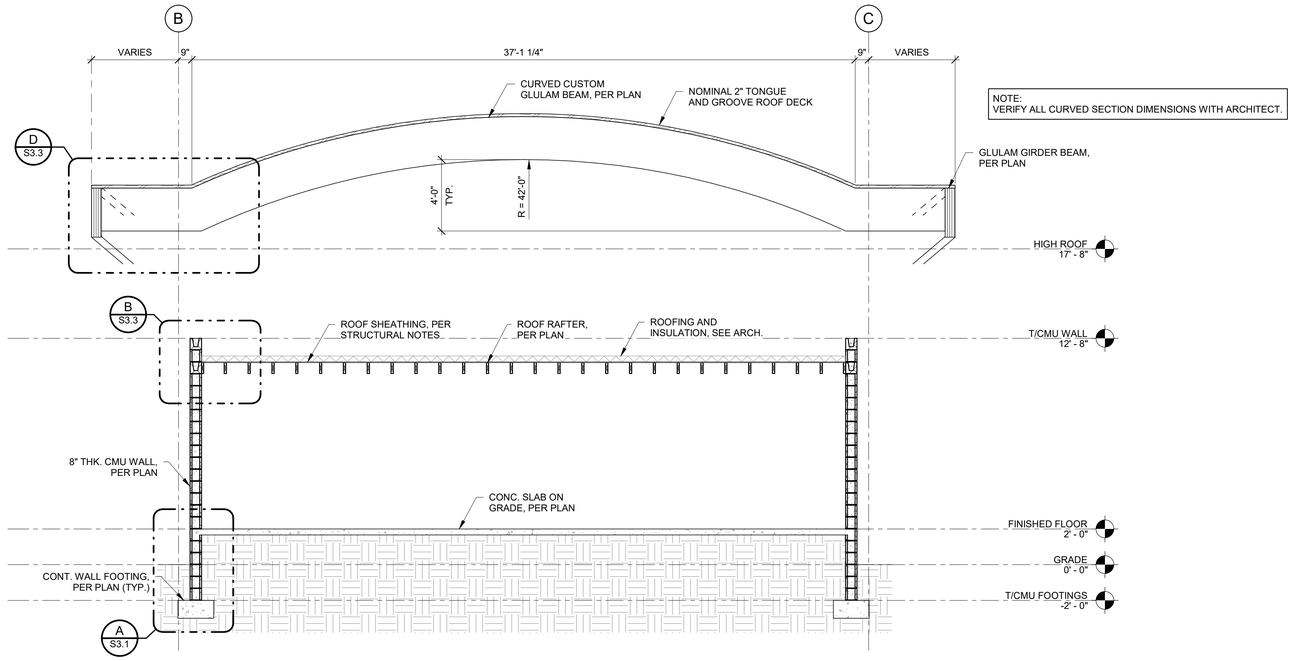
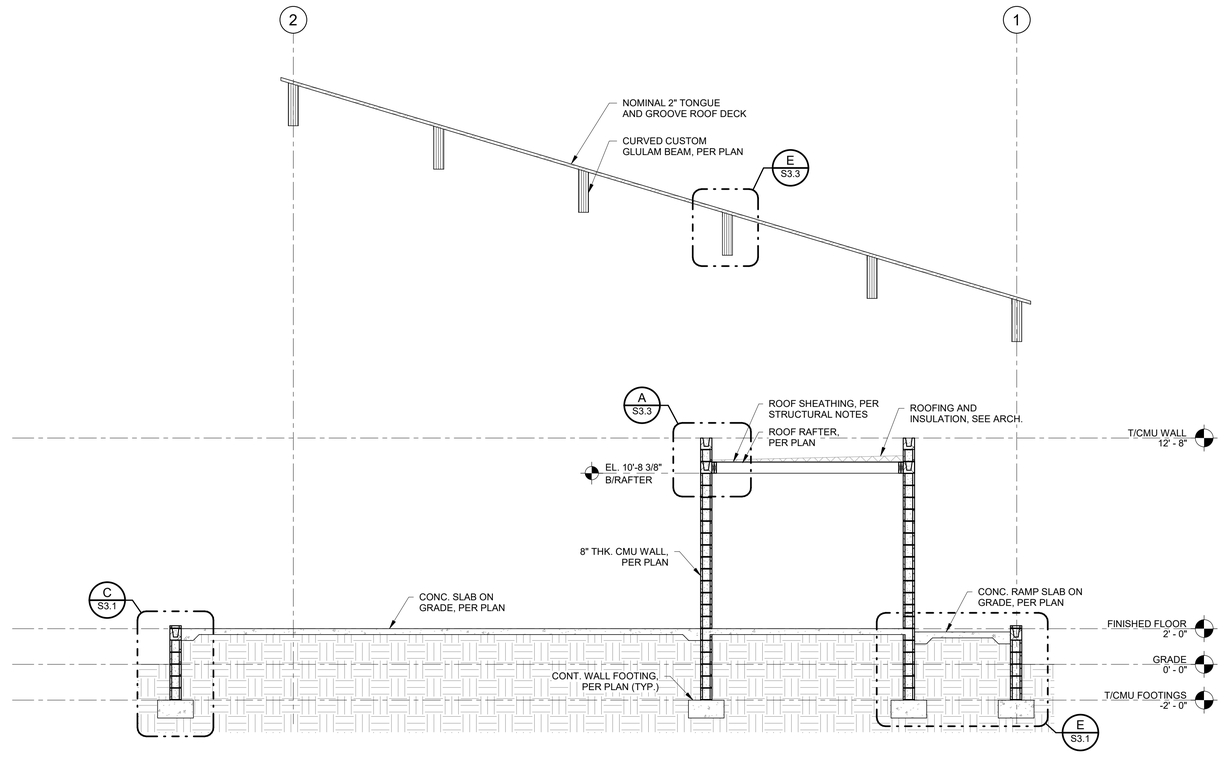
RETAINING WALL PLAN AND DETAILS

S1.5



1 SECTION 1
1/4" = 1'-0"



2 SECTION 2
1/4" = 1'-0"

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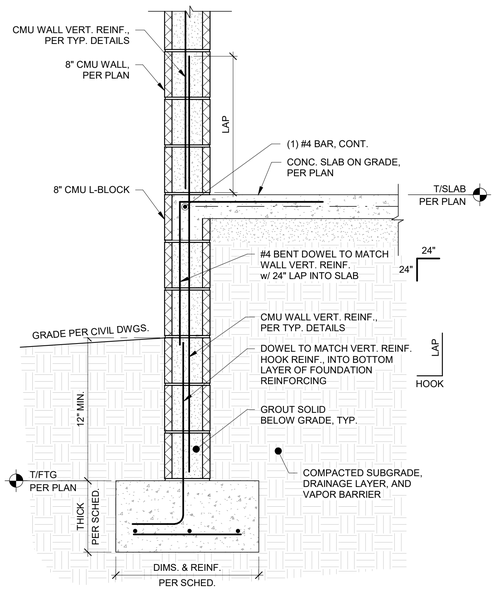
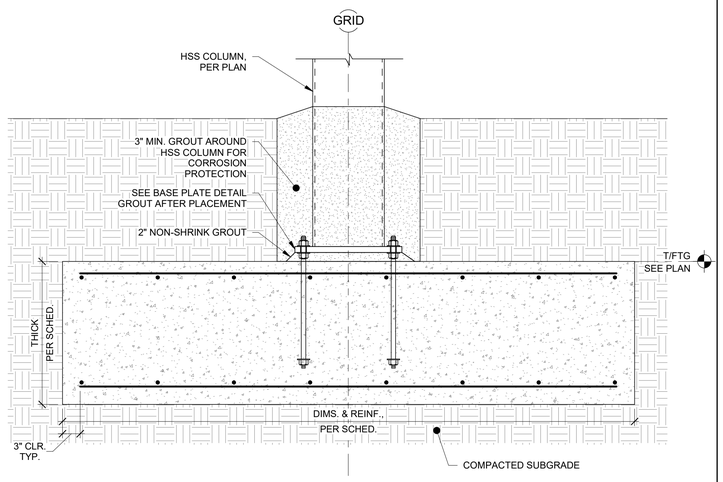
STRUCTURAL SECTIONS

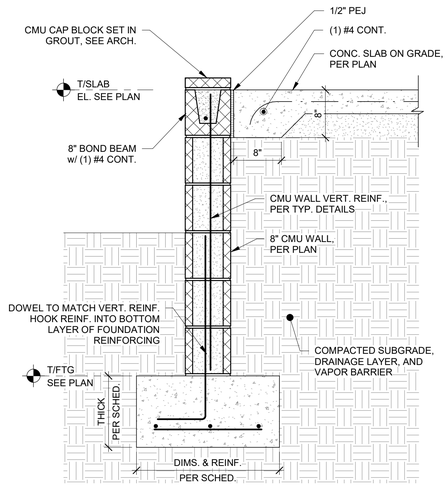
S2.1



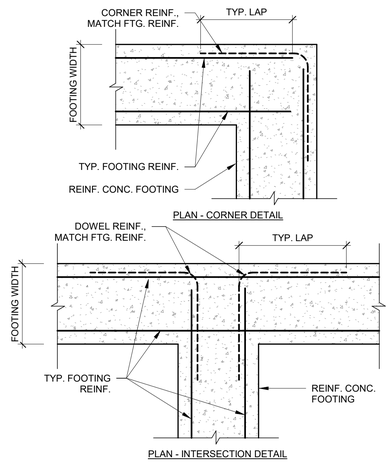
A
1" = 1'-0"
EXTERIOR CMU WALL FOUNDATION DETAIL



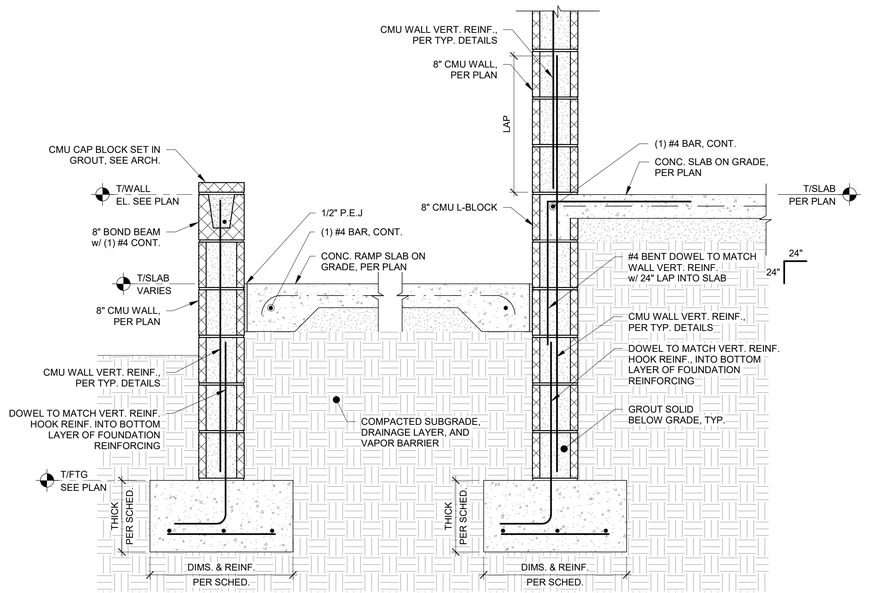
B
1" = 1'-0"
EXTERIOR COLUMN FOUNDATION WITH PIER



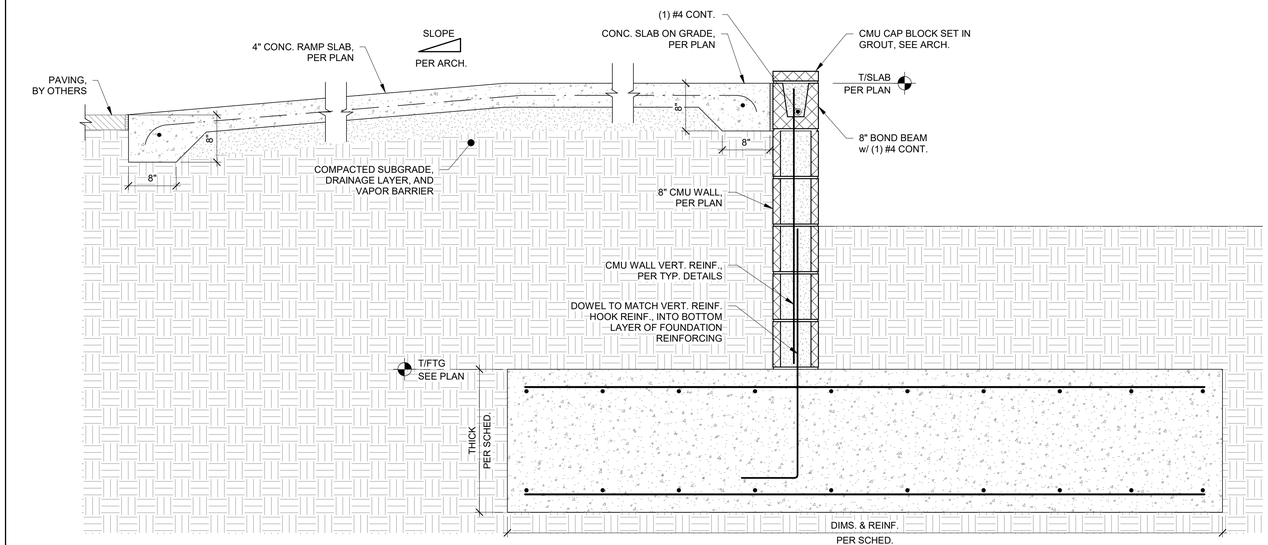
C
1" = 1'-0"
EXTERIOR CMU WALL FOUNDATION DETAIL



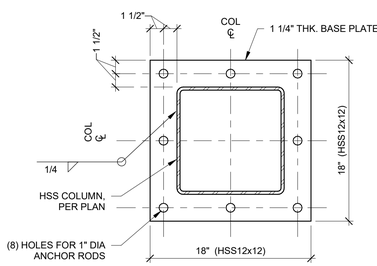
D
3/4" = 1'-0"
REINFORCEMENT AT INTERSECTIONS



E
1" = 1'-0"
RAMP SLAB SECTION

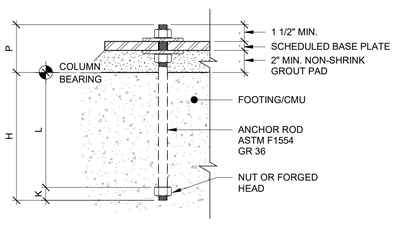


F
1" = 1'-0"
RAMP SLAB SECTION



HSS SQUARE BASEPLATES

G
1 1/2" = 1'-0"
BASEPLATE DETAILS



TYPICAL ANCHOR ROD SCHEDULE	
DIMENSIONS	TYP.
BASEPLATE THICKNESS	1 1/4"
BOLT Ø, d	1"
L	16"
H	18"
K	2"
P (MIN)	4 1/4"

H
3" = 1'-0"
TYPICAL ANCHOR ROD DETAIL

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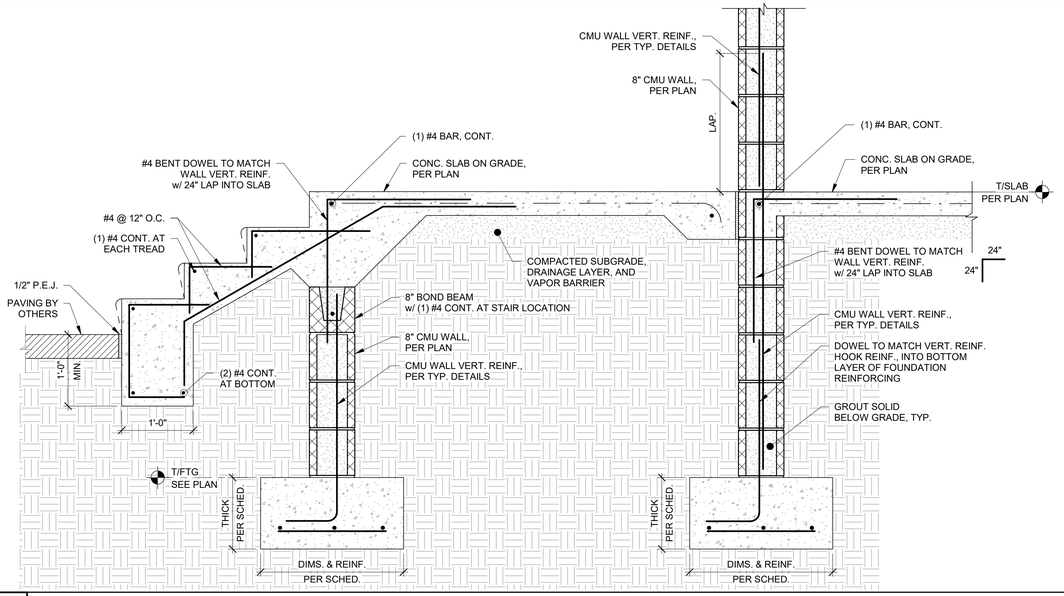
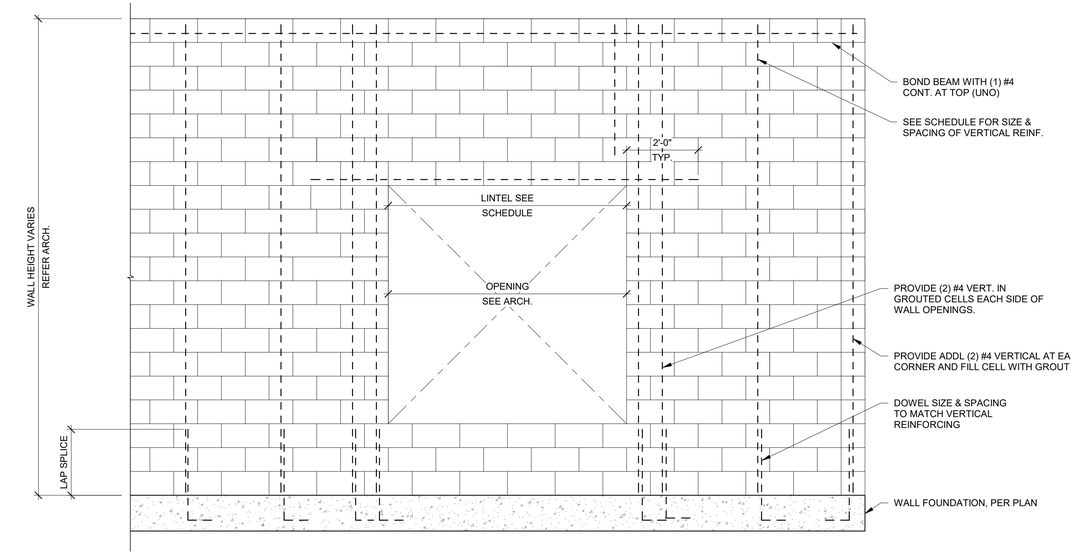
STRUCTURAL DETAILS

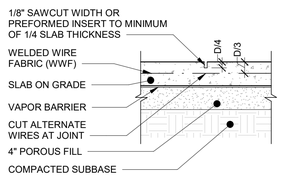
S3.1



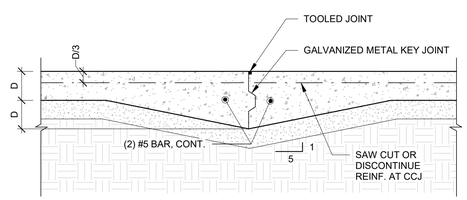
A
1" = 1'-0"
CONCRETE STAIR REINFORCEMENT



B
1/2" = 1'-0"
TYPICAL CMU WALL OPENING ELEVATION2

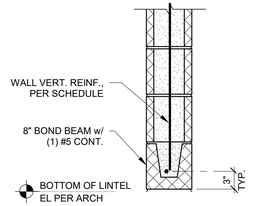


CONTROL JOINT (CJ)



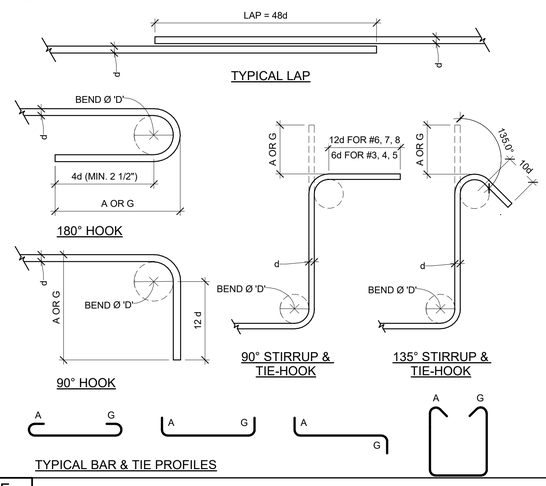
CONSTRUCTION JOINT (CCJ)

C
1" = 1'-0"
TYPICAL SLAB ON GRADE DETAILS



UP TO 6'-4"

D
1" = 1'-0"
MASONRY LINTEL DETAIL

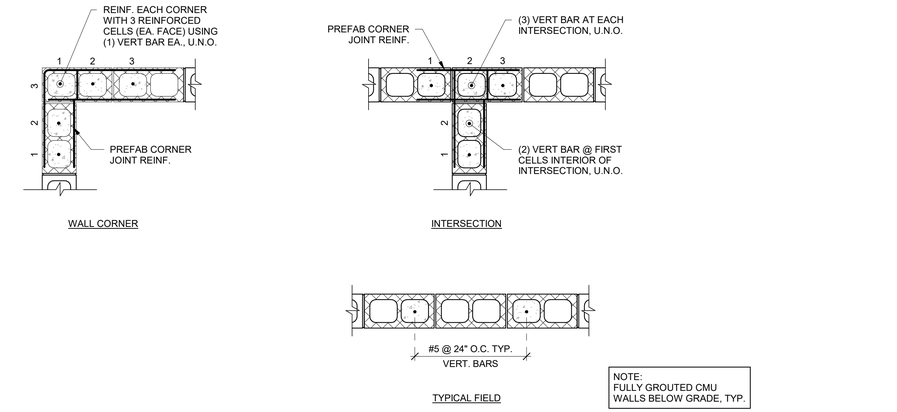


TYPICAL LAP & HOOK SCHEDULE							
BAR SIZE	TYP. LAP (IN.)	STANDARD HOOK			STIRRUP & TIE HOOK		
		D (IN.)	180° HOOK A OR G (IN.)	90° HOOK A OR G (IN.)	D (IN.)	90° HOOK A OR G (IN.)	135° HOOK A OR G (IN.)
#3	18	2 1/4	5	6	1 1/2	4	4
#4	24	3	6	8	2	4 1/2	4 1/2
#5	30	3 3/4	7	10	2 1/2	6	5 1/2
#6	36	4 1/2	8	1-0	4 1/2	1-0	8
#7	42	5 1/4[a]	10	1-2	5 1/4[a]	1-2	9
#8	48	6[a]	11	1-4	6[a]	1-4	10 1/2
#9	54	9 1/2	1-3	1-7			
#10	62	10 3/4	1-5	1-10			
#11	68	12	1-7	2-0			
#14	82	18 1/4	2-3	2-7			
#18	108	24	3-0	3-5			

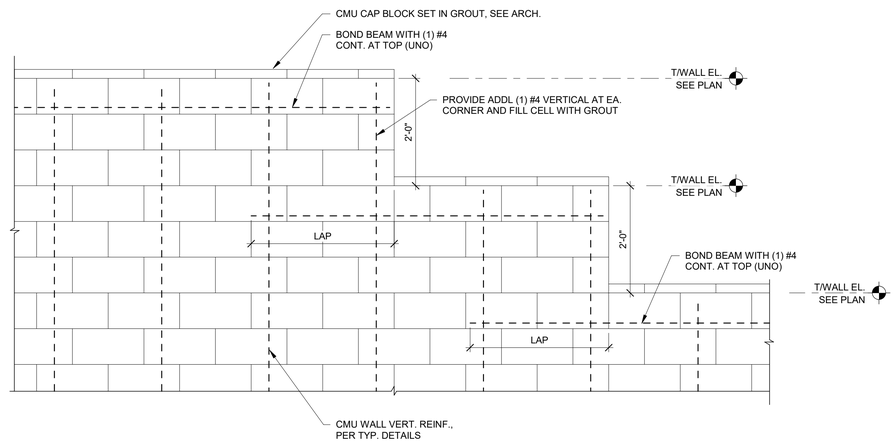
NOTE:
UNITS = IN. *OR* FT.-IN.

NOTE a:
ASTM A767 REQUIRES THAT BARS BENT COLD PRIOR TO HOT DIP GALVANIZING MUST BE FABRICATED TO A MINIMUM BEND DIAMETER EQUAL TO 7 IN. FOR #7 BAR AND 8 IN. FOR #8 BAR.

F
1" = 1'-0"
REINFORCING LAP AND HOOK LENGTHS



E
3/4" = 1'-0"
TYPICAL CMU WALL REINFORCEMENT DETAILS



G
3/4" = 1'-0"
MASORY WALL STEP DETAIL

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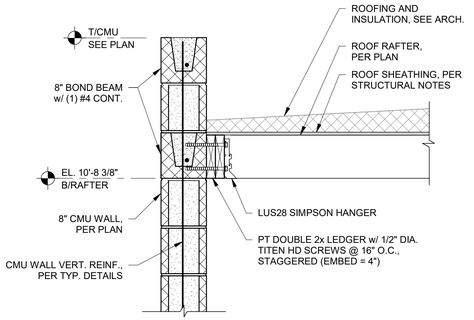
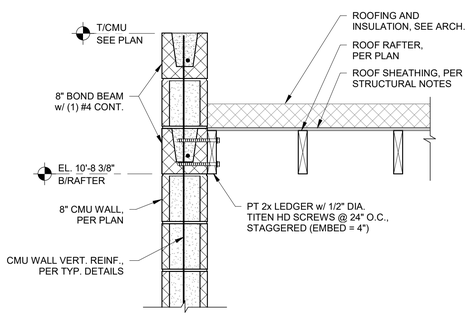
S3.2



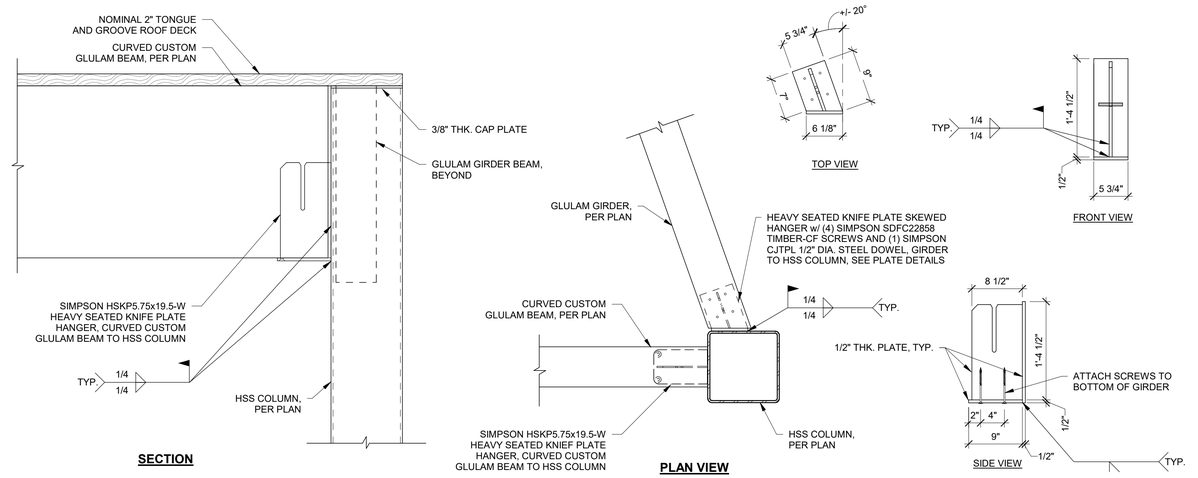
A
1" = 1'-0"

ROOF RAFTER CONNECTION WITH CMU WALL



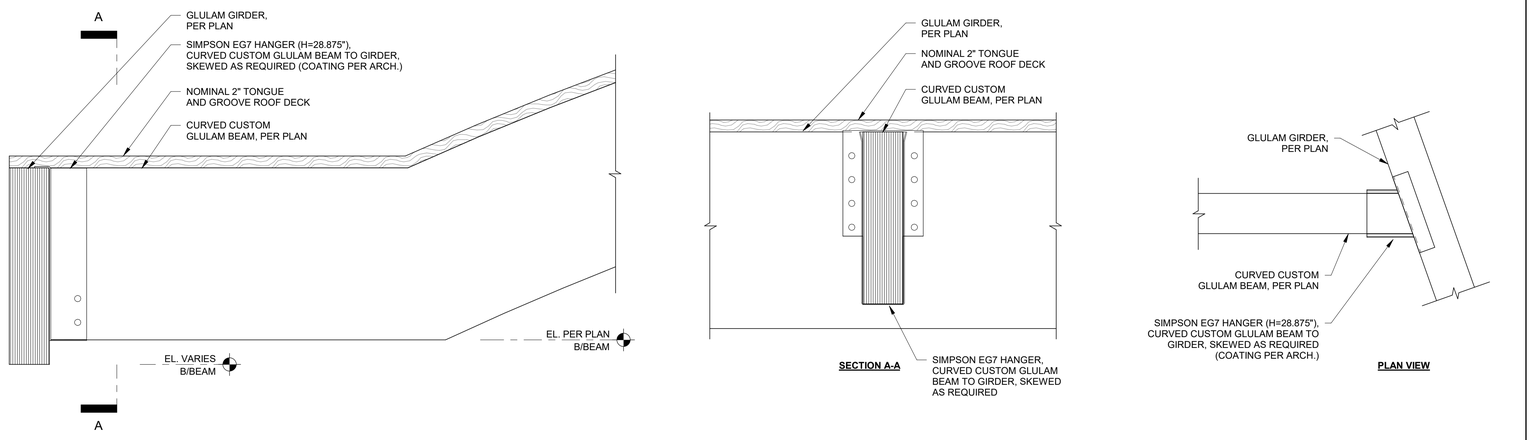
B
1" = 1'-0"

ROOF RAFTER PARALLEL TO CMU WALL



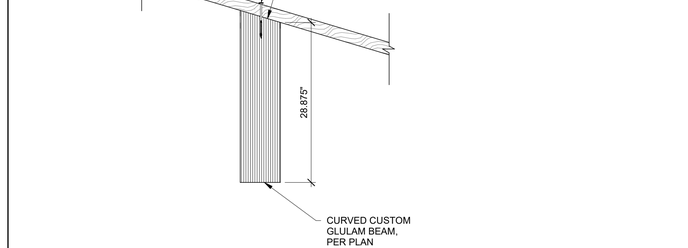
C
1" = 1'-0"

GLULAM BEAM TO STEEL COLUMN CONNECTION DETAIL



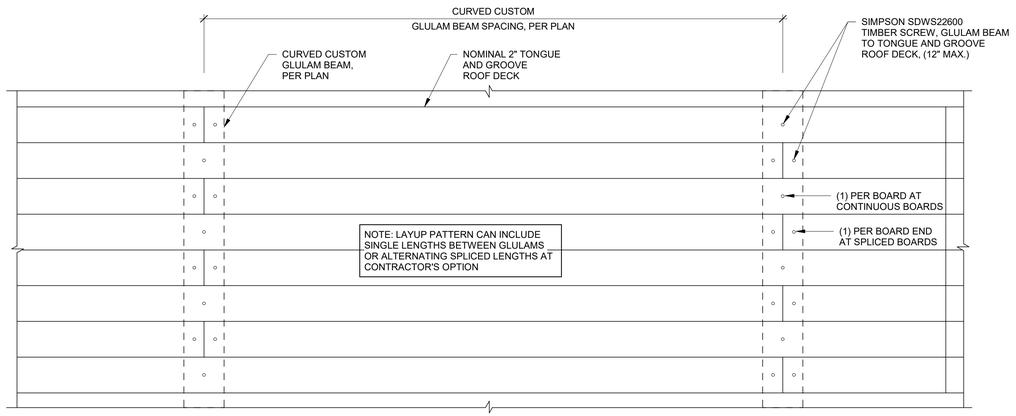
D
1" = 1'-0"

GLULAM GIRDER AND BEAM CONNECTION



E
1" = 1'-0"

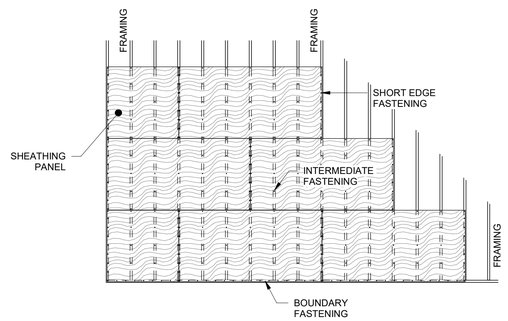
DECK TO GLULAM BEAM CONNECTION DETAIL



F
1" = 1'-0"

TONGUE & GROOVE ROOF DECK SIMPLE SPAN LAYUP

ROOF SHEATHING SCHEDULE	
USAGE:	STORAGE
THICKNESS:	1/2 IN.
TYPE:	OSB OR PLYWOOD
BOND CLASSIFICATION:	EXP. 1
PANEL GRADE:	SHEATHING
SPAN RATING:	32/16
BLOCKED/UNBLOCKED:	UNBLOCKED
FASTENER TYPE:	8d COMMON (2-1/2"x0.131")
BOUNDARY FASTENER SPACING:	6 IN.
SHORT EDGE FASTENER SPACING:	6 IN.
LONG EDGE FASTENER SPACING:	N/A
INTERMEDIATE FASTENER SPACING:	12 IN.



UNBLOCKED

G
1/4" = 1'-0"

ROOF SHEATHING

SAPP STRUCTURAL
ENGINEERING | INSPECTIONS
601 EAST 69th STREET
SAVANNAH, GA 31405
(912) 590-0542
SAPPSTRUCTURAL.COM

GEORGIA REGISTERED 2/13/25 No. SE0008802 STRUCTURAL ENGINEER BRIAN K. SAPP

REVISIONS

#	DATE	DESCRIPTION

M.E. SACK ENGINEERING

EAST SIDE PARK AMPHITHEATER

5 WEST MAIN STREET,
ALAMO, GEORGIA 30411

PROJECT NO.	24.076
DATE	02/13/25
DRAWN BY	SRS
CHECKED BY	BKS

STRUCTURAL DETAILS

S3.3

STRUCTURAL DESIGN CRITERIA

BUILDING CODE

2018 INTERNATIONAL BUILDING CODE (IBC)

DEAD LOAD

DESIGN DEAD LOAD TABLE	
CONSTRUCTION	DEAD LOAD
ROOF	5 PSF
COLLATERAL	5 PSF

FLOOR LIVE LOAD

FLOOR LIVE LOAD TABLE		
FLOOR USE	UNIFORM LIVE LOADING	CONCENTRATED LIVE LOADING
BASKET BALL COURT	200 PSF	--

ROOF LIVE LOAD

ROOF LIVE LOAD TABLE		
ROOF TYPE	UNIFORM LIVE LOADING	CONCENTRATED LIVE LOADING
ORDINARY FLAT AND PITCHED ROOF	20 PSF	300 LBS

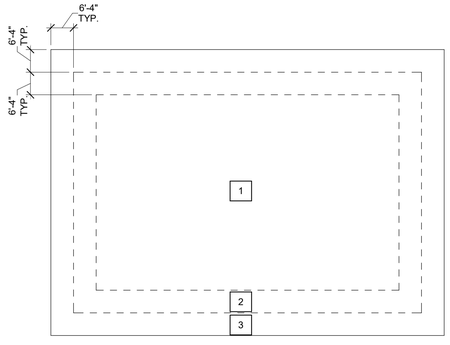
ROOF SNOW LOAD DATA

GROUND SNOW LOAD, p_g = 0 PSF

WIND DESIGN DATA

ULTIMATE DESIGN WIND SPEED, V_{ult} = 111 MPH
NOMINAL DESIGN WIND SPEED, V_{asd} = 86 MPH
RISK CATEGORY = II
WIND EXPOSURE = B
INTERNAL PRESSURE COEFFICIENT, (GCpi) = 0.0 (OPEN)
COMPONENTS AND CLADDING ULTIMATE WIND PRESSURES = (SEE TABLE BELOW)

COMPONENTS & CLADDING ULTIMATE WIND PRESSURES				
ELEMENT	ZONE	AREA (SQ. FT.)	p_{net} (PSF)	
			POSITIVE	NEGATIVE
ROOF	1	64	16.0	-16.7
		128	16.0	-16.7
		256	16.0	-16.7
	2	64	24.3	-25.2
		128	24.3	-25.2
		256	15.9	-16.7
	3	64	31.7	-50.0
		128	24.3	-25.2
		256	16.0	-16.7



TYPICAL ROOF PLAN

EARTHQUAKE DESIGN DATA

RISK CATEGORY = II
SEISMIC IMPORTANCE FACTOR, I_e = 1.00
SITE CLASS = D
S_S = 0.164g
S_1 = 0.075g
S_{DS} = 0.175g
S_{D1} = 0.119g
T_L = 8 sec
SEISMIC DESIGN CATEGORY = B

SEISMIC FORCE RESISTING SYSTEM					
SEISMIC FORCE RESISTING SYSTEM	DETAILING SECTION	R	Ω_0	C_d	h_n LIMIT
H. STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE	14.1	3	3	3	SDC B = NL

RESPONSE MODIFICATION COEFFICIENT, R = 3
ANALYSIS PROCEDURE UTILIZED = EQUIVALENT LATERAL FORCE PROCEDURE (ASCE 7-16 12.8)
SEISMIC RESPONSE COEFFICIENT, C_s = 0.06
SEISMIC BASE SHEAR, V = 5.3 KIPS

GEOTECHNICAL INFORMATION

PROJECT GEOTECHNICAL REPORT = TERRACON, PROJECT NO. ES245082, DATED MAY 1, 2024
ALLOWABLE VERTICAL BEARING PRESSURE = 2,500 PSF (ISOLATED); 2,500 PSF (CONTINUOUS)
MODULUS OF SUBGRADE REACTION = 120 PSI/IN BELOW NATURAL GRADE

FLOOD DESIGN DATA

FLOOD ZONE = X

GENERAL REQUIREMENTS

1. THE INTENT OF THESE DRAWINGS IS TO SHOW ALL ITEMS NECESSARY TO COMPLETE THE FOUNDATION FOR THE PRE-ENGINEERED METAL BUILDING. FOR ITEMS, METHODS AND/OR MATERIALS NOT SHOWN, THE MINIMUM REQUIREMENTS OF THE 2018 INTERNATIONAL BUILDING CODE SHALL GOVERN, AS AMENDED BY THE STATE AND LOCAL GOVERNING AGENCIES OF THE PROJECT LOCATION.
2. ALL DRAWINGS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO START OF CONSTRUCTION SO THAT A CLARIFICATION CAN BE PROVIDED.
3. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT THEIR OWN EXPENSE AND AT NO EXPENSE TO THE OWNER, ARCHITECT OR ENGINEER.
4. DRAWINGS ARE NOT TO BE SCALED. WRITTEN DIMENSIONS SHALL GOVERN CONSTRUCTION. THE CONTRACTOR SHALL VERIFY DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS AND THE SITE CONDITIONS PRIOR TO CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND THE ENGINEER SO THAT CLARIFICATION CAN BE PROVIDED.
5. THE STRUCTURAL CONSTRUCTION DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OR SEQUENCE OF CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR AND PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES AND SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS AND THE PROGRAMS INCIDENT THERETO NOR SHALL OBSERVATION VISITS TO THE SITE INCLUDE INSPECTION OF THESE ITEMS). THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND IMPLEMENTATION OF ALL SCAFFOLDING, BRACING AND SHORING.
6. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT.
7. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.
8. ANY DELEGATED ENGINEERING DESIGN TO BE PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL MEET THE CRITERIA HEREIN, AND SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT.
9. ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW.
10. NO STRUCTURAL MEMBER SHALL BE CUT OR NOTCHED OR OTHERWISE REDUCED IN STRENGTH UNLESS APPROVED BY THE STRUCTURAL ENGINEER.

EXISTING CONDITIONS

1. THE CONTRACTOR SHALL FIELD VERIFY CONDITIONS THAT MAY AFFECT THE STRUCTURAL DESIGN. IF ANY DEVIATIONS ARE DISCOVERED BETWEEN ACTUAL CONDITIONS AND THE CONDITIONS SHOWN ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND ENGINEER IMMEDIATELY, AS MODIFICATIONS MAY BE REQUIRED.
2. ITEMS REQUIRING FIELD VERIFICATION INCLUDE:
   a. PLAN DIMENSIONS
3. CONTRACTOR SHALL CONTACT ARCHITECT AND ENGINEER OF SIGNIFICANT DECAY, SPALLS, CORROSION, OR ANY DEFECT OF EXISTING STRUCTURAL MEMBERS AND CONNECTIONS.

STRUCTURAL SPECIAL INSPECTIONS

1. SPECIAL STRUCTURAL TESTS AND INSPECTIONS SHALL BE PERFORMED ON THIS PROJECT IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE (IBC). THE FOLLOWING DOCUMENTS HAVE BEEN PREPARED FOR THIS PROJECT AS A PART OF THESE CONSTRUCTION DOCUMENTS:
   a. STATEMENT OF SPECIAL INSPECTIONS
   b. SCHEDULE OF SPECIAL INSPECTIONS
   c. STATEMENT OF SPECIAL INSPECTIONS REQUIREMENTS FOR WIND RESISTANCE
   d. STATEMENT OF SPECIAL INSPECTIONS REQUIREMENTS FOR SEISMIC RESISTANCE
2. SPECIAL STRUCTURAL TESTS AND INSPECTIONS SHALL BE PERFORMED BY AN AGENCY SELECTED BY THE CONTRACTOR AND APPROVED BY THE ENGINEER OF RECORD. THE AGENCY SHALL MEET ALL OF THE REQUIREMENTS FOR APPROVAL INDICATED IN IBC SECTION 1703.1. SPECIAL INSPECTORS SHALL BE QUALIFIED PERSONS WHO SHALL DEMONSTRATE COMPETENCE TO THE SATISFACTION OF THE BUILDING OFFICIAL FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
3. THE CONTRACTOR SHALL COORDINATE THE INSPECTION SERVICES IN ACCORDANCE WITH THE PROGRESS OF THE WORK. THE CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE TO THE INSPECTOR TO ALLOW PROPER SCHEDULING OF PERSONNEL.
4. ALL REPORTS AND SHOP CERTIFICATION OF SPECIAL INSPECTIONS TO BE PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP SHALL BE SUBMITTED TO THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DISTRIBUTING THESE REPORTS TO THE SPECIAL INSPECTOR, THE ARCHITECT, AND THE ENGINEER OF RECORD IN A TIMELY MANNER.
5. THE COSTS OF THE SPECIAL INSPECTOR'S SERVICES SHALL BE PAID FOR BY THE OWNER.SPECIAL INSPECTIONS REPORTS AND A FINAL REPORT IN ACCORDANCE WITH IBC SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF THE WORK IS APPROVED FOR OCCUPANCY.
6. REPORTS SHALL INDICATE THAT THE WORK WAS PERFORMED AND CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. WORK NOT IN CONFORMANCE SHALL BE IDENTIFIED IN THE REPORT AND SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR.
7. A FINAL REPORT OF INSPECTIONS DOCUMENTING REQUIRED SPECIAL INSPECTIONS, INCLUDING ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL, THE ARCHITECT, AND THE ENGINEER OF RECORD PRIOR TO COMPLETION OF THE STRUCTURAL SYSTEMS BUT AT A FREQUENCY NOT TO EXCEED 60 DAYS.

SUBGRADE PREPARATION

1. CONTRACTOR SHALL FOLLOW SITE WORK RECOMMENDATIONS LISTED IN THE PROJECT GEOTECHNICAL REPORT BY TERRACON DATED MAY 1, 2024 .

FOUNDATIONS

1. THE FOUNDATION IS DESIGNED BASED UPON THE RECOMMENDATIONS AND DESIGN PARAMETERS INCLUDED IN THE PROJECT GEOTECHNICAL REPORT PREPARED BY TERRACON DATED MAY 1, 2024
2. SOIL PRESSURES USED FOR FOUNDATION DESIGN:
   a. ALLOWABLE BEARING PRESSURE = 2,500 PSF
   b. MODULUS OF SUBGRADE REACTION = 120 PSI/IN
3. ALL FOUNDATIONS SHALL BE PLACED ON COMPACTED SUBGRADE. SEE SUBGRADE PREPARATION NOTES.
4. THE BOTTOM OF ALL EXTERIOR FOUNDATIONS SHALL BE A MINIMUM OF 18 INCHES BELOW FINISHED GRADE UNLESS NOTED OTHERWISE.
5. REMOVE ALL WATER SOFTENED SOILS FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE. FILL REMAINING VOIDS WITH ADDITIONAL CONCRETE.
6. ALL FOUNDATION REINFORCEMENT SHALL BE PROPERLY TIED IN PLACE PRIOR TO PLACEMENT OF CONCRETE.
7. WHERE FINISHED GRADES DIFFER ON OPPOSITE SIDES OF FOUNDATION WALLS, PROVIDE TEMPORARY BRACING TO PREVENT LATERAL MOVEMENT UNTIL ALL ADJACENT FILL, COMPACTION, FLOOR SLABS, AND FRAMING AT NEXT LEVEL HAS BEEN COMPLETED..

REINFORCING STEEL

1. FABRICATING, PLACING, AND SUPPORTING REINFORCEMENT SHALL COMPLY WITH CRSI'S "MANUAL OF STANDARD PRACTICE."
2. REINFORCING BARS SHALL BE ASTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE.
3. REINFORCING BARS IN WELDED CONDITIONS, WHERE PERMITTED, SHALL BE ASTM A 706, DEFORMED.
4. STEEL WELDED-WIRE REINFORCEMENTS SHALL BE ASTM A 1064 WITH 70 KSI MINIMUM YIELD STRENGTH.
5. NO REINFORCEMENT SHALL BE FLAME-CUT OR BENT IN FIELD WITHOUT GUIDANCE FROM STRUCTURAL ENGINEER.
6. REINFORCING STEEL SHALL HAVE COVER PROTECTION AS FOLLOWS:

CONCRETE COVER PROTECTION TABLE	
CONDITION	MINIMUM COVER
CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:	
WALL PANELS, SLABS, JOISTS	1 INCH
OTHER MEMBERS	1½ INCHES
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS	¾ INCHES
BEAMS, COLUMNS:	
PRIMARY REINFORCEMENT	1½ INCHES
TIES, STIRRUPS, SPIRALS	1 INCH

SLABS ON GRADE

1. ALL SLABS ON GRADE SHALL BE ON COMPACTED SUBGRADE WITH 4 INCHES MINIMUM OF POROUS FILL MATERIAL. SEE SUBGRADE PREPARATION NOTES.
   ALL SLABS ON GRADE SHOULD BE SUPPORTED ON A MINIMUM OF 4-INCHES OF GRANULAR, FREE-DRAINING POROUS FILL WITH A VAPOR BARRIER AS A CAPILLARY LAYER BETWEEN THE SLAB AND THE SUBGRADE.
2. THE CONTRACTOR SHALL COORDINATE ALL LIMITS AND DEPTHS OF DEPRESSIONS FOR FLOOR FINISHES WITH ARCHITECTURAL DRAWINGS AND SCHEDULES. LIMITS SHOWN ON STRUCTURAL DRAWINGS ARE SCHEMATIC.
3. THE USE OF POLYPROPYLENE FIBERS IN LIEU OF WELDED WIRE FABRIC IS PROHIBITED WITHOUT THE WRITTEN AUTHORIZATION OF THE ENGINEER.
4. THE FINISH TOLERANCE OF ALL SLABS SHALL BE IN ACCORDANCE WITH ACI 301, TYPE A.
5. SLABS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE FOLLOWING FLATNESS / LEVELNESS REQUIREMENTS:
   a. FF = 35
   b. FL = 25
7. FF AND FL TOLERANCES SHALL BE TESTED IN ACCORDANCE WITH ASTM E 1155. ACTUAL OVERALL F-NUMBERS SHALL BE CALCULATED USING THE INFERIOR / SUPERIOR AREA METHOD. ALL FLOOR TOLERANCE MEASUREMENTS SHALL BE MADE WITHIN 48 HOURS AFTER SLAB INSTALLATION. IN ALL CASES, TOLERANCE MEASUREMENTS SHALL PRECEDE THE REMOVAL OF SHORES AND FORMS. RESULTS OF ALL FLOOR PROFILE TESTS INCLUDING A RUNNING TABULATION OF THE OVERALL FF AND FL VALUES FOR ALL OF THE RANDOM TRAFFIC SLABS INSTALLED TO DATE SHALL BE PROVIDED TO THE CONTRACTOR WITHIN 72 HOURS AFTER EACH SLAB INSTALLATION.
8. WALKWAYS AND OTHER EXTERIOR SLABS ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS. SEE THE SITE PLAN AND ARCHITECTURAL DRAWINGS FOR LOCATIONS, DIMENSIONS, ELEVATIONS, JOINTING DETAILS AND FINISH DETAILS.
9. SAW-CUT CONTRACTION JOINTS SHALL BE CUT AS SOON AS THE CONCRETE CAN BE CUT WITHOUT RAVELING. CONVENTIONAL CAW-CUT JOINTS SHOULD BE RUN WITHIN 4-12 HOURS AFTER THE CONCRETE HAS BEEN FINISHED. JOINTS PLACED WITH AN EARLY ENTRY SAW MAY BE CUT 1-4 HOURS AFTER THE SLAB HAS BEEN FINISHED.
10. SLAB JOINTS SHALL BE FILLED WITH APPROVED MATERIAL. THIS SHOULD TAKE PLACE AS LATE AS POSSIBLE PRIOR TO FILLING, REMOVE ALL DEBRIS FROM THE SLAB JOINTS, THEN FILL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AS FOLLOWS:
    a. CONTROL JOINT SEALANT = EUCLID CHEMICAL DURAL 340 SL OR APPROVED EQUAL
    b. EXPANSION/ISOLATION JOINT SEALANT = EUCLID CHEMICAL EUCOLASTIC 1 SL OR APPROVED EQUAL
11. CONCRETE FINISH FLOORS SHALL HAVE A HARD STEEL TROWELED FINISH UNLESS INDICATED OTHERWISE ON THE DRAWINGS. PLACE, STRIKE OFF, CONSOLIDATE, LEVEL AND FLOAT TO THE PROPER ELEVATION. TROWELING SHALL BEGIN AFTER SURFACE HAS RECEIVED A FLOAT FINISH. THE SLAB DRYING MUST PROCEED NATURALLY AND MUST NOT BE HASTENED BY THE DUSTING ON OF DRY CEMENT OR SAND. LIGHTLY TOOL ALL EDGES AT CONSTRUCTION JOINTS AND EXERCISE CARE THAT SLAB EDGES ARE NOT DEPRESSED ALONG BULKHEADS DURING FINISHING OPERATIONS, PARTICULARLY HAND TROWELING. EXTERIOR SLABS, SIDEWALKS, PADS AND RAMPS SHALL HAVE A LIGHT BROOM FINISH UNLESS INDICATED OTHERWISE ON THE DRAWINGS. PROVIDE STANDARD TROWEL FINISH AT ALL SUB-SLABS.
12. PROVIDE 1/2" PREMOLDED EXPANSION JOINT (P.E.J.) FILLER AROUND PERIMETER OF SLABS WHERE THEY ABUT VERTICAL SURFACES AND AT COLUMN ISOLATION JOINTS AS DETAILED.

CAST-IN-PLACE CONCRETE

1. ALL CAST-IN-PLACE CONCRETE CONSTRUCTION SHALL COMPLY WITH THE FOLLOWING ACI PUBLICATIONS:
   a. ACI 301-10 - GENERAL CONSTRUCTION REQUIREMENTS
   b. ACI 117-10 - TOLERANCES FOR CONCRETE CONSTRUCTION
2. CONCRETE SHALL BE NORMAL-WEIGHT CONCRETE (145 PCF) WITH MIXES MEETING THE FOLLOWING CRITERIA:
   a. FOUNDATION ELEMENTS & SLAB ON GRADE
      - EXPOSURE CLASS = F0
      - MINIMUM 28-DAY COMPRESSIVE STRENGTH = 4000 PSI
      - MAXIMUM WATER-TO-CEMENTITOUS MATERIALS RATIO = 0.50
      - SLUMP LIMIT = 5 INCHES (±1 INCH)
      - NOMINAL MAXIMUM AGGREGATE SIZE = 1 INCH
      - SLUMP LIMIT = 5 INCHES (±1 INCH)
      - AIR CONTENT = 4.5% (+/-1.5%)
3. ACCEPTABLE CEMENTIOUS MATERIALS:
   a. PORTLAND CEMENT - ASTM C 150, TYPE II
   b. FLY ASH - ASTM C 618 (NOT PERMITTED FOR TILTUP WALL PANELS)
   c. SLAG CEMENT - ASTM C989
   d. BLENDED HYDRAULIC CEMENT - ASTM C 595, TYPE IS OR TYPE IP
4. ALL EXPOSED CONCRETE EDGES SHALL HAVE A 3/4-INCH CHAMFER.
5. OPENINGS, POCKETS, ETC., LARGER THAN 6" SHALL NOT BE PLACED IN CONCRETE SLABS, DECKS, WALLS, UNLESS SPECIALLY DETAILED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER WHEN DRAWINGS BY OTHERS SHOW OPENINGS, POCKETS, ETC., LARGER THAN 6" NOT SHOWN ON THE STRUCTURAL DRAWINGS, BUT WHICH ARE LOCATED IN STRUCTURAL MEMBERS. FOR ANY FURTHER RESTRICTIONS ON OPENINGS IN STRUCTURAL ELEMENTS, SEE APPLICABLE SECTIONS BELOW.
6. PIPES LARGER THAN 1 ½" DIAMETER SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE EXCEPT WHEN WHERE SPECIFICALLY APPROVED. NO CONDUITS SHALL BE PLACED IN CONCRETE FILL OVER METAL DECKING.
7. REQUIRED CAST-IN-PLACE CONCRETE SUBMITTALS TO ENGINEER:
   a. PRODUCT DATA - SUBMIT TECHNICAL PRODUCT DATA FOR ANY ADMIXTURES OR CONCRETE-RELATED CONSTRUCTION PRODUCTS.
   b. DESIGN MIXTURES - THE FOLLOWING ITEMS ARE REQUIRED:
      - MIX IDENTIFICATION BY MEANS OF CLASS OR LOCATION WHERE MIX WILL BE USED.
      - STRENGTH OF CONCRETE.
      - TARGET SLUMP, WATER-TO-CEMENT RATIO, DENSITY, AND AIR CONTENT.
      - LIST OF ALL MATERIALS, ADMIXTURES, AND ADDITIVES ALONG WITH THEIR PROPORTIONS.
      - NOMINAL MAXIMUM AGGREGATE SIZE AND COMBINED AGGREGATE GRADATION.
      - CALCULATIONS AND TEST RESULTS REQUIRED BY ACI 318-14 CHAPTER 26
      - TEST RESULTS OF TOTAL CHLORIDE CONTENT.
      - INFORMATION ON CONCRETE MATERIALS AS PER ACI 301-14 SECTION 26.4
      - TEST RESULTS PER ASTM C33, INCLUDING THE CLEANNESS VALUE, SAND EQUIVALENT, AND ALKALI-SILICA REACTIVITY (ASR) POTENTIAL AND MITIGATION, IF REQUIRED.
      - MILL CERTIFICATE FOR THE CEMENT INDICATING THE SOURCE OF THE CEMENT AND COMPLIANCE WITH THE PROJECT SPECIFICATION.
      - MILL ANALYSIS FOR SUPPLEMENTARY CEMENTITIOUS MATERIALS (INCLUDING FLY ASH AND SLAG CEMENT) AND AGGREGATES FROM THE MANUFACTURER.
      - CERTIFICATION BY THE MANUFACTURERS THAT THE ADMIXTURES CONFORM TO THE SPECIFIED STANDARDS.
      - WHETHER MIX IS APPROPRIATE FOR PUMPING.
      - THERMAL CONTROL PLAN, INCLUDING HOT WEATHER AND COLD WEATHER PLACEMENT.
   c. STEEL REINFORCEMENT SHOP DRAWINGS - PLACING DRAWINGS THAT DETAIL FABRICATION, BENDING, AND PLACEMENT OF REINFORCEMENT.
   d. 28-DAY CONCRETE ACCEPTANCE TEST REPORT AS REQUIRED BY ACI 318-14 SECTION 26.13.2

METAL BUILDING SYSTEMS

1. THE METAL BUILDING SYSTEM SHALL BE DESIGNED BY THE METAL BUILDING MANUFACTURER IN CONFORMANCE TO THE PROVISIONS OF THE INTERNATIONAL BUILDING CODE AND THE "LOW-RISE BUILDING SYSTEMS MANUAL" AS PUBLISHED BY THE METAL BUILDING MANUFACTURER'S ASSOCIATION.
2. IT IS THE MANUFACTURER'S RESPONSIBILITY TO DESIGN THE COMPLETE BUILDING SYSTEM (STEEL FRAMING, ANCHORS BOLTS, COMPONENTS, ATTACHMENTS, ETC.); THE MANUFACTURER SHALL SUBMIT A CERTIFICATION LETTER BEARING THE SEAL OF A PROFESSIONAL ENGINEER STATING THAT THE BUILDING SYSTEM DESIGN MEETS THE INDICATED CODE, PERFORMANCE AND LOADING REQUIREMENTS.
3. THE METAL BUILDING SYSTEM SHALL BE PROVIDED BY AN IAS-AC472 ACCREDITED MANUFACTURER.
4. THE SIZE, NUMBER AND PLACEMENT PATTERN OF ALL ANCHOR RODS SHALL BE DETERMINED BY THE PRE-ENGINEERED BUILDING MANUFACTURER AND MUST BE IN ACCORDANCE WITH AISI 318-14 SPACING REQUIREMENTS. ANCHOR ROD EMBEDMENTS ARE INDICATED ON THE DRAWINGS.
5. THE METAL BUILDING SYSTEM ERECTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING NECESSARY FOR SAFE ERECTION.
6. UNLESS OTHERWISE NOTED OR SPECIFIED, ALL STEEL MEMBERS SHALL BE CLEANED AND PAINTED IN ACCORDANCE WITH MANUFACTURER'S STANDARD PROCEDURES.
7. CALCULATIONS FOR FRAME DEFLECTIONS SHALL BE DONE USING ONLY THE BARE FRAME METHOD. REDUCTIONS BASED ON ENGINEERING JUDGMENT USING THE ASSUMED COMPOSITE STIFFNESS OF THE BUILDING ENVELOPE SHALL NOT BE PERMITTED. DRIFT SHALL FOLLOW AISC'S "SERVICEABILITY DESIGN CONSIDERATIONS FOR LOW-RISE BUILDINGS." CALCULATIONS SHALL BE SUBMITTED VERIFYING THAT THE ACTUAL DRIFT UNDER CODE REQUIRED LOADINGS DOES NOT EXCEED THE ALLOWABLE.
8. THE MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND BUILDING REACTIONS INCLUDING THE FOLLOWING:
   a. SHOP DRAWINGS SHALL BE SEALED BY A PROFESSIONAL ENGINEER IN THE PROJECT STATE AND INCLUDE THE FOLLOWING:
      - FULL BUILDING PLAN
      - ANCHOR AND BASEPLATE LAYOUT PLAN
      - ELEVATIONS, SECTIONS, AND DETAILS
   b. BUILDING REACTIONS SHALL BE SEALED BY A PROFESSIONAL ENGINEER IN THE PROJECT STATE AND INCLUDE THE FOLLOWING:
      - MAXIMUM/MINIMUM REACTIONS FOR EACH COLUMN WITH GOVERNING LOAD COMBINATIONS
      - REACTIONS FOR EACH LOAD CASE
      - DEFINITION OF LOAD CASES AND LOAD COMBINATIONS
9. STRUCTURAL DESIGN CRITERIA:
   - WIND SPEED AS SHOWN IN MAIN STRUCTURAL DESIGN CRITERIA
   - SEISMIC DESIGN CRITERIA AS SHOWN IN MAIN STRUCTURAL DESIGN CRITERIA
   - COLLATERAL LOAD = 5 PSF
   - ROOF LIVE LOAD = 20 PSF
   - PURLIN AND RAFTER VERTICAL DEFLECTION LIMIT = L/240
   - LATERAL DRIFT LIMIT = H/100

SAPP STRUCTURAL ENGINEERING | INSPECTIONS

601 EAST 69th STREET
SAVANNAH, GA 31405
(912) 590-0542
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GEORGIA REGISTERED No. SE000802 STRUCTURAL ENGINEER BRIAN K. SAPP

REVISIONS

#	DATE	DESCRIPTION

M.E. SACK COMPANY

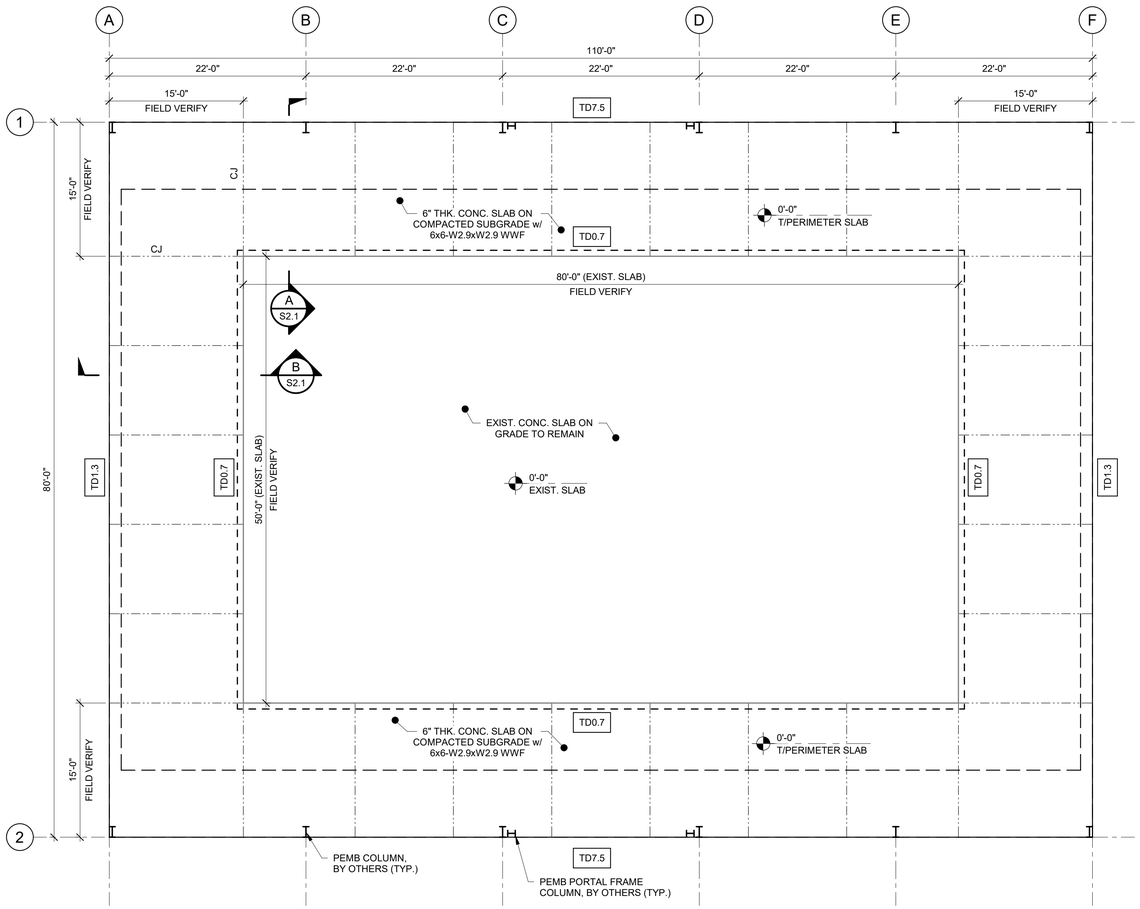
EAST SIDE PARK REMODEL

120 EAST RAILROAD STREET, ALAMO, GA

PROJECT NO.	24.076
DATE	9/18/24
DRAWN BY	SS
CHECKED BY	BKS

STRUCTURAL NOTES

S4.1



FOUNDATION AND SLAB PLAN NOTES:

1. EXIST. SLAB ELEV. (0'-0") IS FOR REFERENCE ONLY. COORDINATE ELEVATION DATUM WITH CIVIL AND ARCHITECTURAL DRAWINGS.
2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS AND ELEVATIONS, DOOR AND WINDOW OPENING LOCATIONS NOT SHOWN.

PLAN LEGEND	
(#)	STRUCTURAL GRID LINE
	FOUNDATION EXTENTS
T#.#	INDICATES TURNDOWN EDGE OR THICKENED SLAB FOUNDATION - SEE SCHEDULE
CJ	SLAB CONTROL JOINT
±FT-IN TARGET	ELEVATION INDICATOR RELATIVE TO REFERENCE ELEV.
I	PEMB COLUMN BY METAL BUILDING MANUFACTURER

SLAB FOUNDATION SCHEDULE

TAG	FOOTING SIZE	REINFORCEMENT
TD0.7	0'-8" x CONT. x 12" THICK.	TOP : (1)#5 CONT.
TD1.3	1'-4" x CONT. x 24" THICK.	TOP : (1)#5 CONT. BOT: (2)#5 CONT. HOOKED BAR: #4 @ 18" O.C.
TD7.5	7'-6" x CONT. x 24" THICK.	TOP : (7)#5 CONT. BOT: (7)#5 CONT. HOOKED BAR: #5 @ 18" O.C.

BUILDING REACTION VERIFICATION/ADJUSTMENT NOTE:

THIS FOUNDATION AND ANCHORAGE DESIGN IS BASED ON PRELIMINARY ESTIMATED BUILDING REACTIONS. AFTER THE PRE-ENGINEERED METAL BUILDING DESIGN HAS BEEN FINALIZED AND BUILDING REACTIONS ARE PROVIDED, THE FOUNDATION AND ANCHORAGE DESIGN WILL REQUIRE REVIEW AND POSSIBLE ADJUSTMENTS. THE DESIGN SHOWN HEREIN IS NOT FINAL UNTIL THIS REVIEW AND VERIFICATION OR ADJUSTMENT.

1 FOUNDATION AND SLAB PLAN
1/8" = 1'-0"

SAPP STRUCTURAL
ENGINEERING | INSPECTIONS
601 EAST 69th STREET
SAVANNAH, GA 31405
(912) 590-0542
SAPPSTRUCTURAL.COM

GEORGIA REGISTERED No. SE000802 STRUCTURAL ENGINEER BRIAN K. SAPP
11/6/24

REVISIONS

#	DATE	DESCRIPTION

M.E. SACK COMPANY

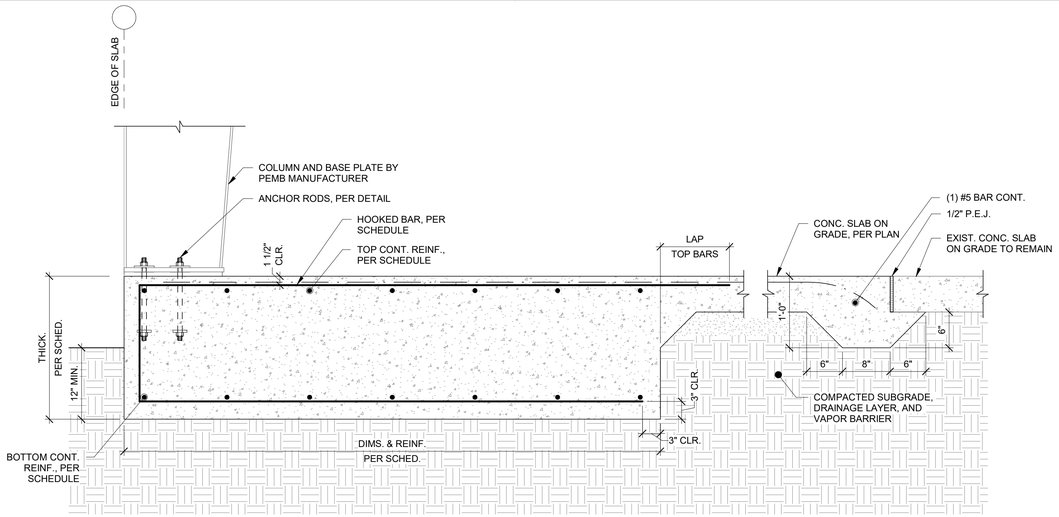
EAST SIDE PARK REMODEL

120 EAST RAILROAD STREET, ALAMO, GA

PROJECT NO.	24.076
DATE	9/18/24
DRAWN BY	SS
CHECKED BY	BKS

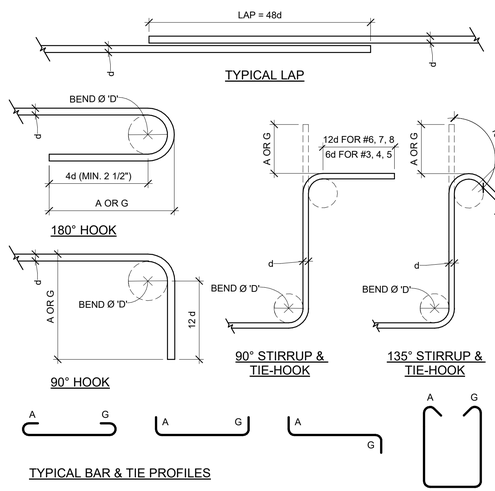
FOUNDATION & SLAB PLAN

S5.1



A
1" = 1'-0"

PEMB MAIN FRAME FOUNDATION



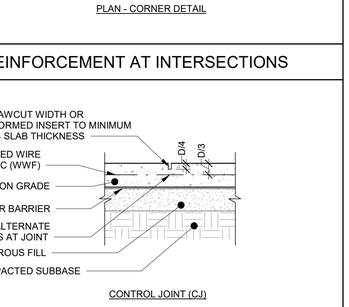
TYPICAL LAP & HOOK SCHEDULE							
BAR SIZE	TYP. LAP (IN.)	STANDARD HOOK			STIRRUP & TIE HOOK		
		D (IN.)	180° HOOK A OR G (IN.)	90° HOOK A OR G (IN.)	D (IN.)	90° HOOK A OR G (IN.)	135° HOOK A OR G (IN.)
#3	18	2 1/4	5	6	1 1/2	4	4
#4	24	3	6	8	2	4 1/2	4 1/2
#5	30	3 3/4	7	10	2 1/2	6	5 1/2
#6	36	4 1/2	8	1-0	4 1/2	1-0	8
#7	42	5 1/4[a]	10	1-2	5 1/4[a]	1-2	9
#8	48	6[a]	11	1-4	6[a]	1-4	10 1/2
#9	54	9 1/2	1-3	1-7			
#10	62	10 3/4	1-5	1-10			
#11	68	12	1-7	2-0			
#14	82	18 1/4	2-3	2-7			
#18	108	24	3-0	3-5			

NOTE:
UNITS = IN. *OR* FT.-IN.

NOTE a:
ASTM A767 REQUIRES THAT BARS BENT COLD PRIOR TO HOT DIP GALVANIZING MUST BE FABRICATED TO A MINIMUM BEND DIAMETER EQUAL TO 7 IN. FOR #7 BAR AND 8 IN. FOR #8 BAR.

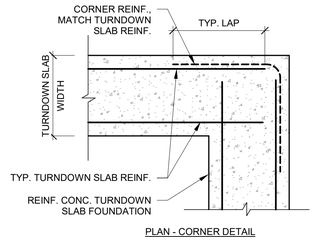
C
1" = 1'-0"

REINFORCING LAP AND HOOK LENGTHS



B
1" = 1'-0"

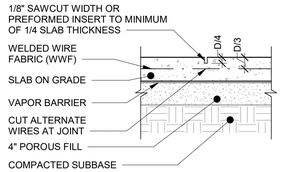
TURNDOWN SLAB FOUNDATION



PLAN - CORNER DETAIL

D
3/4" = 1'-0"

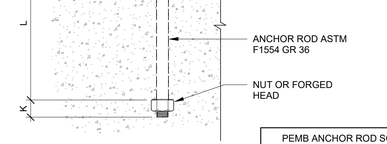
REINFORCEMENT AT INTERSECTIONS



CONTROL JOINT (CJ)

E
1" = 1'-0"

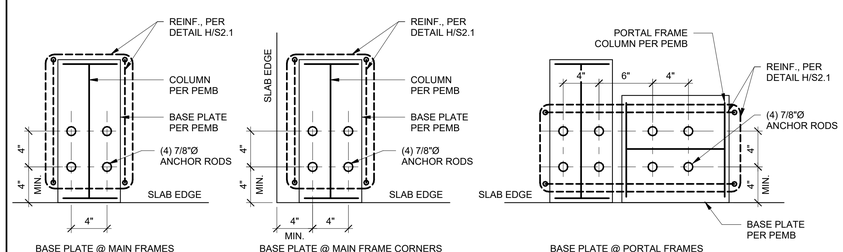
TYPICAL SLAB ON GRADE DETAILS



PEMB ANCHOR ROD SCHEDULE	
DIMENSIONS	BOLT DIAMETER, d
	7/8"
L	9"
K	2"
P (MIN)	3"

F
1" = 1'-0"

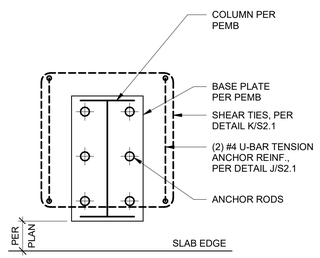
PEMB ANCHOR ROD DETAIL



NOTES:
1. ANCHORS SHALL BE ARRANGED TO MINIMIZE ECCENTRICITY ON COLUMN BASE PLATE. LAYOUTS w/ MORE THAN 1 1/2" ECCENTRICITY WILL REQUIRE ANALYSIS OF ANCHORAGE BY ENGINEER FOR APPROVAL.
2. DIMENSIONS SHOWN ARE MINIMUM REQUIREMENTS.

G
1 1/2" = 1'-0"

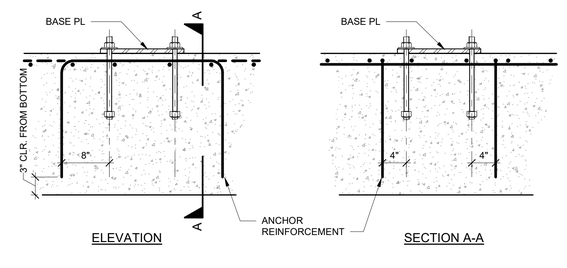
PEMB ANCHOR ROD LAYOUT DETAILS



TYPICAL PLAN DETAIL @ TENSION & SHEAR REINFORCED ANCHORS

H
1 1/2" = 1'-0"

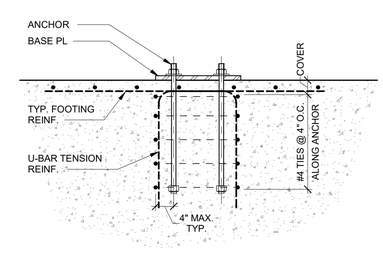
TENSION & SHEAR REINFORCING @ PEMB COLUMN ANCHORAGE



U-BAR TENSION REINFORCING

J
1" = 1'-0"

ANCHOR REINFORCING FOR TENSION @ BASEPLATES



SECTION

SHEAR TIE REINFORCING

K
1" = 1'-0"

ANCHOR REINFORCING FOR SHEAR

601 EAST 69th STREET
SAVANNAH, GA 31405
(912) 590-0542
SAPPSTRUCTURAL.COM

GEORGIA REGISTERED No. SE000802 STRUCTURAL ENGINEER BRIAN K. SAPP
11/6/24

REVISIONS

#	DATE	DESCRIPTION

M.E. SACK COMPANY

EAST SIDE PARK REMODEL

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STRUCTURAL DETAILS

S6.1

LEGEND

	LIGHTING AND POWER
###	CONDUIT RUN CONCEALED ABOVE CEILING OR IN WALL CONTAINING 3 NUMBER 12 CONDUCTORS UNLESS SHOWN OTHERWISE. HASH MARKS, IF SHOWN, INDICATE QUANTITY OF NUMBER 12 CONDUCTORS. WHERE DRAWING SPACE PROHIBITS HASH MARKS BEING SHOWN REFER TO CIRCUIT NUMBERS AND PROVIDE REQUIRED NUMBER OF CONDUCTORS PER CIRCUIT TYPE.
###	CONDUIT RUN CONCEALED IN OR BELOW FLOOR SLAB, OR UNDERGROUND.
### A-2,4,6	HOMERUN TO PANELBOARD, LETTER OR LETTERS INDICATE PANELBOARDS, NUMBERS INDICATE CIRCUIT NUMBERS.
2-E	L.E.D. LIGHTING FIXTURE, "2" INDICATES THE CIRCUIT NUMBER AND "E" THE FIXTURE TYPE. SEE FIXTURE SCHEDULE FOR DIMENSIONS AND MOUNTING TYPE.
	L.E.D. SPORTS LIGHTING FIXTURE. SEE SPORTS LIGHTING FIXTURE SCHEDULE FOR FIXTURE REQUIREMENTS.
	TAMPER RESISTANT DUPLEX CONVENIENCE OUTLET, GFI TYPE. +18" TO CENTER LINE UNLESS OTHERWISE NOTED. "WP" WHERE SHOWN INDICATES WEATHER-RESISTANT DEVICE WITH METAL IN-USE WEATHERPROOF COVER.
	PANELBOARD, SEE SCHEDULE.
	DISCONNECT SWITCH, SIZE AS NOTED ON DRAWINGS. FUSED PER MANUFACTURER'S NAME PLATE DATA OF EQUIPMENT SERVED.
$	SINGLE POLE TOGGLE SWITCH, +46" TO CENTER LINE MOUNTING HEIGHT.

GENERAL NOTES:

1. DO NOT SCALE DRAWINGS TO LOCATE EQUIPMENT OR OUTLETS.
2. MOUNTING HEIGHTS AS INDICATED ON THE DRAWINGS SHALL BE FROM THE FINISHED FLOOR TO THE CENTER LINE OF THE OUTLET BOX.
3. THE ELECTRICAL DRAWINGS ARE ONLY A PART OF THE CONTRACT DOCUMENTS, ALL OF THE DRAWINGS AND SPECIFICATIONS MUST BE REVIEWED FOR THEIR INTERRELATIONSHIP AND REQUIRED COORDINATION BETWEEN DISCIPLINES.
4. ALL CONDUIT AND OUTLET BOXES BE APPROPRIATELY SUPPORTED THROUGHOUT THE PROJECT.
5. PRIOR TO PROJECT COMPLETION, ELECTRICAL CONTRACTOR SHALL PROVIDE TYPEWRITTEN PANELBOARD DIRECTORIES THAT REFLECT SPACE DESIGNATION OF EACH CIRCUIT. NO EXCEPTIONS.
6. ALL CONDUIT ROUTED FROM SLAB/FINISH GRADE UP TO PANELS OR DEVICES EXPOSED SHALL BE GALVANIZED RIGID STEEL.
7. ELECTRICAL CONTRACTOR SHALL BE WARRANTY ALL EQUIPMENT AND INSTALLATION OF SUCH FOR ONE (1) YEAR FROM DATE OF PROJECT ACCEPTANCE. WARRANTY APPLIES TO ENTIRE ELECTRICAL CONTRACTOR'S SCOPE.
8. REFER TO SPECIFICATIONS FOR LABELING OF PANELBOARDS. PROVIDE ENGRAVED I.D. TAGS AS REQUIRED. SEE DETAIL 8/E301 FOR ADDITIONAL REQUIREMENTS.
9. REFER TO SPECIFICATIONS FOR COLORED TRACER REQUIRED ON ALL NEUTRAL CONDUCTORS FOR LIGHTING AND RECEPTACLE CIRCUITS.
10. REFER SPECIFICATIONS FOR REQUIREMENTS OF BONDING BUSHINGS.
11. ALL THHN / THWN WIRE SHALL HAVE FACTORY INSTALLED COLOR CODED OUTER JACKET. REFER TO SPECIFICATIONS. CONTRACTOR IS TO NOTE THAT ALL FEEDER CONDUCTORS SHALL HAVE FULL COLOR CODED OUTER JACKET INTEGRAL TO THE CONDUCTOR INSULATION. USE OF COLOR PHASING TAPE IS NOT ALLOWED.

LIGHTING FIXTURE SCHEDULE

TYPE	DESCRIPTION	WATTS	LUMENS	COLOR TEMP	MANUFACTURERS	NOTES
C	4' STRIP LIGHT FIXTURE SURFACE MOUNTED WITH WHITE ACRYLIC LENS AND WHITE FINISH. (SURFACE, STEM, OR CHAIN SUSPEND WHERE REQUIRED)	38 W	4000	4000 K	LITHONIA "CSS" SERIES METALUX, COLUMBIA, H.E. WILLIAMS, ILP	
D	10.90" LED CIRCULAR HIGHBAY WITH POLYCARBONATE LENS. MOUNT FIXTURE DIRECTLY TO STRUCTURE, PROVIDE SAFETY CHAIN, ATTACHED TO STRUCTURE, TO HOUSING. PROVIDE WIREGUARD.	97 W	16895	4000 K	ORION "RSHB1" SERIES OR EQUAL	
G	LED CIRCULAR CEILING MOUNT FIXTURE WITH ELEGANT DIE-CAST ALUMINUM HOUSING. MOUNT FIXTURE DIRECTLY TO CANOPY. LIGHT DISTRIBUTION SHALL BE TYPE 5 OPTIMIZED.	200 W	18000	4000 K	NLS LIGHTING "ORGANIX ORX-2-CM" OR EQUAL	

SPORTS LIGHTING FIXTURE SCHEDULE

TYPE	DESCRIPTION	WATTS	LUMENS	COLOR TEMP	MANUFACTURERS
F1	LED SPORTS LIGHTING YOKE MOUNTED WITH 10KV SURGE SUPPRESSION AND CASAMBI WIRELESS LIGHTING CONTROLLER. LIGHT DISTRIBUTION SHALL BE 20°. BLACK FINISH WITH A VISOR AND 6FT POWER CORD. 0-10V DIMMABLE.	1000 W	160000	5000 K	JADEMAR "JSLP-HE-1000W-50K-20D-(OPT IC)-V-BK-10SP-YK"
F2	LED SPORTS LIGHTING YOKE MOUNTED WITH 10KV SURGE SUPPRESSION AND CASAMBI WIRELESS LIGHTING CONTROLLER. LIGHT DISTRIBUTION SHALL BE 40°. BLACK FINISH WITH A VISOR AND 6FT POWER CORD. 0-10V DIMMABLE.	1000 W	160000	5000 K	JADEMAR "JSLP-HE-1000W-50K-40D-(OPT IC)-V-BK-10SP-YK"

MOUNTING LOCATION SUMMARY

POLE LABEL	MTG HT (FEET)	FIXTURE QTY PER POLE	TYPE	
			F1	F2
P1	55	10	4	6
P2	55	10	3	7
P3	55	10	4	6
P4	55	10	3	7
TOTAL		40	14	26

PANELBOARD: MP

LOCATION:
MOUNTING: SURFACE NEMA 3R
MAIN DEVICE: 225.0 A MAIN CB
BUS AMPS: 225 AMPS
VOLTAGE: 240/120 High Leg V. 3 ø...
A.I.C. RATING: 22,000 AMPS SYMMETRICAL
SPECIAL:
FED FROM:

LOAD NAME	NOTE	BKR	P	CKT	A		B		C		CKT	P	BKR	NOTE	LOAD NAME
RECEPTACLES		20 A	1	1	0.2	0.0					2	1	20 A		SPARE
POLE P1		35 A	3	3			3.3	3.3			4	3	35 A		POLE P2
				5					3.3	3.3	6				
				7	3.3	3.3					8				
POLE P3		35 A	3	9			3.3	3.3			10	3	35 A		POLE P4
				11					3.3	3.3	12				
				13	3.3	3.3					14				
HIGH LEG DO NOT USE		--	1	15			--	--			16	1	--		HIGH LEG DO NOT USE
SPARE		20 A	1	17					0.0	0.0	18	1	20 A		SPARE
PANEL AM		60 A	2	19	0.6	0.0					20	3	30 A		SPARE
				21			0.5	0.0			22				
PANEL BC		60 A	2	23					1.5	0.0	24				
				25	1.0	0.0					26	3	30 A		TVSS
PANEL CS		100 A	2	27			6.0	0.0			28				
				29					6.0	0.0	30				
		TOTAL LOAD:			15 kVA		20 kVA		21 kVA						
		TOTAL AMPS:			125 A		165.6 A		174 A						

LOAD CLASSIFICATION	CONNECTED	DEMAND	ESTIMATED	PANEL TOTALS	
RECEPTACLES	1620 VA	100.00%	1620 VA		
LITES	42140 VA	125.00%	52675 VA	CONNECTED LOAD:	55760 VA
				ESTIMATED DEMAND:	66295 VA
				CONNECTED CURRENT:	134.1 A
				EST. DEMAND CURRENT:	159.5 A

NOTES:
PANEL IS 120/240V HIGH-LEG, PHASE B SHALL NOT BE USE.

PANELBOARD: AM

LOCATION:
MOUNTING: SURFACE NEMA 1
MAIN DEVICE: 60.0 A MCB
BUS AMPS: 60 AMPS
VOLTAGE: 120/240 V. 1 ø 3 W.
A.I.C. RATING: 10,000 AMPS SYMMETRICAL
SPECIAL:
FED FROM: MP

LOAD NAME	NOTE	BKR	P	CKT	A		B		CKT	P	BKR	NOTE	LOAD NAME
RECEPTACLES		20 A	1	1	0.4	0.0			2	1	20 A		SPARE
RECEPTACLES		20 A	1	3			0.5	0.0	4	1	20 A		SPARE
LIGHTING		20 A	1	5	0.3	0.0			6	1	20 A		SPARE
SPARE		20 A	1	7			0.0	0.0	8	1	20 A		SPARE
SPARE		20 A	1	9	0.0	0.0			10	2	20 A		SPARE
SPARE		20 A	1	11			0.0	0.0	12				
		TOTAL LOAD:			1 kVA		1 kVA						
		TOTAL AMPS:			5 A		4.5 A						

LOAD CLASSIFICATION	CONNECTED	DEMAND	ESTIMATED	PANEL TOTALS	
RECEPTACLES	900 VA	100.00%	900 VA		
LITES	251 VA	125.00%	314 VA	CONNECTED LOAD:	1110 VA
				ESTIMATED DEMAND:	1166 VA
				CONNECTED CURRENT:	4.6 A
				EST. DEMAND CURRENT:	4.9 A

NOTES:

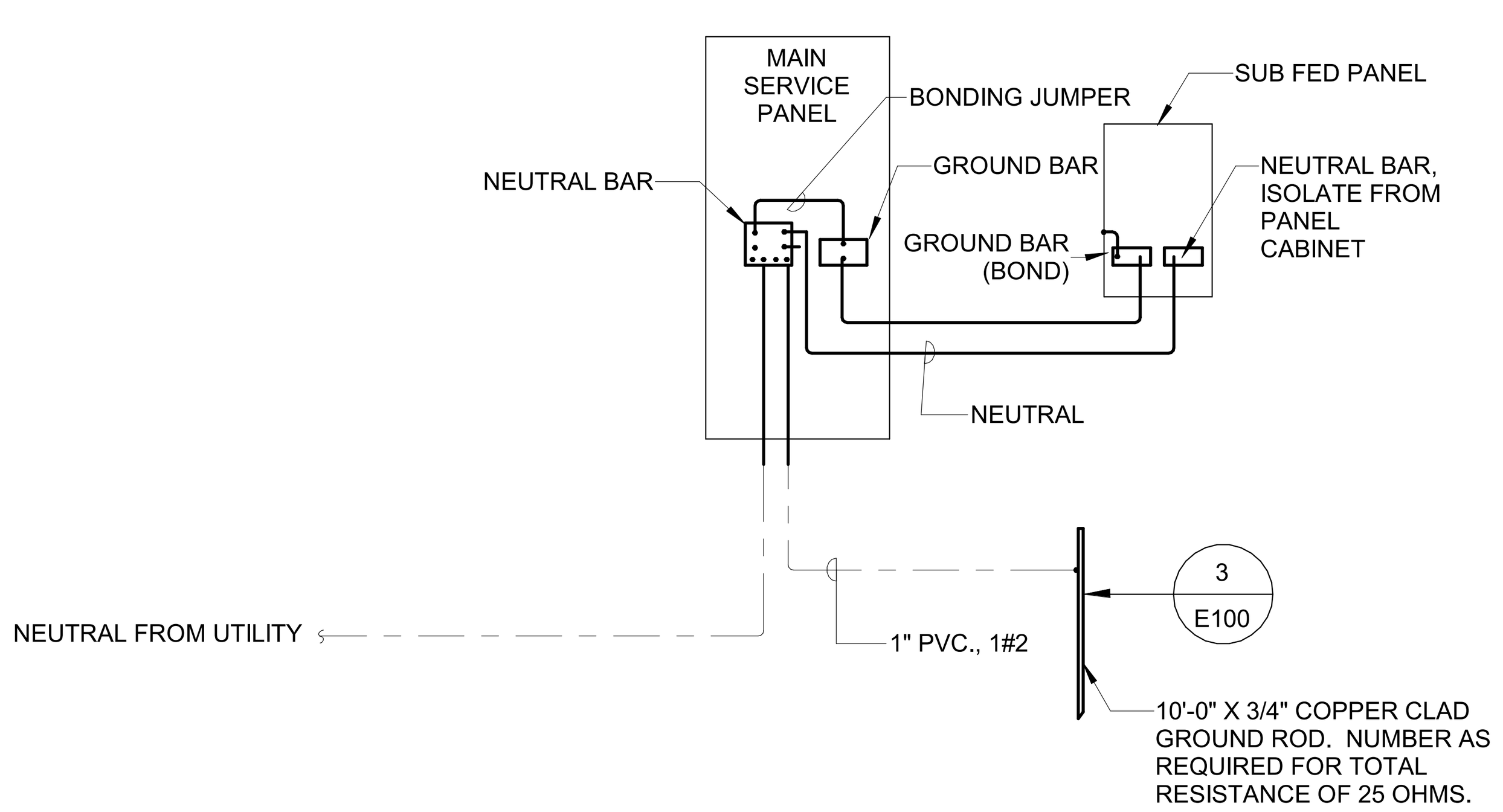
PANELBOARD: BC

LOCATION:
MOUNTING: SURFACE NEMA 3R
MAIN DEVICE: 60.0 A MCB
BUS AMPS: 60 AMPS
VOLTAGE: 120/240 V. 1 ø 3 W.
A.I.C. RATING: 10,000 AMPS SYMMETRICAL
SPECIAL:
FED FROM: MP

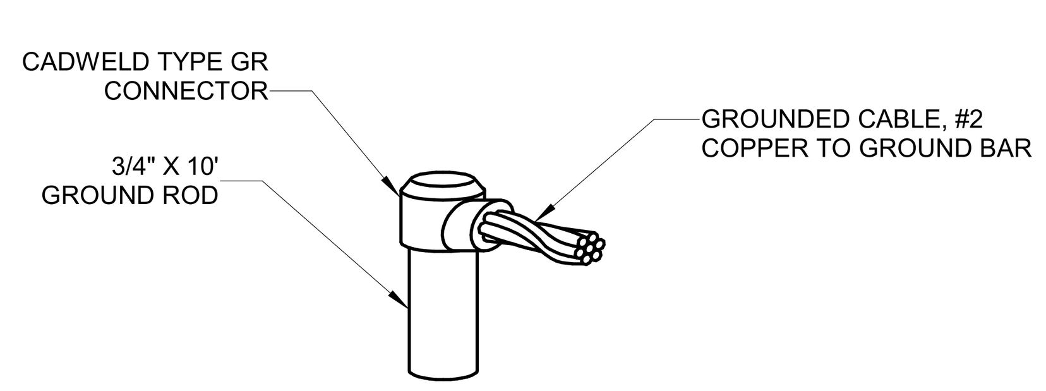
LOAD NAME	NOTE	BKR	P	CKT	A		C		CKT	P	BKR	NOTE	LOAD NAME
RECEPTACLES		20 A	1	1	0.5	0.0			2	1	20 A		SPARE
LIGHTING		20 A	1	3			1.0	0.0	4	1	20 A		SPARE
LIGHTING		20 A	1	5	1.0	0.0			6	1	20 A		SPARE
SPARE		20 A	1	7			0.0	0.0	8	1	20 A		SPARE
SPARE		20 A	1	9	0.0	0.0			10	1	20 A		SPARE
		TOTAL LOAD:			2 kVA		1 kVA						
		TOTAL AMPS:			13 A		8 A						

LOAD CLASSIFICATION	CONNECTED	DEMAND	ESTIMATED	PANEL TOTALS	
RECEPTACLES	540 VA	100.00%	540 VA		
LITES	1940 VA	125.00%	2425 VA	CONNECTED LOAD:	2480 VA
				ESTIMATED DEMAND:	2965 VA
				CONNECTED CURRENT:	10.3 A
				EST. DEMAND CURRENT:	12.4 A

NOTES:



2 E100 DETAIL - SERVICE GROUNDING
NO SCALE



3 E100 DETAIL - CABLE CONNECTION TO GROUND ROD
NO SCALE

T.V.S.S. SCHEDULE

TYPE	DESCRIPTION	NOTES
1	MAIN DISTRIBUTION PANEL (120/240 VOLTS, 3 PHASE) - 300,000 AMP RATED	1

SCHEDULE NOTES:

1. SEE SPECIFICATIONS FOR DETAILS. NEMA 3R ENCLOSURE.

KEYED NOTES:

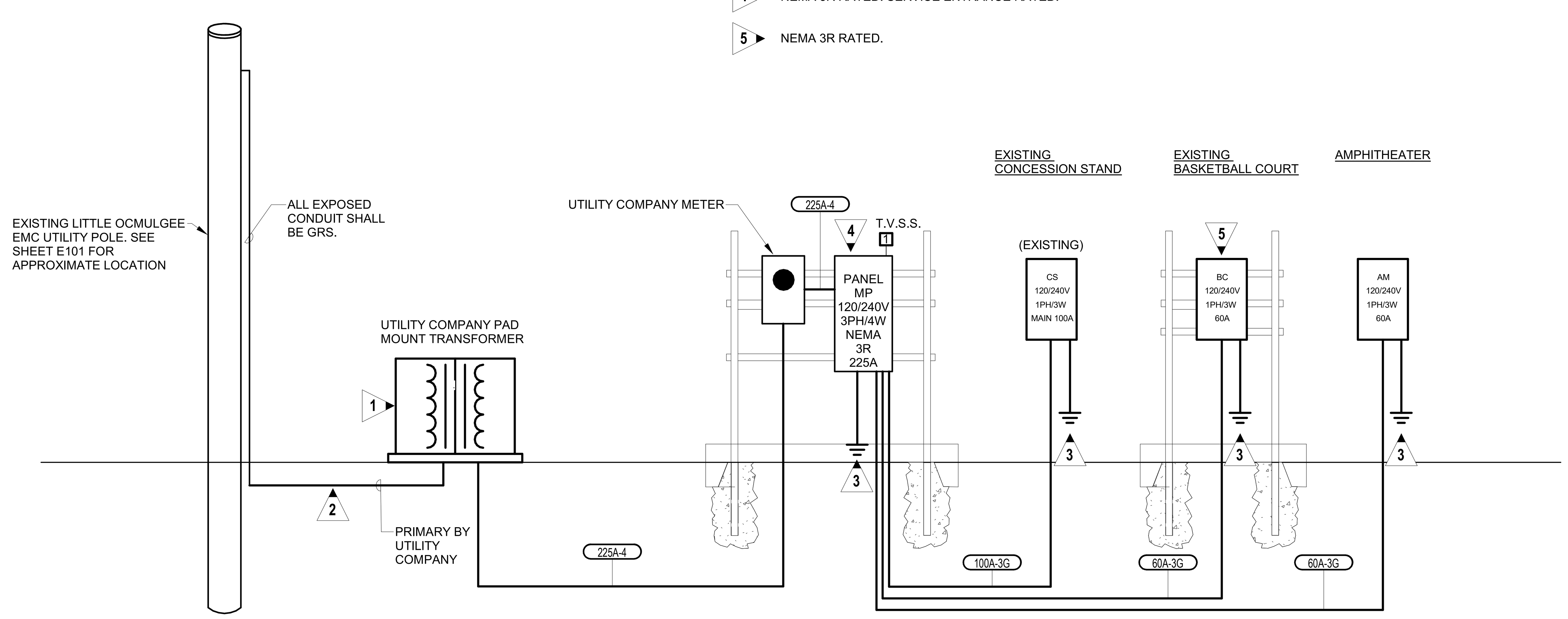
1 ▶ UTILITY COMPANY PAD MOUNTED TRANSFORMER OR TERMINATION CABINET. SEE ELECTRICAL SITE PLAN FOR EXACT LOCATION. PROVIDE CONCRETE PAD PER UTILITY REQUIREMENTS.

2 ▶ UTILITY COMPANY PRIMARY. COORDINATE EXACT ROUTING WITH UTILITY COMPANY PRIOR TO PAD INSTALLATION.

3 ▶ SEE DETAIL 2/E100 FOR SERVICE GROUNDING AND BONDING PLATE DETAIL.

4 ▶ NEMA 3R RATED. SERVICE ENTRANCE RATED.

5 ▶ NEMA 3R RATED.



1 E100 POWER RISER DIAGRAM
NO SCALE

FEEDER SCHEDULE

DESIGNATION	CONDUIT & CONDUCTORS
60A-3G	1.25" C - 3#6, 1#10G
100A-3G	1.5" C - 3#2, 1#8G
225A-4	2.5" C - 4#4/0

200A - 4 G - N FEEDER DESIGNATION

(N) - INDICATES 200% NEUTRAL CONDUCTOR

GROUND CONDUCTORS:
(0) - NO GROUND
(G) - EQUIPMENT OR ISOLATED GND
(BJ) - GROUNDING ELECTRODE
(BJIG) - GROUNDING ELECTRODE AND ISOLATED GND

SYSTEM DESCRIPTION:
(3) - 1Ø 3W OR 3Ø 3W
(4) - 3Ø, 4W

CONDUCTOR AMPACITY:
(AL) - INDICATES ALUMINUM CONDUCTORS

LIGHT FIXTURE DESIGNATION DESCRIPTION

EXAMPLE: B/HA1-1,b

PANEL DESIGNATION:
CIRCUIT NUMBER:
FIXTURE TYPE: TYPE "A" IF NO CAPITAL LETTER SHOWN
SWITCH LEG DESIGNATION WHEN MULTIPLE SWITCHES ARE IN SAME SPARE/AREA

B/ HA1 -1, b

EDC ELECTRICAL DESIGN CONSULTANTS
ELECTRICAL ENGINEERS
SAVANNAH OFFICE - PROJECT #: S24015
edc1973.com

REVISIONS:

DESIGN PROFESSIONAL:
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GEORGIA REGISTERED NO. 038816 PROFESSIONAL ENGINEER BRIAN W. NEWLAND

DATE: 02/14/25

M.E. SACK ENGINEERING 19 80

MUNICIPALITY:
CITY OF ALAMO

COUNTY:
WHEELER COUNTY

OWNER:
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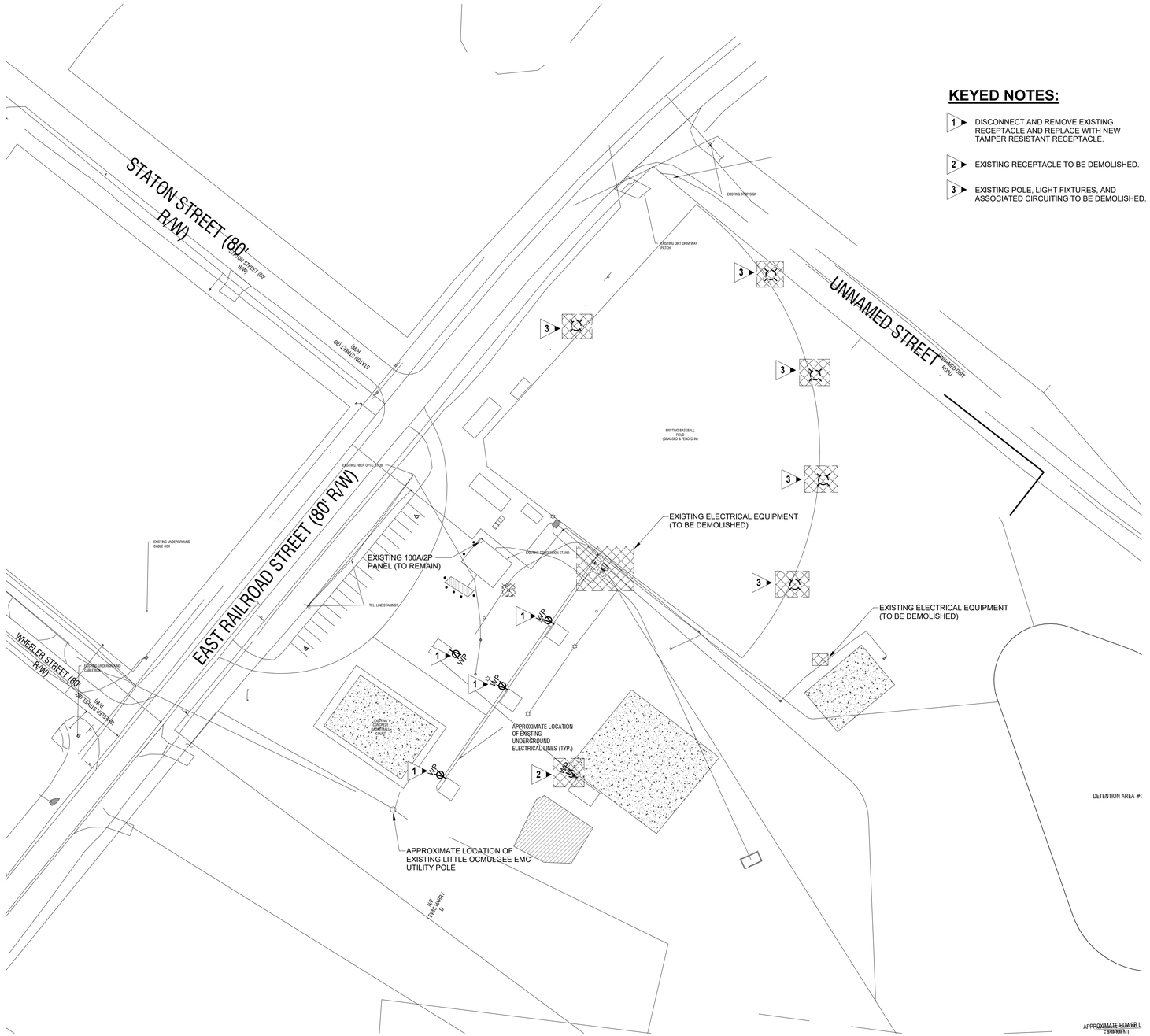
EAST SIDE PARK REMODEL

LEGEND, DETAILS, AND FIXTURE SCHEDULE

E100

FILE NO: 2023-66

PLOT DATE: 02/14/25



1 ELECTRICAL SITE DEMOLITION PLAN
E101 SCALE: 1" = 50'-0"

KEYED NOTES:

1. DISCONNECT AND REMOVE EXISTING RECEPTACLE AND REPLACE WITH NEW TAMPER RESISTANT RECEPTACLE.
2. EXISTING RECEPTACLE TO BE DEMOLISHED.
3. EXISTING POLE, LIGHT FIXTURES, AND ASSOCIATED CIRCUITING TO BE DEMOLISHED.

DEMOLITION NOTES:

1. THE CONTRACTOR SHALL FIELD VERIFY EXACT ROUTINGS OF EXISTING RACEWAYS BEFORE STARTING ANY WORK AND NOTIFY THE ARCHITECT OF ANY KNOWN DISCREPANCIES.
2. THE CONTRACTOR SHALL REMOVE EXISTING CONDUCTORS AND INSTALL NEW CONDUCTORS AS SHOWN OR AS REQUIRED TO COMPLETE REVISED CIRCUITS AND TO CONFORM TO N.E.C.
3. ALL EXISTING EQUIPMENT REMOVED FROM SERVICE AND NOT INTENDED FOR REUSE SHALL REMAIN THE PROPERTY OF OWNER AND SHALL BE STORED OR DISPOSED OF AS DIRECTED BY THE OWNER.
4. EXISTING CEILING, WALLS AND FLOORS DISTURBED OR DISFIGURED BY THE ELECTRICAL RENOVATIONS SHALL BE PATCHED, MENDED OR REPLACED BY TRADES ACTIVELY PARTICIPATING IN THIS TYPE OF WORK. RESPONSIBILITY FOR REPAIRS SHALL BE COORDINATED BETWEEN GENERAL CONTRACTOR AND ELECTRICAL SUBCONTRACTOR.
5. EXISTING EQUIPMENT SHOWN ON ELECTRICAL DRAWINGS THAT WILL REMAIN SHALL HAVE SERVICE MAINTAINED OR RECONNECTED TO EXISTING OR NEW PANELBOARD AS NECESSARY.
6. ALL EXISTING LIGHT FIXTURES SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE OWNER. (SEE NOTE #3).
7. CONTRACTOR SHALL REMOVE ALL EXISTING ELECTRICAL AND LOW VOLTAGE INFRASTRUCTURE INSIDE OF AND ON EXTERIOR OF EXISTING BUILDING IN ITS ENTIRETY UNLESS OTHERWISE NOTED TO REMAIN OR BEING REUSED TO RECONNECT NEW CIRCUITS.
8. DISCONNECT AND REMOVE ALL ELECTRICAL DEVICES IN WALLS TO BE DEMOLISHED. DISCONNECT AND REMOVE ASSOCIATED CONDUIT AND WIRE BACK TO LAST REMAINING DEVICE. FURNISH AND INSTALL CONDUIT AND WIRE AS NECESSARY FOR CONTINUITY OF CIRCUIT(S) TO ANY EXISTING DEVICES TO REMAIN. COORDINATE AND VERIFY REQUIREMENTS WITH NEW WORK IN AREA.
9. FURNISH AND INSTALL CONDUIT AND WIRE AS NECESSARY FOR CONTINUITY OF BRANCH CIRCUITS ORIGINATING OUTSIDE THE DEMOLITION AREA THAT SERVES ANY ELECTRICAL EQUIPMENT OR DEVICES TO REMAIN AFTER DEMOLITION. MODIFY OR REPLACE AS REQUIRED.

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EAST SIDE PARK REMODEL

ELECTRICAL SITE DEMOLITION PLAN

E101

FILE NO: 2023-66

PLOT DATE: 02/14/25

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ELECTRICAL ENGINEER'S
SAVANNAH GEORGIA PROJECT #: 024115
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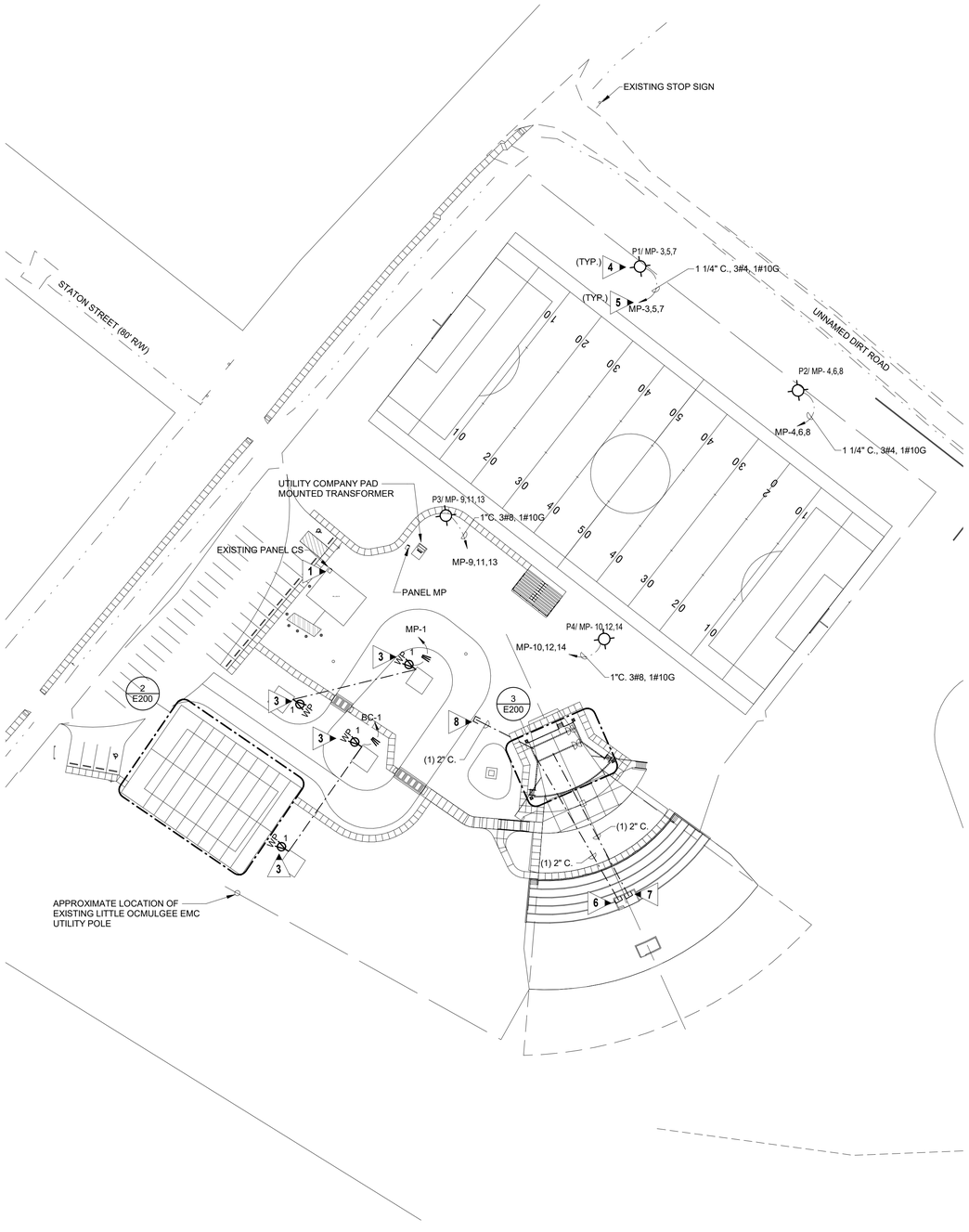
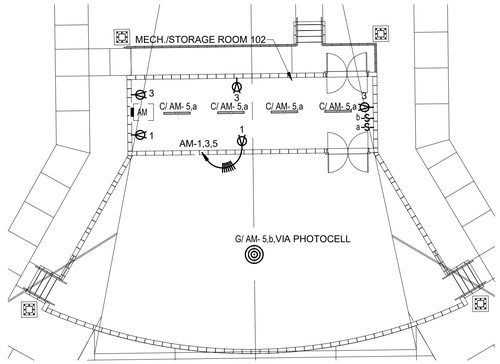
SCALE BAR 1" = 50'
50 0 25 50 100 200



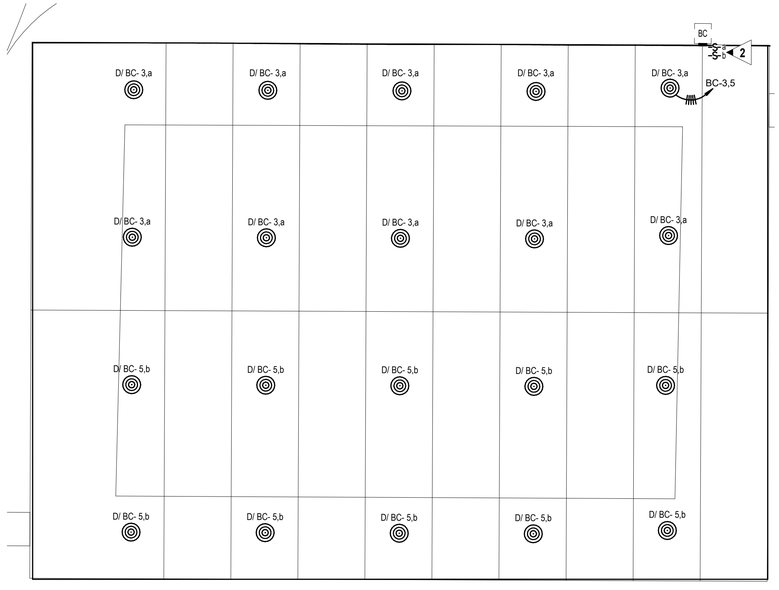
1 E200 ELECTRICAL SITE PLAN
SCALE: 1" = 50'-0"

KEYED NOTES:

1. RE-FEED PANEL "CS" FROM NEW PANEL "MP". SEE RISER ON SHEET E101 FOR ELECTRICAL REQUIREMENTS.
2. PROVIDE SWITCHES IN LOCKABLE WEATHERPROOF JUNCTION BOX.
3. RE-CIRCUIT EXISTING RECEPTACLES TO PANEL "MP".
4. NEW LED SPORTSLIGHTING POLE. SEE SCHEDULE, SPECIFICATIONS, AND DETAILS ON SHEET E301.
5. CONNECT JADEMAR POLE MOUNTED DISCONNECT SERVING LIGHT FIXTURES. HOMERUN TO PANEL INDICATED.
6. STUB OUT CONDUIT FROM FUTURE SOUND BOOTH TO PANEL "AM" LOCATED IN MECH./STORAGE ROOM 102. PROVIDE CONCRETE MARKER, PULL STRING, AND CAP OFF BELOW GRADE AS SHOWN IN DETAIL 7/E301.
7. STUB OUT CONDUIT FROM FUTURE SOUND BOOTH TO MECH./STORAGE ROOM 102. PROVIDE CONCRETE MARKER, PULL STRING, AND CAP OFF BELOW GRADE AS SHOWN IN DETAIL 7/E301.
8. STUB OUT CONDUIT FROM MECH./STORAGE ROOM 102. PROVIDE CONCRETE MARKER, PULL STRING, AND CAP OFF BELOW GRADE AS SHOWN IN DETAIL 7/E301.



3 E200 ELECTRICAL PLAN - AMPHITHEATER
SCALE: 3/32" = 1'-0"



2 E200 ELECTRICAL PLAN - BASKETBALL COURT
SCALE: 3/32" = 1'-0"

GENERAL NOTES:

1. DIVISION 26 TO COORDINATE EXACT LOCATION OF UTILITY TRANSFORMER WITH POWER COMPANY PRIOR TO BID. DIVISION 26 SHALL COORDINATE EXACT ROUTING OF UNDERGROUND PRIMARY CONDUCTORS WITH POWER COMPANY PRIOR TO BID. NO ADDITIONAL COST SHALL BE INCURRED BY THE OWNER DUE TO THE LACK OF COORDINATION BY THE CONTRACTOR.
2. THE EXISTING SITE PLAN DOES NOT SHOW ALL LANDSCAPING OR PLANTING ARRANGEMENTS. THE ELECTRICAL CONTRACTOR SHALL COORDINATE WITH SITE ARCHITECTURAL LANDSCAPING AND CIVIL DRAWINGS TO AVOID CONFLICTS WITH LANDSCAPING FEATURES AND PLANTING REQUIREMENTS.
3. PROVIDE MARKING TAPE OVER ALL CONDUITS OUTSIDE OF BUILDING FOOTPRINT.
4. SURVEY AND SITE INFORMATION PROVIDED BY OTHERS. VERIFY ALL CONDITIONS ON SITE AND WITH OFFICIAL SURVEYS AND OTHERS TRADE.
5. CALL UNDERGROUND UTILITY CENTER AND VERIFY ALL UNDERGROUND UTILITIES.
6. UNDERGROUND CONDUIT SHALL BE SCHEDULE 40 PVC, UNO.
7. CONTRACTOR SHALL STAKE-OFF ALL EXISTING UTILITIES PRIOR TO ROUGH-IN. ALL NEW INSTALLATION SHALL BE COORDINATION WITH EXISTING UTILITY LOCATIONS.
8. PROVIDE HANDHOLES AS REQUIRED BY NEC FOR UNDERGROUND FEEDERS SHOWN.
9. CONDUCTORS ARE SIZED ON THIS PLAN FOR VOLTAGE DROP BASED ON THE CIRCUIT LOADS. PROVIDE LUG ADAPTORS ON BOTH ENDS OF CIRCUIT TO CONNECT TO DISCONNECT AND BREAKER IN PANELS.
10. FIELD VERIFY EXACT SPORTSLIGHTING POLE LIGHT LOCATIONS WITH SPORTSLIGHTING SHOP DRAWINGS PRIOR TO ROUGHING IN.
11. DO NOT ROUTE UNDERGROUND CONDUIT BELOW FOOTBALL FIELD.
12. REFER TO SPECIFICATIONS FOR REQUIREMENTS ON THE SPORTSLIGHTING PACKAGE.
13. ALL CONDUIT ROUTINGS ARE APPROXIMATE. COORDINATE ROUTINGS WITH CIVIL DRAWINGS AND ADJUST ACCORDINGLY.
14. REFER TO CIVIL DRAWINGS FOR EXISTING CONDITIONS (i.e. DRIVEWAYS, SIDEWALKS, ETC.) THAT MAY REQUIRED CUTTING AND PATCHING FOR CONDUIT INSTALLATION.
15. SPORTS LIGHTING CONTROLS BY CASAMBI OR PRIOR APPROVED EQUAL. PROVIDE ALL COMPONENTS NECESSARY FOR A COMPLETE AND OPERATIONAL LIGHTING CONTROL SYSTEM. LIGHTING CONTROL SYSTEM SHALL INCLUDE SYSTEM START-UP AND OWNER TRAINING. SEE SPECIFICATION SECTION 264000 AND 260943.
16. THE USE OF MC CABLE IS NOT ALLOWED.

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EAST SIDE PARK REMODEL

ELECTRICAL SITE PLAN

E200

FILE NO: 2023-66

PLOT DATE: 02/14/25

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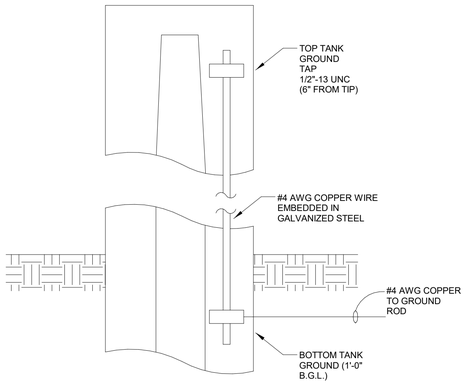
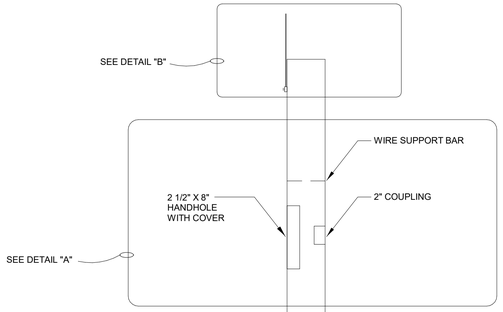
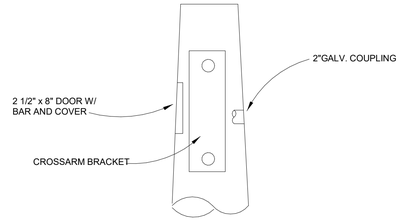
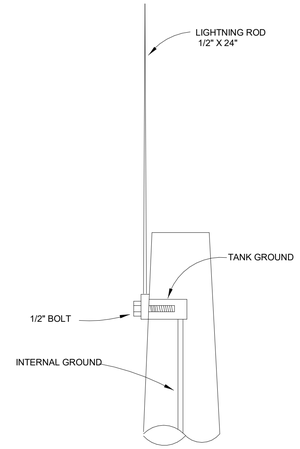


1 / E301 DETAIL - INTERNAL GROUND
NO SCALE

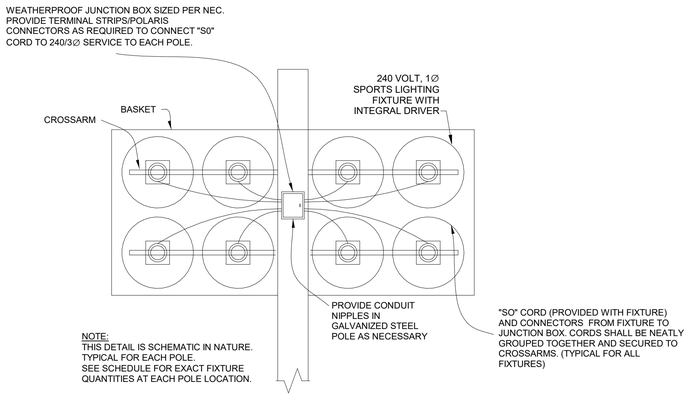




3 / E301 DETAIL A
NO SCALE

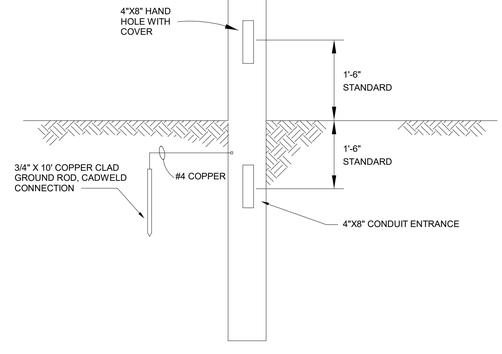


4 / E301 DETAIL B
NO SCALE

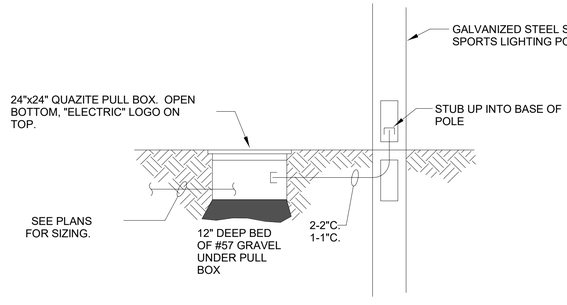


NOTE:
DETAIL SHOWN IS FOR 240 VOLT / SINGLE PHASE FIXTURES.

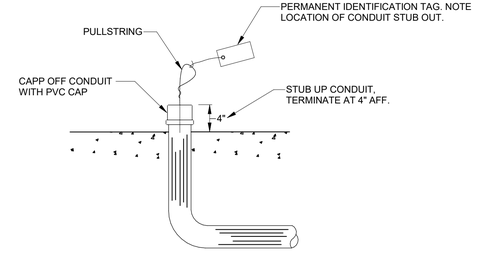
5 / E301 TYPICAL POLE/BASKET DESIGN
NO SCALE



2 / E301 TYPICAL GALVANIZED STEEL POLE
NO SCALE



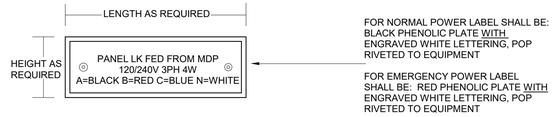
6 / E301 DETAIL - SPORT LIGHTING POLE DETAIL
NO SCALE



7 / E301 DETAIL -STUB UP CONDUIT DETAIL
NO SCALE

FOR 120/240V PANELS:

NOTE: DESIGNATIONS ARE FOR REFERENCE ONLY. LABELING SHALL BE OF EQUIPMENT BEING SERVED.



8 / E301 IDENTIFICATION TAG DETAIL
NO SCALE

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EAST SIDE PARK REMODEL

ELECTRICAL DETAILS

E301

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